REHABILITATION STRUCTURE SURVEY REPORT

DT1696 **⊠** Grade Separation ☐ Stream Crossing ☐ Culvert ☐ Railroad ☐ Retaining Wall □ Noise Barrier Sign Structure Other: For guidance see: http://dotnet/dtid_bos/extranet/structures/reports-checklists.htm Design Project ID Construction Project ID Highway (Project Name) **EAU CLAIRE - CHIPPEWA FALLS** 1190-02-34 1190-02-64 Final Plan Due Date Preliminary Plan Due Date ☐ Town ☐ Village ☐ City 05/01/2018 05/01/2018 **EAU CLAIRE** PS&E Date Letting Date County 08/01/2018 02/12/2019 **EAU CLAIRE** Structure Number Section Town Range B-18-178 27 27N 09W Station Latitude: 444715.0 Structure Located on National Highway System 238+10.34 - 239+58.16 Longitude: 912710.6 For Survey and CADD Files **Traffic Forecast Data** Horizontal Coordinate System: Average Daily Roadway Vertical Datum: Design Year Traffic (ADT) Design Speed **Functional Class** Feature On Feature On Principal 18800 70 MPH USH 53 SB 2008 Arterial Feature Under Feature Under Principal 15400 50 MPH HASTINGS WAY 2014 Arterial Region Contact: Adam Hetrick Consultant Contact: (Area Code) Telephone Number(s): 715-836-2855 (Area Code) Telephone Number(s): Email: adam.hetrick@dot.wi.gov Fmail: Work To Be Performed Field Information Required Item Number (see Pages 2-4) ☐ A. Structural Repair1−3, 22 ☑ B. Overlay.......1–3, 10–22, 26–28, 32, 34 ☐ Concrete Overlay ☐ Asphalt Overlay ☐ Polymer Modified Asphalt Overlay ☐ Other: □ D. New Railings15–17, 20–23 ☐ H. New Deck......1–6, 9, 10, 13–28, 32–34 ☐ M. Slope Stabilization......1–3, 30 □ P. Other:

Field Information Required

If no structure number exists provide the following: Small County Map on which the location of proposed structure is shown in red and any highway relocation in green. In addition, provide Location Map of scale not less than 1" = 2000' showing the structure location and number.

\boxtimes	1.	Most recent inspection report, brief history of bridge construction date, and description of repairs with dates.
\boxtimes	2.	Outline deficient areas on existing structure plan or drawing.
	3.	Photographs of details requiring repairs or modifications, such as: bearings, x-frames, joints, etc. Photograph all deficient areas. Clearly label all photographs.
	4.	Provide proposed typical section for roadway and structure showing dimensions and cross slopes.
	5.	Survey beam seat or girder elevations at both sides of bridge at all substructure units.
	6.	Provide cross-section elevations at 10 foot intervals extending across the structure and a minimum of 100 feet beyond each end. Sections should be normal to centerline and show elevations at centerline roadway and gutter line. Take elevations along joints and at floor drains.
	7.	Show and identify starting stationing on bridge.
	8.	Record measurement, temperature of the structure, and date taken for each of the following: (a) Joint opening measured normal to joint at centerline of roadway and both curb lines. (b) Clearance between girder ends at piers. (c) Distance from front face of abutment backwall to closest point of girder end measured parallel to girder. (d) Temperature of structure determined by averaging top and under deck (if accessible) readings.
	9.	Fixed and expansion bearings - condition and orientation.
⊠1	0.	Number and width of proposed pours including construction staging sequence.
⊠1	1.	Location of existing construction joints in the deck.

Sq. Yd. <u>0</u>	
Sq. Yd. <u>0</u>	
Sq. Yd. <u>0</u>	Galvanic Anodes? NO
Sq. Ft. <u>0</u>	Galvanic Anodes? NO
Sq. Ft. <u>0</u>	Galvanic Anodes? NO
LF. <u>0</u>	Galvanic Anodes? NO
	Sq. Yd. <u>0</u> Sq. Ft. <u>0</u> Sq. Ft. <u>0</u>

⊠13. Sufficiency number: 100.0 (obtain from HSI Bridge Inventory System)

	Deck Condition	Superstructure Condition	Substructure Condition	Load Capacity Appraisal	Structural EVAL Appraisal
Current	8	8	8	5	8

	Inventory	Operational
Current	11000	11070
Calculated Date: 6/5/2013	HS28	HS73
After		
Completed by Bridge Designer		

	☐ Yes ☒ N	Owner and Contact Information	Size	Opening at Abutment	Weight	Pressure
	1960	Owner and Contact Information	O.Z.C	Abathon	Weight	Tressure
\boxtimes	-	dge railing deficient? No If Yes – Replacement Rail Type:				
	18. Drains to be: ☐ Raised	☐ Closed ☐ Downspouted ☐ New				
\boxtimes		ined on bridge during work? No If Yes – Include sketches				
\boxtimes	20. Will guard rail ☐ Yes ⊠ N	be attached? No If Yes – Which corners? Existing guardrail to remain	ain at all co	orners.		
\boxtimes		e performed eliminate all deficiencies? No If No – Explain:				
\boxtimes		aste (asbestos) to be removed? No If Yes – Explain:				
	23. Wing location	(s) for surface drain anchors:				
\boxtimes		No If Yes – Explain on Page 4 g, color system, containment, bid items)				
		way width: <i>(new deck / widening)</i> Ft. walk clear width: Left: Ft. Right: Ft.				
\boxtimes	26. Maximum inc	rease in grade line elevation 3/8 In.				
\boxtimes	27. Benchmark de	escription to be shown				
\boxtimes	28. Desired final	cross slopes on bridge $SE = 0.06$ Ft./Ft.				
		t -				
	30. Slope stabiliza	ation, provide: CY. Ft./Ft. Fill:CY.				
	-	CY.				

\boxtimes	32.	Report submitted with Preliminary Plan requires no CADD file submittal (See ESubmittal instructions).
	33.	Report submitted for development of Preliminary Plan to structure design engineer requires CADD file (if available) submittal and Report submittal to Soils Engineer if project involves foundation modifications.
	34.	Coordinate with structure design engineer before going into the field if existing structure has no available plans, if staged construction is planned, or if there are adjoining/adjacent structures that will remain in place.
	35.	If project involves substructure widening coordinate with structure and/or hydraulic design engineer to determine if information on the separation and/or stream crossing SSR will be required.

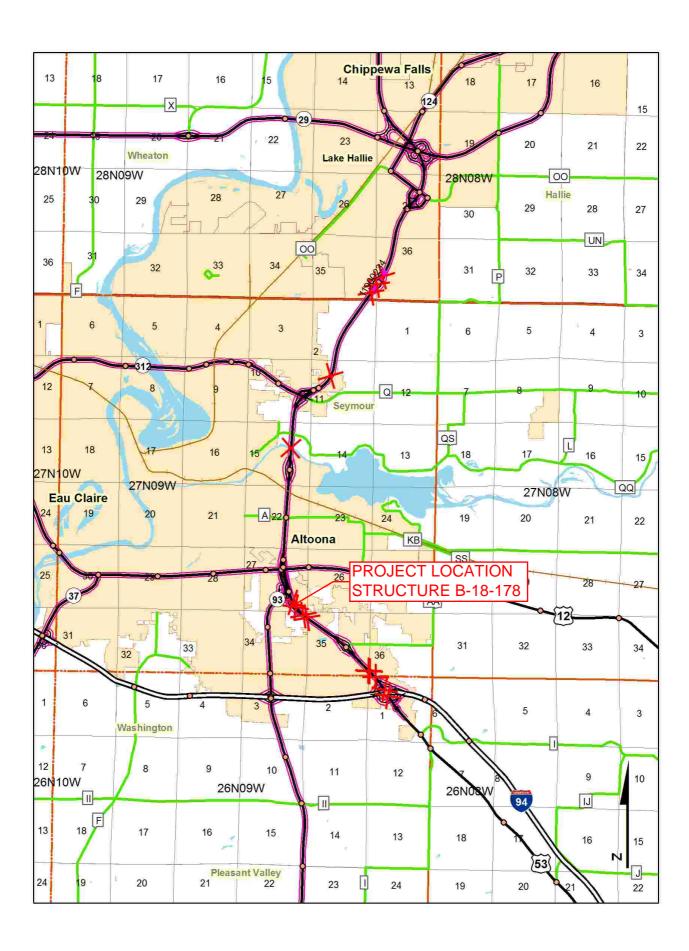
Additional Information

Elaborate on other concerns such as: DNR, Local, Utility Conflicts, Aesthetics, Railing Type and Staged Construction.

Please be as detailed and specific as possible.

- 1.) Structure built in 2006. No repairs since construction. See attached Bridge Inspection Report.
- 2.) Deficient areas to be determined in the field by the engineer. See attached Bridge Inspection Report. A Polymer Overlay is proposed because of deficiency over the entire structure due to poor bridge deck surface. The deck has a couple hairline diagonal cracks at the deck ends and a few hairline transverse cracks. The deck has diagonal cracking at the northwest and southeast corners. The northwest approach shoulder has a large diagonal crack in it and has settled about 2 inches. The northeast, southeast, and southwest approach shoulders have settled about 2 inches.
- 3.) See attached photographs.
- 10.) This work will be constructed half at a time under traffic using single lane closures during non-peak hours with night work. Nighttime ramp closures are anticipated at some structures. All lanes and ramps will be opened to traffic daily.
- 11.) See asbuilt plans.
- 16.) No utilities on or near structure. No conflicts anticipated.
- 19.) This work will be constructed half at a time under traffic using single lane closures during non-peak hours with night work. Nighttime ramp closures are anticipated at some structures. All lanes and ramps will be opened to traffic daily.
- 22.) See attached Asbestos Inspection Report. No asbestos-containing material was found.
- 27.) To be determined.
- 32.) See preliminary plans.

CDR Map



₽

ORDER	0F	SHEETS

Section No. 1

Section No. 2 Typical Sections and Details Estimate of Quantities Section No. 3 Miscellaneous Quantities Section No. 4 Right of Way Plat

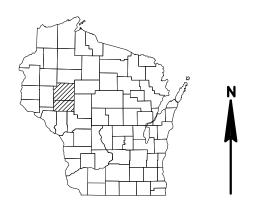
Section No. 5 Plan and Profile

Section No. 6 Standard Detail Drawings

Section No. 9 Computer Earthwork Data

Section No. 9 Cross Sections

TOTAL SHEETS =



DESIGN DESIGNATION

A.A.D.T. A.A.D.T. D.H.V. D.D. DESIGN SPEED **ESALS**

CONVENTIONAL SYMBOLS PI AN CORPORATE LIMITS PROPERTY LINE LOT LINE LIMITED HIGHWAY EASEMENT EXISTING RIGHT OF WAY PROPOSED OR NEW R/W LINE SLOPE INTERCEPT

REFERENCE LINE EXISTING CULVERT PROPOSED CULVERT COMBUSTIBLE FLUIDS

MARSH AREA

WOODED OR SHRUB AREA

CULVERT (Profile View) UTILITIES ELECTRIC FIBER OPTIC SANITARY SEWER STORM SEWER TELEPHONE UTILITY PEDESTAL POWER POLE ₫ Ø TELEPHONE POLE

PROFILE

GRADE LINE

ORIGINAL GROUND

SPECIAL DITCH

GRADE ELEVATION

MARSH OR ROCK PROFILE

(To be noted as such)

STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

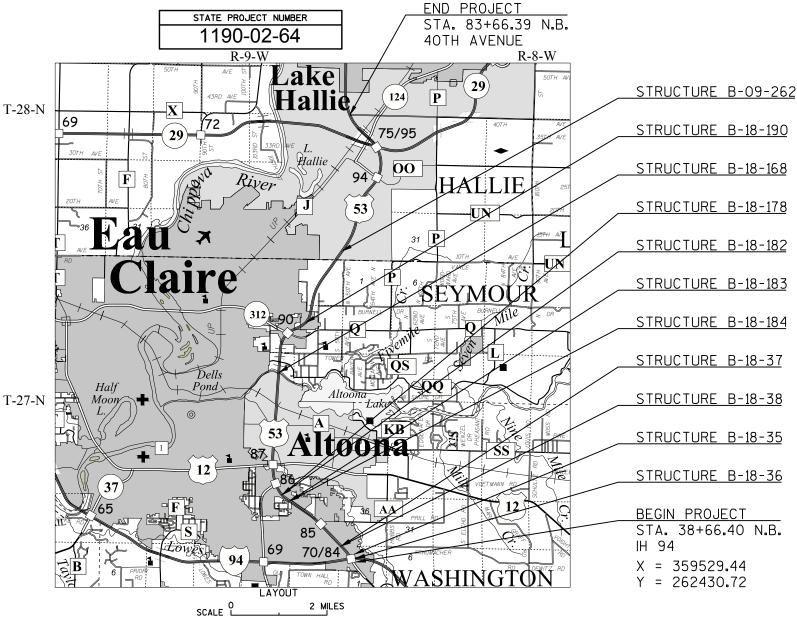
PLAN OF PROPOSED IMPROVEMENT

EAU CLAIRE - CHIPPEWA FALLS

IH 94 TO 40TH AVENUE (11 BRIDGES)

USH 53

EAU CLAIRE AND CHIPPEWA COUNTIES



HORIZONTAL POSITIONS SHOWN ON THIS PLAN ARE WISCONSIN COUNTY COORDINATES, EAU CLAIRE COUNTY, NAD83 (1991), IN U.S. SURVEY FEET. VALUES ARE GRID COORDINATES, GRID BEARINGS, AND GRID DISTANCES. GRID DISTANCES MAY BE USED AS GROUND DISTANCES.

PREPARED BY WISDOT Surveyor ADAM HETRICK DAVID KOEPP REGIONAL EXAMINE TMOTHY MASON APPROVED FOR THE DEPARTMENT

STATE OF WISCONSIN

DEPARTMENT OF TRANSPORTATION

FEDERAL PROJECT

CONTRACT

PROJECT

STATE PROJECT

1190-02-64

(Signature)

TOTAL NET LENGTH OF CENTERLINE = 11.26 MILES

Ε



Lat: 44.78777818 Long: -91.45303642 Elev: 796.57 ft.

\\doteauplog1p\photolog\Rg5\053S_R5_2013\\Front\Dir_140\F_14071.jpg



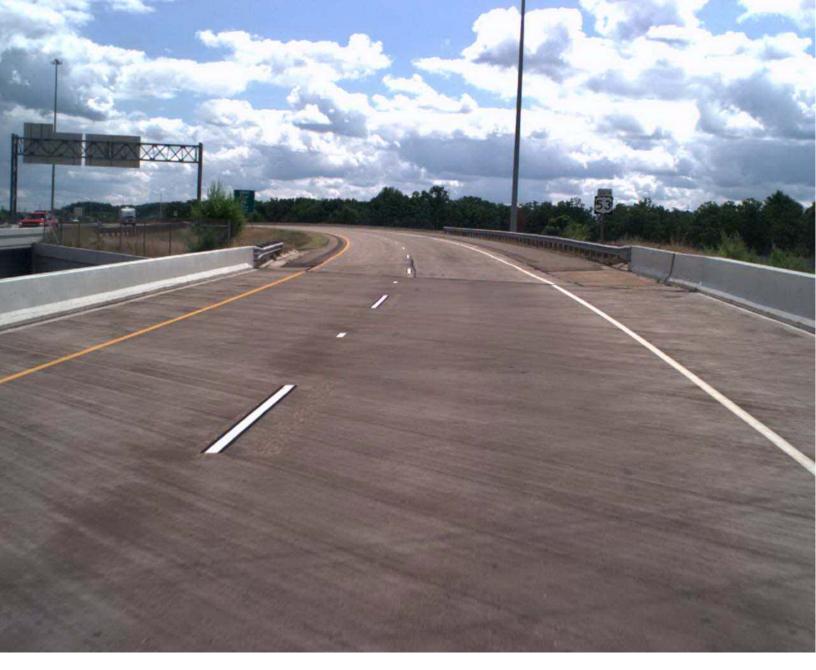
Lat: 44.78764373 Long: -91.45296299 Elev: 798.21 ft.

\\doteauplog1p\photolog\Rg5\053S_R5_2013\\Front\Dir_140\F_14072.jpg



Lat: 44.78750909 Long: -91.45288552 Elev: 799.85 ft.

\\doteauplog1p\photolog\Rg5\053S_R5_2013\\Front\Dir_140\F_14073.jpg



Lat: 44.78737642 Long: -91.45280568 Elev: 801.19 ft.

\\doteauplog1p\photolog\Rg5\053S_R5_2013\\Front\Dir_140\F_14074.jpg



Lat: 44.78724346 Long: -91.45272118 Elev: 802.63 ft.

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Inspection Report for B-18-178

USH 53 SB over HASTINGS WAY Jul 15,2015

	Туре				Prior	Frequency (mos)	Performed
	Routine				07-15-15	24	X
	SI&A				07-18-11	48	X
Latitude	44°47'15.00"N			Owner	STATE HIGHWA	AY DEPT	
Longitude	91°27'10.60"W			Maintainer	STATE HIGHWA	AY DEPT	
	Time Log		Team memb	ers			
	Hours 1	Minutes 11					
	Name		Number	Signature			Date
Inspector							
	Frueh, Rick J		1003	Completed by HSI Sy	stem Account(HSI)		
Reviewer							

page 2

Identification & Location

Feature On: USH 53 SB	ection Town Range: Structure Number:				
Feature Under: HASTINGS WAY	County: EAU CLAIRE(18)	B-18-178			
Location 0.7M S JCT CTH HH CITY-EAU CLAIRE(18221)		Structure Name:			

Geometry Traffic

measurements in feet, except where noted				Lanes
Approach Roadway Width: 40	Bridge Roadway Width: 40.0	Total Length: 148.4	On	2
Approach Pavement Width: 24	Deck Width: 45.4	Deck Area (sq ft): 6737	Under	2

	Lanes	ADT	ADT year	Traffic Pattern
On	2	22000	2004	ONE WAY TRAFFIC
Under	2			ONE WAY TRAFFIC

Capacity Load Rating

Inventory rating: HS28	Overburden depth (in): 0.0	Last rating date: 06-05-13	Controlling: INTERIOR DECK GIRDER Positive Moment
Operating rating: HS73	Deck surface material: CONCRETE	Re-rate for capacity (Y/N):	Control location: SPAN 1
Posting:	Re-rate notes:		

Hydraulic Classification

Scour Critical Code(113): (N) NO WATERWAY	Q100 (ft3/sec): 0	
High water elevation (ft): 0.0	Velocity (ft/sec): 0.0	Sufficieny #: 100.0

Span(s)

Span #	Material	Configuration	Depth (in)	Length (ft)	Main
1	PREST CONCRETE	DECK GIRDER	70	142.1	Υ

Expansion joint(s) Temperature: File: New:

Vertical Clearance Measurement file (ft)

Measurement file (ft)	File Date	Measurement new (ft)
17.1	29-Apr-2003	
	Measurement file (ft) 17.1	

page 3 Structure No.: **B-18-178**

Elements

	ients						Quantity in Co	ondition State	
hk	Element	Defect	Description	UOM	Total	1	2	3	4
X	12		Reinforced Concrete Deck	SF	6,737	6,733	4	0	0
_			Cracking (RC)	SF		133	4	0	0
		1130	Couple hairline diagonal cracks at deck ends		I.	1			
	8000		Wearing Surface (Bare)	SF	6,737	6,720	17	0	0
	8000								
			Crack (Wearing Surface)	SF		333	17	0	0
		3220	Few hairline diagonal cracks off deck ends. Few hairline transverse cracks through out de	ck.					
			Coated Reinforcing	SF	6,737	0	0	0	0
	8522		Diagonal cracking at the NW and SE corners	•					
			Prestressed Concrete Open Girder	LF	1,002	1,002	0	0	0
Х	109		7 Girders.	•					
			Reinforced Concrete Abutment	LF	150	142	8	0	0
Х	215		Behind MSE wall.	•					
			Cracking (RC)	LF		50	8	0	0
		1130	Few hairline vertical cracks under girders.	'	1				
			Reinforced Concrete Bridge Rail	LF	300	260	36	4	0
Х	331			•					
			Delamination - Spall - Patched Area	LF		0	0	4	0
		1080	Northeast corner back side 2" x 2" spall with	exposed	rusty reb	ar.			
			Cracking (RC)	LF		23	36	0	0
		1130	Few hairline vertical cracks.	•					
			Integral Wingwall	EA	4	2	2	0	0
Х	8400		Northwest is not a wing wall but a retaining wall No wing walls or retaining walls at southeast of	ıll. or northe	ast corner	s.			
			Wingwall Movement	EA		0	2	0	0
		8902	Southwest wingwall tip outward about 4". Northwest wingwall tipped inward about 2 1/4"	-	ange in 20	11-2013-20	15.		

page 4 Structure No.: **B-18-178**

Overtity in Condition State

Assessments

							Quantity in C	ondition State	
Chk	Element	Defect	Description	UOM	Total	1	2	3	4
			Drainage - Deck	EA	1	1	0	0	0
X	9004		At northeast cornerSurface AC flume off north a	oproach	slab.		•	•	
			Signs - Object Markers	EA	4	0	0	0	4
X	9030						•	•	
			Steel Diaphragm	EA	12	12	0	0	0
X	9167		Between girders.				•	•	
			Approach Roadway - Concrete (non-structural)	EA	2	0	2	0	0
x	9322		Northwest approach shoulder has settle about 2". Northwest approach shoulder has on large diagonal crack in it. Southeast and southwest approach shoulder has settled about 2 inches. Northeast shoulder has settle about 2 inches. Southwest shoulder about 12 ft south of deck joint has settle about 2 inches.						

NBI Ratings

	File	New
Deck	8	8
Superstructure	8	8
Substructure	8	8
Culvert	N	N
Channel	N	N
Waterway	N	N

Structure Specific Notes

Piece of MSE wall are being pushed outward.

Inspection Specific Notes

Monitor settlement behind SE wing at retaining wall, front face of wall displaced est. 2 inches with grass growing at seams. Also, sand has leaked from joints at bottom of NW corner.

Inspector Site-Specific Safety Considerations

Ctrustura	Inchastion	Procedures	

Walk around.

Special Requirements

	Chk	Comments
Traffic Control		
Access Equipment		
Other		

Construction History

Year	Work Performed	FOS id
2006	NEW STRUCTURE	1190-00-72

Maintenance Items History

Item Recommended by Status Status change Year completed

page 5 Structure No.: **B-18-178**

Maintenance Items

Item	Priority	Recommended by	Status	Status change		
Drainage - Repair Washouts / Erosion	HIGH	Frueh, Rick J (1003)	COMPLETE	07/26/15		
		, , ,				
Large washout behind southwest wing wall and abutment.						

STRUCTURE INVENTORY AND APPRAISAL FIELD REVIEW FORM

B-18-178 USH 53 SB over HASTINGS WAY

LOCATION

CITY-EAU CLAIRE(18221) (3) Municipality: 44°47'15.00"N (16) Latitiude(° ' "): (17) Longitude(° ' "): 91°27'10.60"W TRAFFIC SERVICE (28A) Lanes On: 2 (28B) Lanes Under: (102) Traffic Pattern On: -NO TRAFFIC X-ONE WAY TRAFFIC -TWO WAY TRAFFIC (102) Traffic Pattern Under: -NO TRAFFIC X-ONE WAY TRAFFIC -TWO WAY TRAFFIC (19) Detour Length(mi): 0 **GEOMETRY** (49) Structure Length(ft): 148.4 (50) Sidewalk Width(ft): Left: 0.0 Right: 0.0 (50) Curb Width(ft): (52) Culvert Barrel Length(ft): (34) Skew: Direction: -RIGHT FORWARD X-LEFT FORWARD Angle(°): 52 Cardinal Width Non-Cardinal Width (51) Bridge Roadway(ft): 40.0 40.0 (52) Deck(ft): 45.4 45.4 (32) Approach Roadway(ft): 40 0 Cardinal Under Clearance Non-Cardinal Under Clearance 75.0 17.0 (47) Minimum Horizontal(ft): (55) Minimum Right Lateral(ft): (55) Minimum Left Lateral(ft): 40.0 **RAILING APPRAISAL** (36A) Bridge Rail Adequacy: -SUB-STANDARD X-STANDARD -NOT APPLICABLE (36B) Transition Adequacy: (36C) Approach Guardrail Adequacy: -SUB-STANDARD X-STANDARD -NOT APPLICABLE -SUB-STANDARD X-STANDARD -NOT APPLICABLE (36D) Guardrail Termination Adequacy: -SUB-STANDARD X-STANDARD -NOT APPLICABLE **Outer Rail:** Right Type Left TYPE F (TWO SQUARE TUBES) - STEEL(8) TYPE F (3 SQUARE TUBES) - STEEL(65) TYPE F (4 SQUARE TUBES) - STEEL(72) TYPE M-STEEL 3 SQUARE TUBES(93) SLOPED FACE PARAPET LF(91) X SLOPED FACE PARAPET HF(92) VERTICAL FACE PARAPET TYPE A(74) TYPE W-THRIE BEAM(79) TYPE H ON VERTICAL PARAPET(80) TIMBER(38) OTHER(99) (Please specify) **Transition Type:** CONT GUARD RAIL NO APP GRDRL NO ATTACHMENT 22 MM(7/8") BOLT (Please enter quantity) 25 MM(1") BOLT (Please enter quantity) OTHER (Please specify) (01) ENERGY ABSORBING TERMINAL/EAT **Guardrail Termination Type:** (02) TURN DOWN (99) OTHER (Please specify) **ROADWAY ALIGNMENT APPRAISAL** (3) INTOLERABLE- Horizontal or Vertical curvature requires a substantial reduction in vehicle operating speed (72) Approach Alignment Appraisal:

(6) FAIR- Horizontal or Vertical curvature requires a very minor speed reduction

(8) GOOD- No speed reduction required



Bridge Asbestos Inspection Report

WisDOT Project ID: 1190-02-34

Structure Number: B-18-0178, B-18-0182
Structure Name: USH 53 over Hastings Way
City/County, City of Fay Clairs, Fay Clairs, Cay

City/County: City of Eau Claire, Eau Claire County Lat/Long Coordinates: 444715.0/ 912710.6, 444712.5/ 912706.7

TRC Project Number: 235777.0000.0000

Date Inspected: October 14, 2015

Inspected By/License Number: John Roelke, All-119523

Findings:

The inspection to identify and collect samples of potential asbestos-containing material (ACM) was completed following WisDOT standard sampling procedure for bridge inspections found in FDM 21-35-45.

None of the materials that were identified as potentially ACM and sampled tested positive for asbestos. The overlay on the bridges can proceed as planned. Standard Special Provision (STSP) 107-125 should be included in the specifications.

				Friable/	Quantity		
Sample	Sample	Sample	Analytical Results and	Non-friable or	of ACM		
Number	Description	Location	Method	No ACM	Material		
B-18-01	78						
1	Caulk	Abutment joint	PLM, non-detect	No ACM	0		
2	Caulk	Abutment joint	PLM, non-detect	No ACM			
3	Caulk	Abutment joint	PLM, non-detect	No ACM			
B-18-01	B-18-0182						
1	Caulk	Abutment joint	PLM, non-detect	No ACM	0		
2	Caulk	Abutment joint	PLM, non-detect	No ACM			
3	Caulk	Abutment joint	PLM, non-detect	No ACM			

If you have any questions, please contact me, at (608) 826-3628.

TRC Environmental Corporation

Danul Hank

Daniel Haak John Roelke

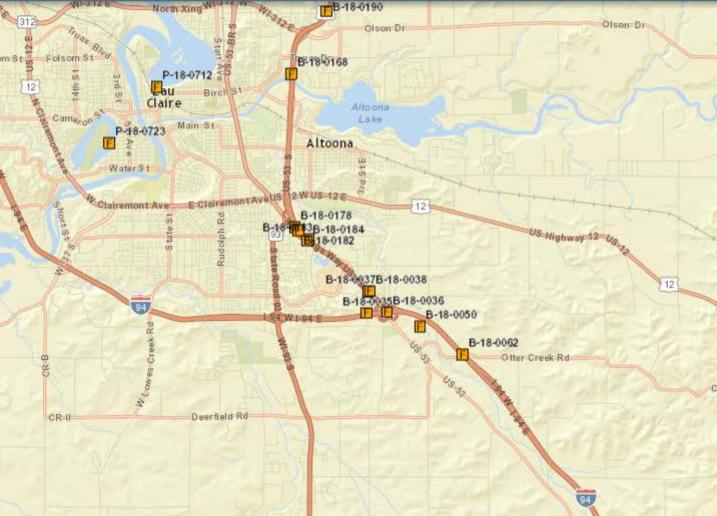
Project Manager Asbestos Inspector

Attachments: Location Map, Photos, and Laboratory Reports

Report Distribution:

Recipient	Electronic (PDF) Copy	Paper Copy
BTS-ESS sharlene.tebeest@dot.wi.gov	X (via email)	X
REC amy.adrihan@dot.wi.gov;	X (via email)	
nicholasA.schaff@dot.wi.gov		
Project Manager david.koepp@dot.wi.gov	X (via email)	
Other		

John Rollke W



B-18-0178











Caulk in abutment joint

B-18-0182











Caulk in abutment joint

Industrial Hygiene Laboratory 21 Griffin Road North Windsor, CT 06095 (860) 298-6308



BULK ASBESTOS ANALYSIS REPORT

CLIENT:

Wisconsin Department of Transportation

Lab Log #:

0047026

Project #:

235777.0000.0000

Date Received:

10/16/2015

Date Analyzed:

10/19/2015

Site:

DOT Bridge Inspection, B-18-178

POLARIZED LIGHT MICROSCOPY by EPA 600/R-93/116

Sample No.	Color	Homogenous	Multi- Layered	Layer No.	Other Matrix Materials	Asbestos %	Asbestos Type
B-18-178 (1)	Grey	Yes	No			ND	None
B-18-178 (2)	Grey	Yes	No			ND	None
B-18-178 (3)	Grey	Yes	No			ND	None

Reporting limit- asbestos present at 1%

ND - asbestos was not detected

Trace - asbestos was observed at level of less than 1%

NA/PS - Not Analyzed / Positive Stop

SNA- Sample Not Analyzed- See Chain of Custody for details

Note: Polarized-light microscopy is not consistently reliable in detecting asbestos in floor coverings and similar non-friable organically bound materials. In those cases, EPA recommends, and certain states (e.g. NY) require, that negative results be confirmed by quantitative transmission electron microscopy.

The Laboratory at TRC follows the EPA's Interim Method for the Determination of Asbestos in Bulk Insulation (1982), and the EPA recommended Method for the Determination of Asbestos in Bulk Building Materials (EPA/600/R-93/116), July 1993, R.L. Perkins and B.W. Harvey which utilizes polarized light microscopy (PLM). Our analysts have completed an accredited course in asbestos identification. TRC's Laboratory is accredited under the National Voluntary Laboratory Accreditation Program (NVLAP), for Bulk Asbestos Fiber Analysis, NVLAP Code 18/A01, effective through June 30, 2016. TRC is an American Industrial Hygiene Association (AIHA) accredited lab for PLM effective through October 1, 2016. Asbestos content is determined by visual estimate unless otherwise indicated. Quality Control is performed in-house on at least 10% of samples and the QC data related to the samples is available upon written request from the client.

This report shall not be reproduced, except in full, without the written approval of TRC. This report must not be used by the client to claim product endorsement by NVLAP or any agency of the U.S. Government. This report relates only to the items tested.

Analyzed by:

Welles Reviewed by: Kathleen Williamson, Laboratory Manager

Amanda Parkins, Approved Signatory

Date Issued

10/20/2015

Industrial Hygiene Laboratory 21 Griffin Road North Windsor, CT 06095 (860) 298-6308



BULK ASBESTOS ANALYSIS REPORT

CLIENT: Wisconsin Department of Transportation

Lab Log #:

0047032

Project #:

235777.0000.0000

Date Received:

10/16/2015

Date Analyzed:

10/19/2015

Site:

DOT Bridge Inspection, B-18-182

POLARIZED LIGHT MICROSCOPY by EPA 600/R-93/116

Sample No.	Color	Homogenous	Multi- Layered	Layer No.	Other Matrix Materials	Asbestos %	Asbestos Type
B-18-182 (1)	Grey	Yes	No			ND	None
B-18-182 (2)	Grey	Yes	No			ND	None
B-18-182 (3)	Grey	Yes	No			ND	None

Reporting limit- asbestos present at 1%

ND - asbestos was not detected

Trace - asbestos was observed at level of less than 1%

NA/PS - Not Analyzed / Positive Stop

SNA- Sample Not Analyzed- See Chain of Custody for details

Note: Polarized-light microscopy is not consistently reliable in detecting asbestos in floor coverings and similar non-friable organically bound materials. In those cases, EPA recommends, and certain states (e.g. NY) require, that negative results be confirmed by quantitative transmission electron microscopy.

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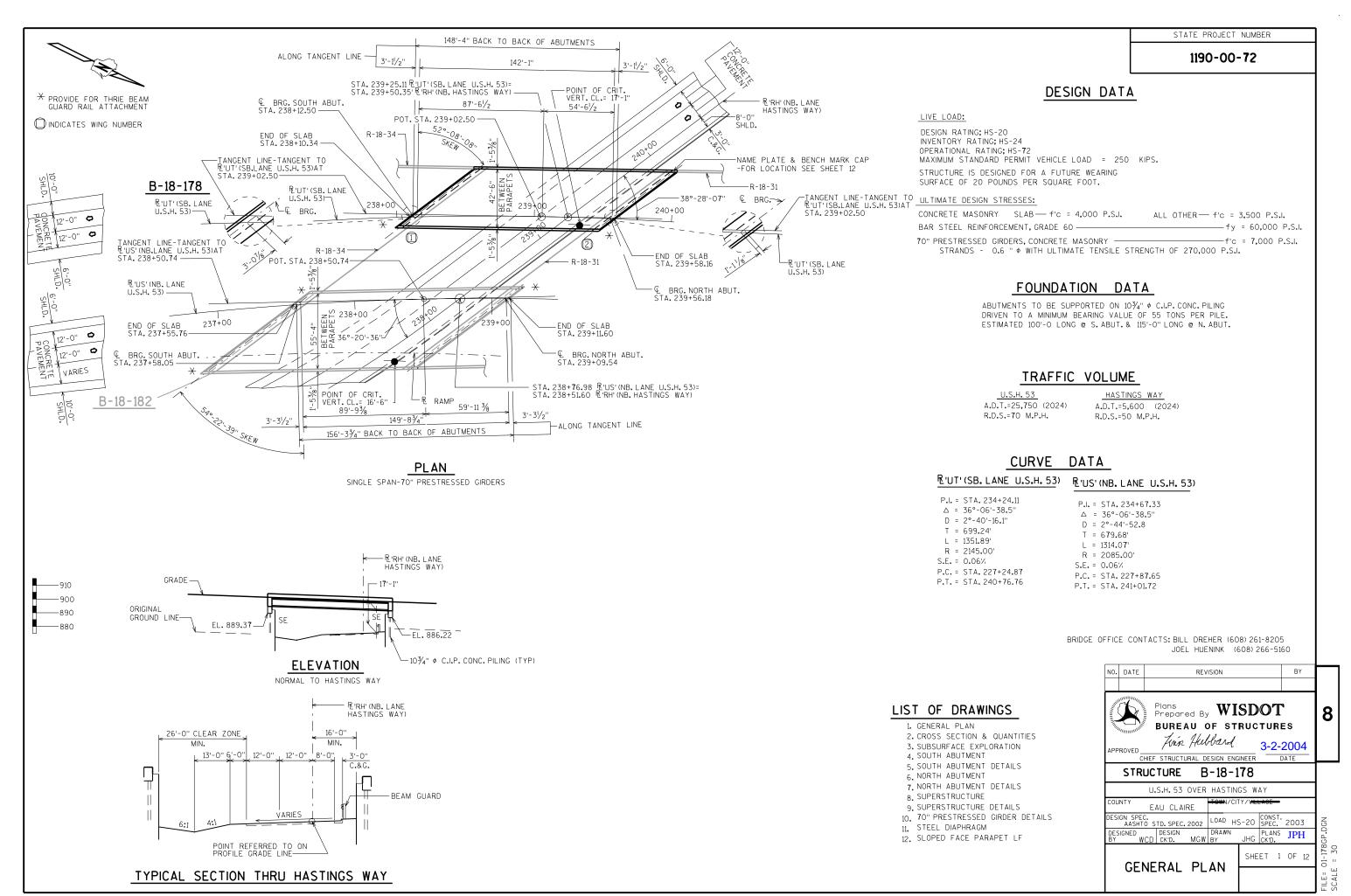
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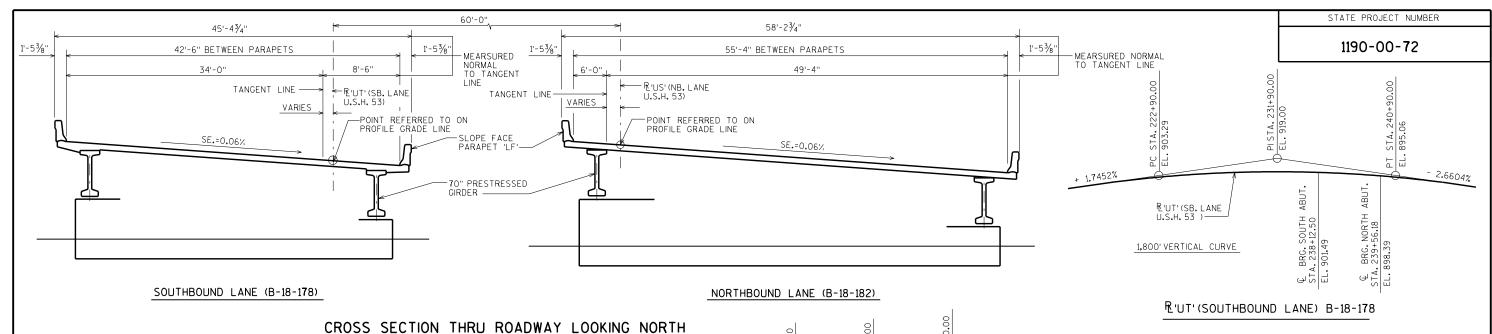
· Reviewed by:

Kathleen Williamson, Laboratory Manager

Amanda Parkins, Approved Signatory

Date Issued 10/21/2015





520' VERTICAL CURVE +0.9311% -R 'RH'(NB.LANE HASTINGS WAY) 200' VERTICAL CURVE

PROFILE GRADE LINE R'RH' (NB. HASTINGS WAY)

TOTAL ESTIMATED QUANTITIES

BID ITEMS	UNIT	SUPER.	SOUTH ABUT.	NORTH ABUT.	TOTALS	
EXCAVATION FOR STRUCTURES BRIDGES B-18-178	L.S.				1	
BACKFILL STRUCTURE	C.Y.		460	470	930	
CONCRETE MASONRY BRIDGES	C.Y.	321	52	53	426	
PROTECTIVE SURFACE TREATMENT	S.Y.	810			810	
PRESTRESSED GIRDER TYPE I 70-INCH	L.F.	1002			1002	
BAR STEEL REINFORCEMENT HS BRIDGES	LB.		3610	3610	7 220	
BAR STEEL REINFORCEMENT HS COATED BRIDGES	LB.	44210	320	320	44850	
BEARING PADS ELASTOMERIC NON-LAMINATED	EACH	14			14	
STEEL DIAPHRAGMS B-18-178	EACH	12			12	
PILING CIP CONCRETE DELIVERED AND DRIVEN 103/4-INCH	L.F.		1600	1840	3440	
RUBBERIZED MEMBRANE WATERPROOFING	S.Y.		16	16	32	
CONCRETE STAINING B-18-178	LS				1	
	S.Y.		15	15	30	
ANCHOR ASSEMBLIES FOR STEEL PLATE BEAM GUARD	EACH	2			2	
QMP CONCRETE STRUCTURES 5-CYLINDER	C.Y.	321	52	53	426	
INCENTIVE STRENGTH CONCRETE STRUCTURES	DOL.	3210	520	530	4260	
NON-BID ITEMS						
FILLER	SIZE				1/2" & 3/4	

GENERAL NOTES

DRAWINGS SHALL NOT BE SCALED.

BAR STEEL REINFORCEMENT SHALL BE EMBEDDED 2" CLEAR UNLESS OTHERWISE SHOWN OR NOTED.

ELASTOMERIC BEARING PADS NEED NOT BE INDIVIDUALLY MOLDED PROVIDED THE CUT EDGES ARE SMOOTH AND TRUE.

AT THE BACKFACE OF ABUTMENT ALL VOLUME WHICH CANNOT BE PLACED BEFORE ABUTMENT CONSTRUCTION AND IS NOT OCCUPIED BY THE NEW STRUCTURE SHALL BE BACKFILLED WITH STRUCTURE BACKFILL.

THE FIRST OR FIRST TWO DIGITS OF THE BAR MARK SIGNIFIES THE BAR SIZE.

THE GRADATION OF THE STRUCTURE BACKFILL SHALL MEET THE REQUIREMENTS OF SECTION 209.2.2 OF THE STANDARD SPECIFICATIONS FOR GRADE 1 MATERIAL.

THE FOLLOWING COMPONENTS SHALL BE STAINED LIGHT GRAY (FEDERAL STANDARD COLOR NO. 36622) IN ACCORDANCE WITH THE SPECIAL PROVISIONS:

- THE UNDERSIDE OF THE DECK OVERHANGS TO THE EXTERIOR GIRDERS.

- THE EDGES OF THE DECK.

- THE BACK FACES OF THE PARAPETS.

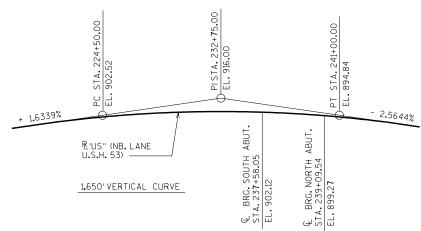
- ALL EXPOSED VERTICAL SURFACES OF THE ABUTMENTS TO 1'-0" BELOW FINISHED GROUNDLINE.

- ALL EXPOSED VERTICAL SURFACES OF THE ABUTMENT DIAPHRAGMS EXCEPT BETWEEN GIRDERS.

THE FOLLOWING COMPONENTS SHALL BE STAINED DARK GRAY (FEDERAL STANDARD COLOR NO. 26293) IN ACCORDANCE WITH THE SPECIAL PROVISIONS:

- THE OUTSIDE FACE AND UNDERSIDE OF THE BOTTOM FLANGE OF THE EXTERIOR GIRDERS.

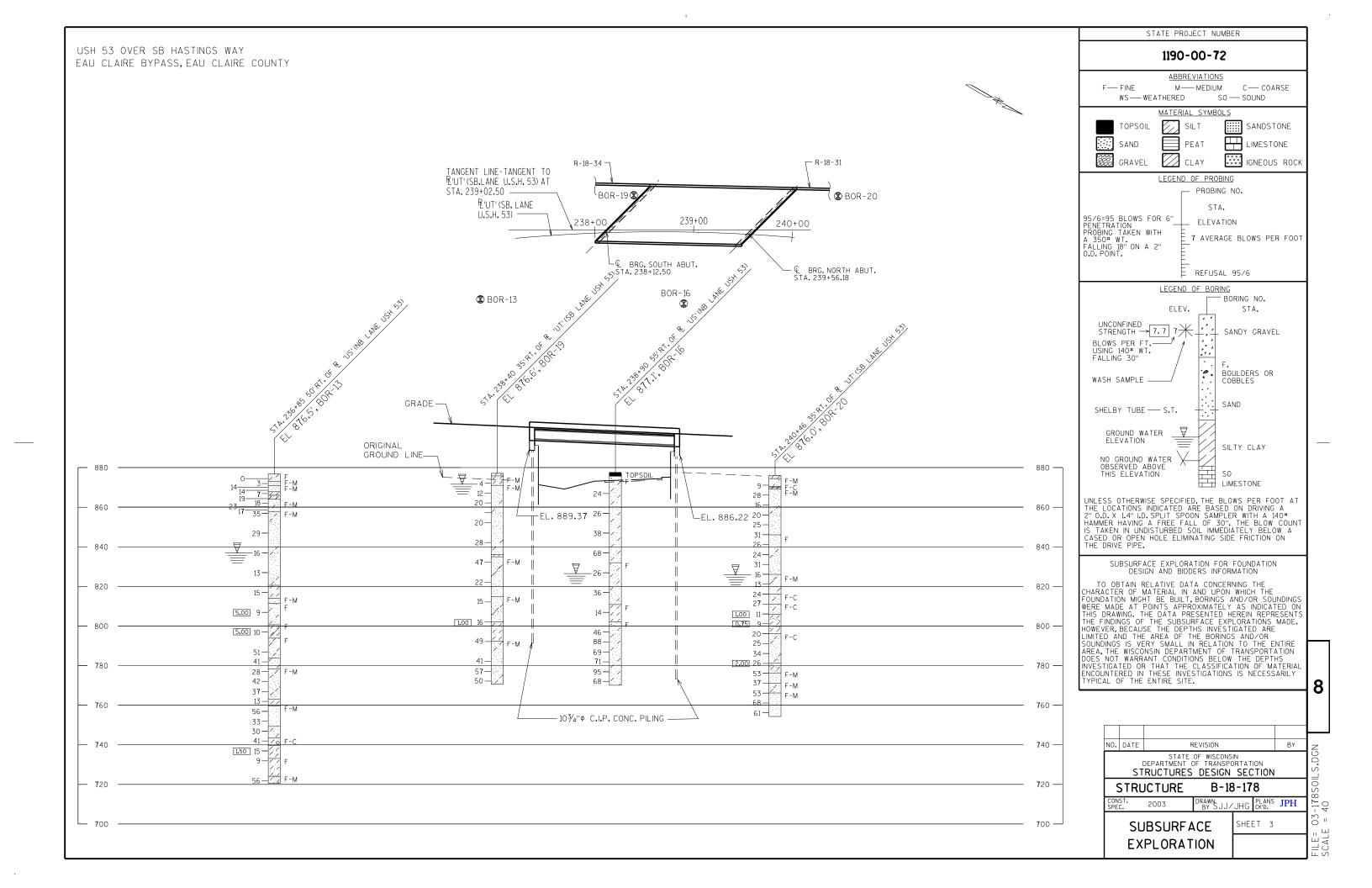
PROTECTIVE SURFACE TREATMENT SHALL BE APPLIED TO THE TOP SURFACE OF THE DECK AND THE INSIDE FACE AND TOP OF PARAPETS.

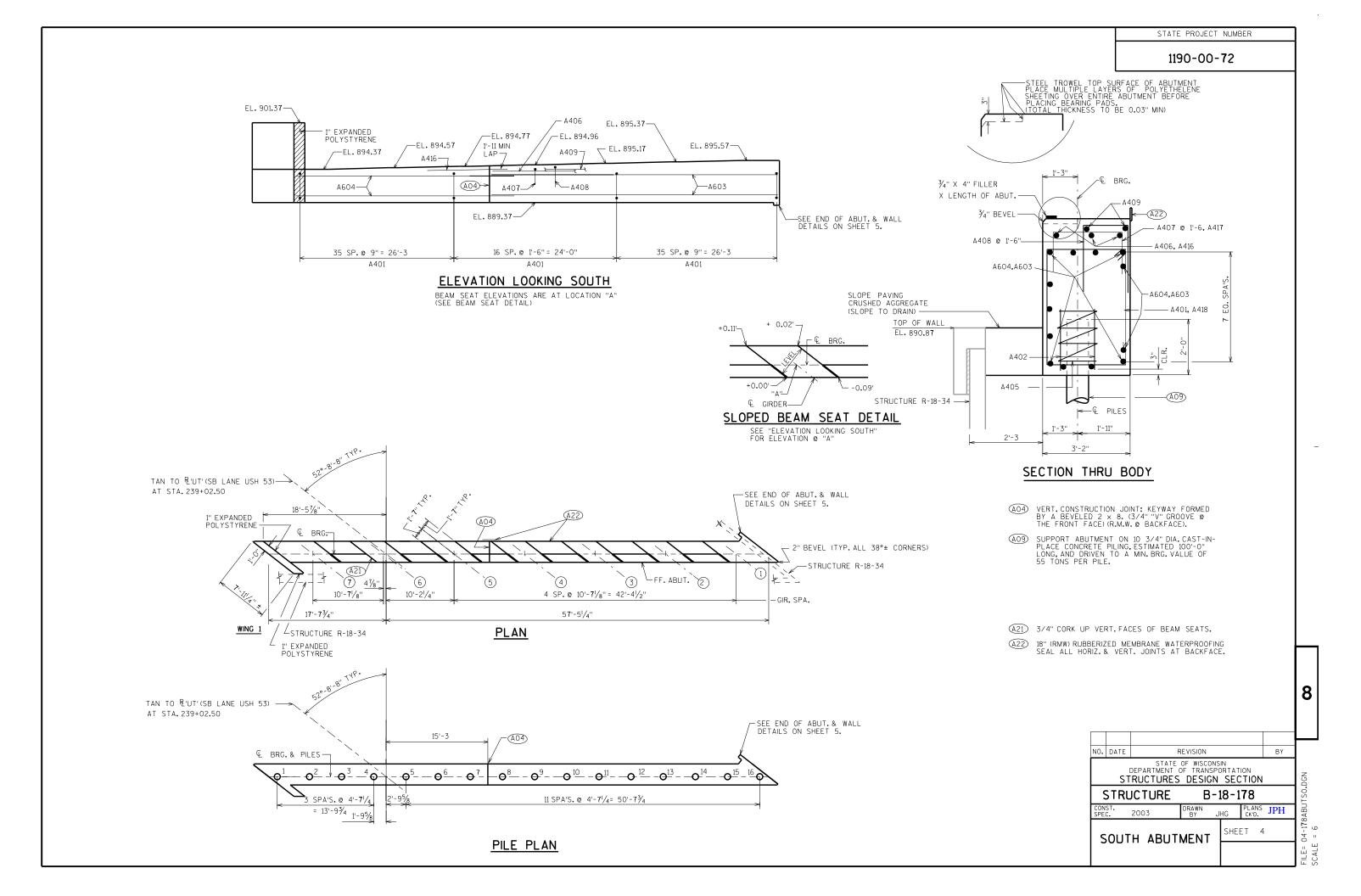


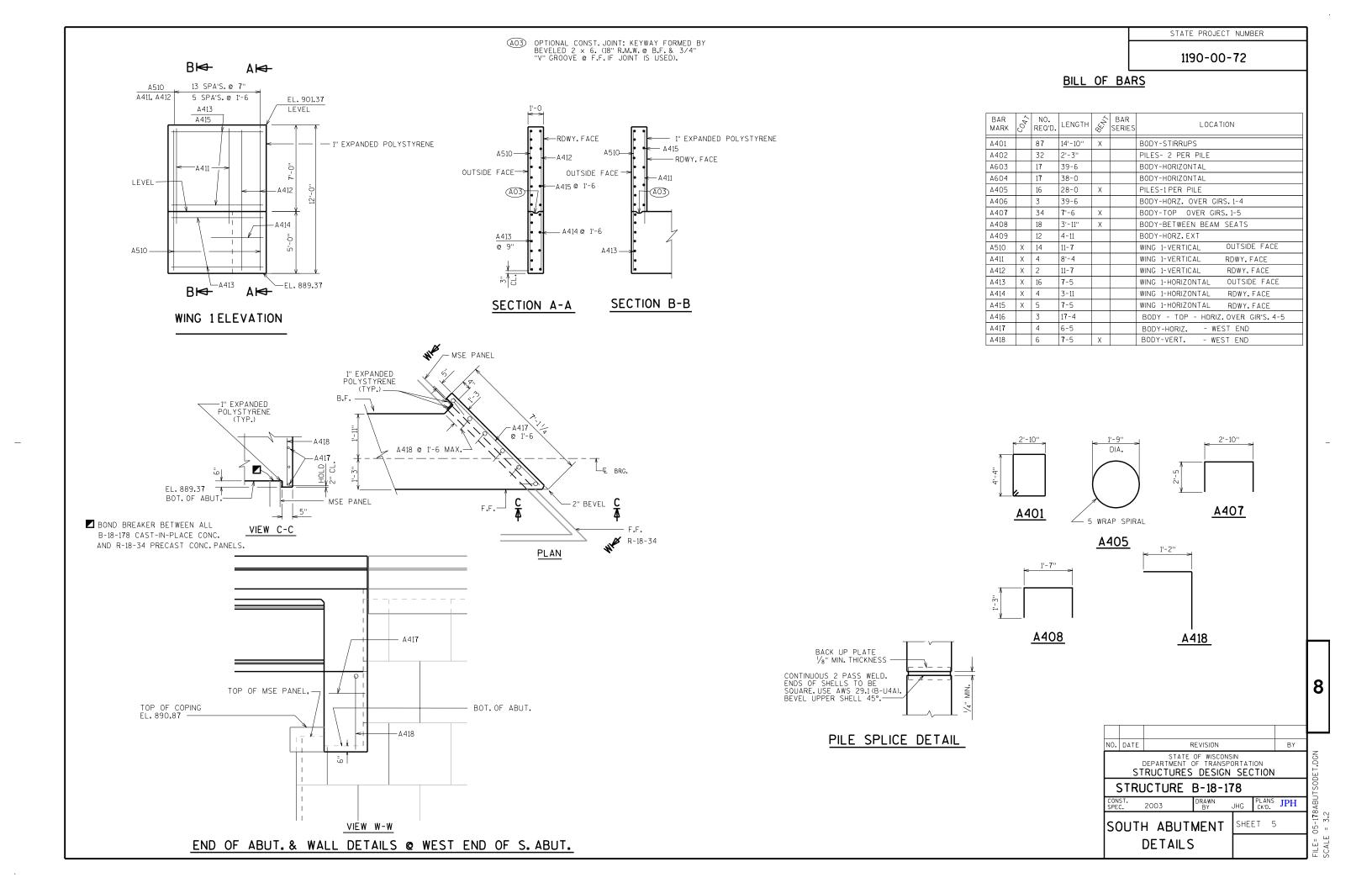
PROFILE GRADE LINE U.S.H. 53

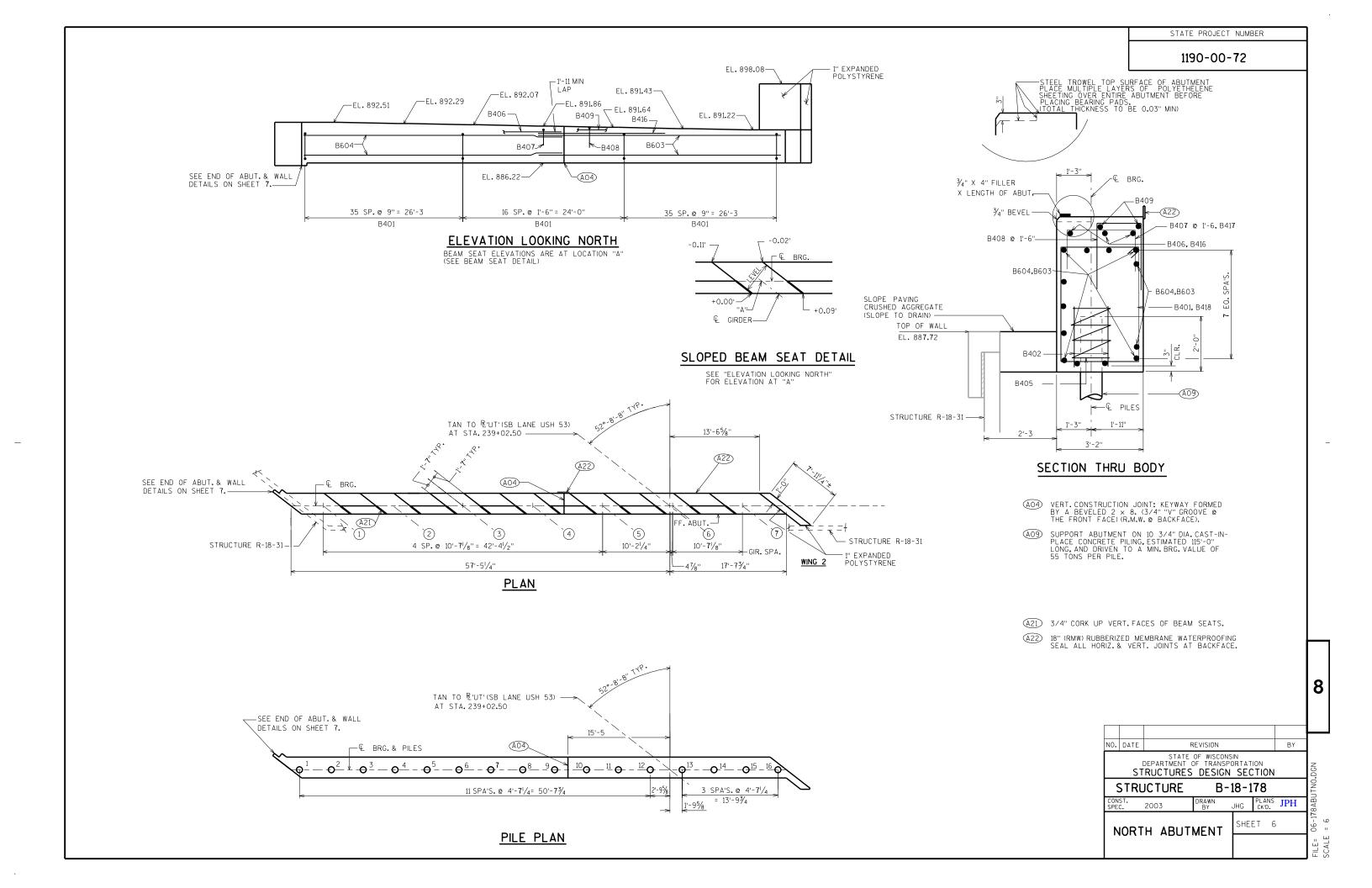
R'US' (NORTHBOUND LANE) B-18-182

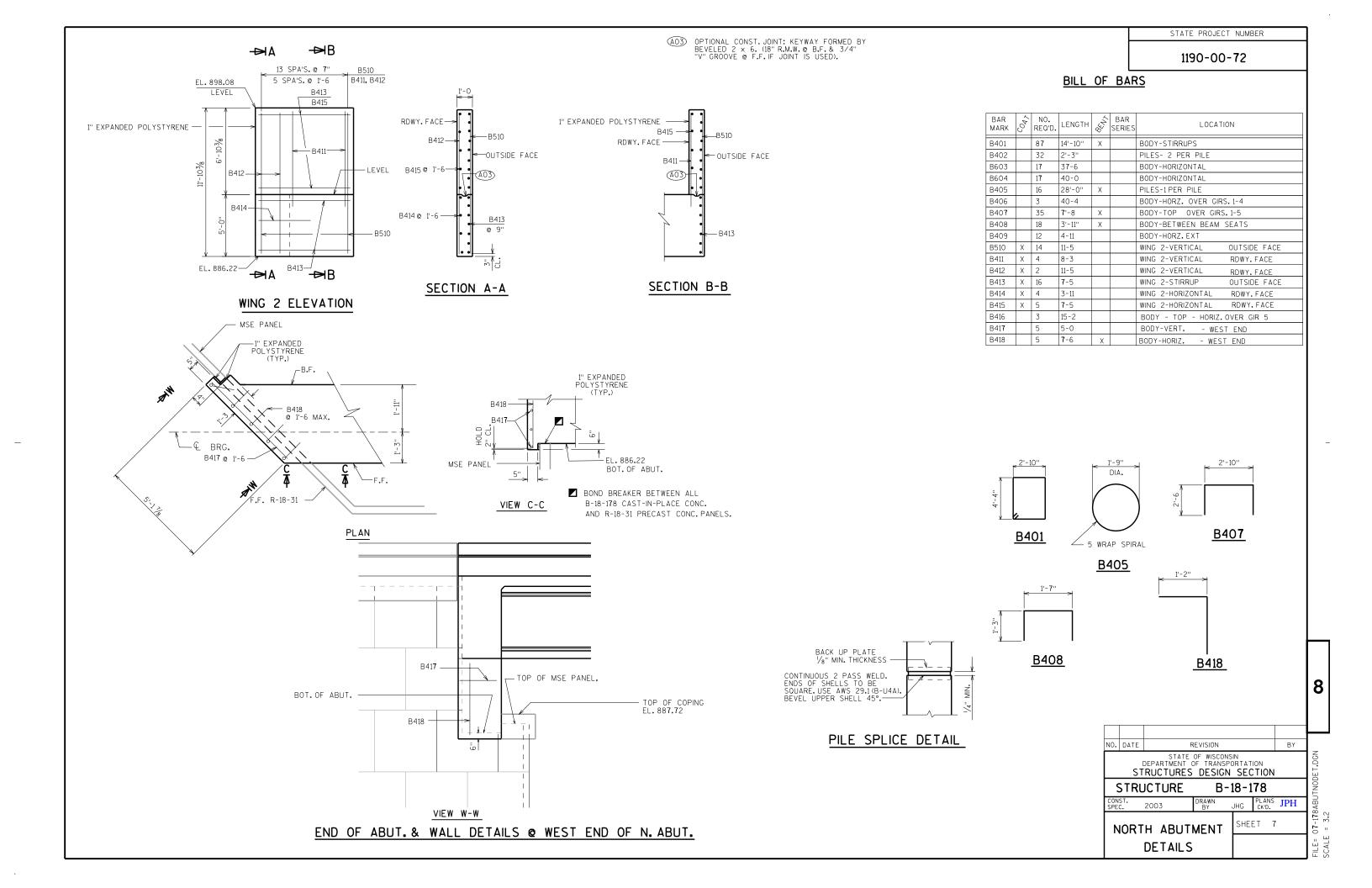
NO. DATE BY REVISION STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN SECTION STRUCTURE B-18-178 JHG PLANS JPH SHEET 2 CROSS SECTION & QUANTITIES

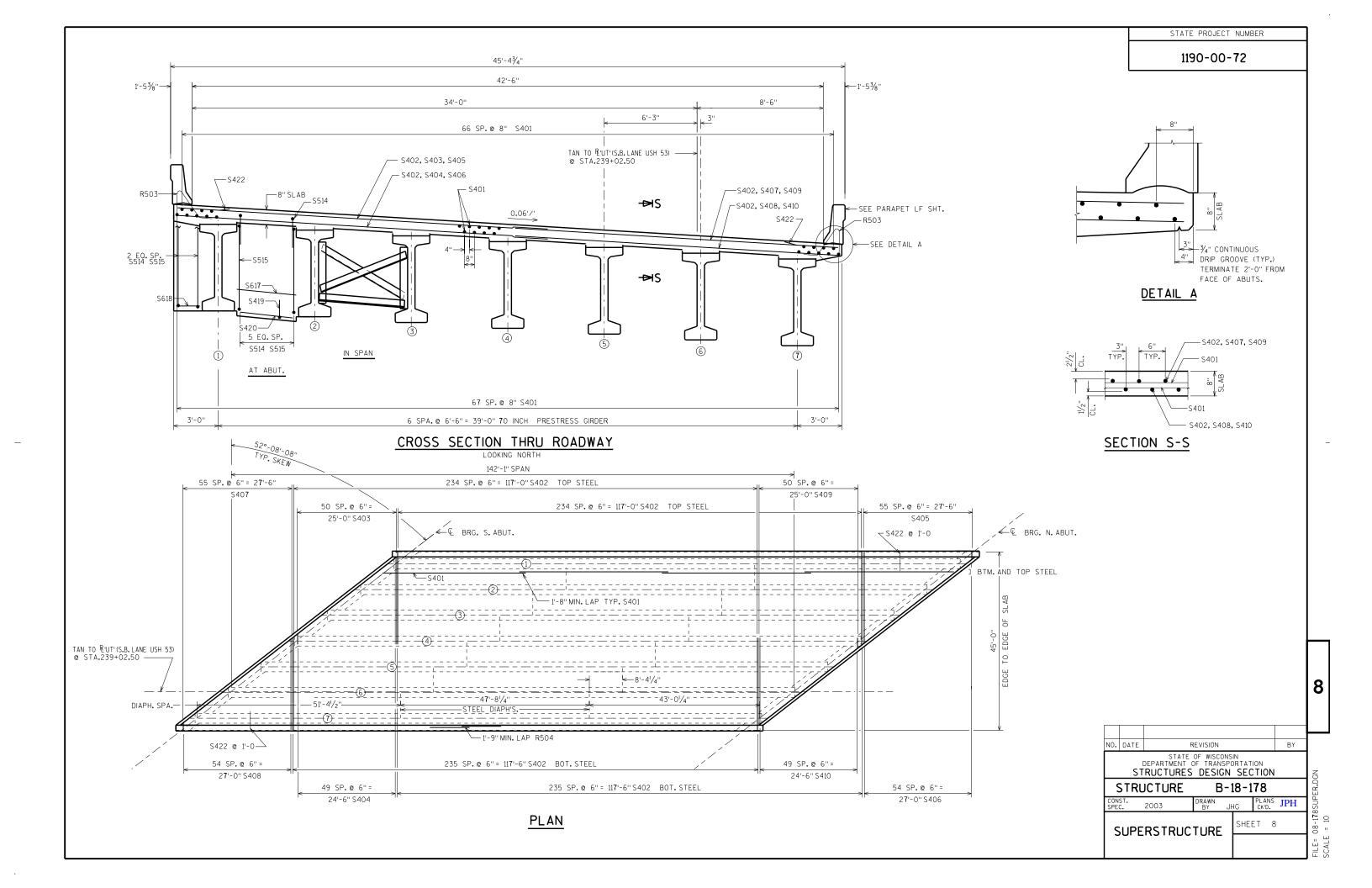


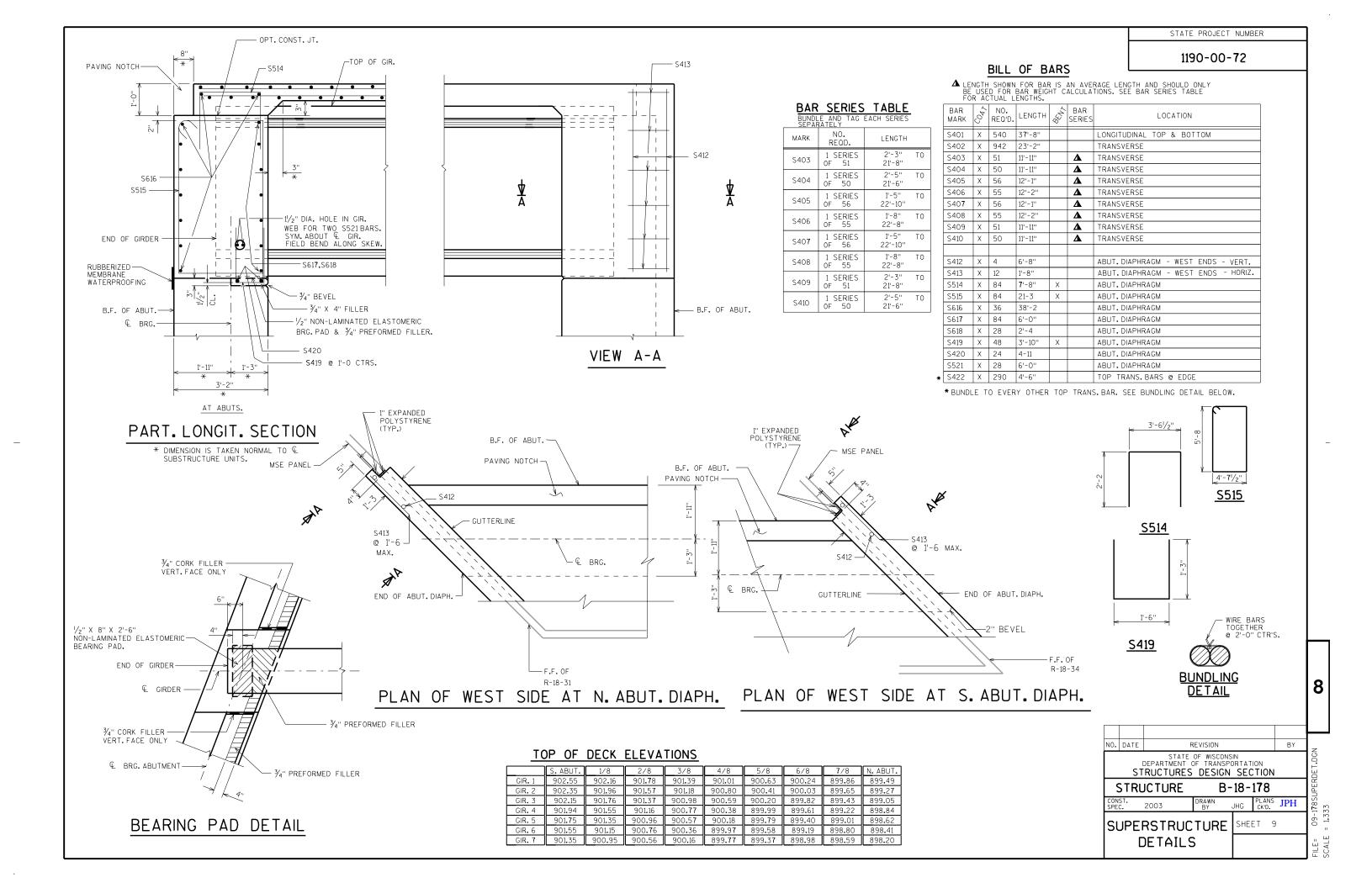












1190-00-72

GIRDER NOTES

TOP OF GIRDER TO BE ROUGH FLOATED AND BROOMED TRANSVERSELY, EXCEPT THE OUTSIDE 2" OF GIRDER, WHICH SHALL BE TROWEL FINISHED.

THE GIRDERS SHALL BE PROVIDED WITH A SUITABLE LIFTING DEVICE FOR HANDLING AND ERECTING THE GIRDERS. PRESTRESSING STRANDS SHALL BE 0.6" \$\phi\$ - 7 WIRE LOW-RELAXATION STRANDS WITH AN ULTIMATE STRENGTH OF 270,000 PSI AND SHALL BE FLUSH WITH THE ENDS OF

BEND EACH END OF #4 STIRRUPS 41/2", #7 STIRRUP 12".

FOR DIAPHRAGM INSERT & CONNECTION DETAILS SEE "STEEL DIAPHRAGM" SHEET.

ON MULTIPLE SPAN STRUCTURES, SET THE END BLOCK LENGTHS OF GIRDERS RESTING ON THE SAME PIER TO ± 2", ON SIMPLE SPANS, SET THE END BLOCK LENGTH ON BOTH GIRDER ENDS TO \pm 2".

IF THE CONTRACTOR USES BOTTOM FLANGE TO SUPPORT CONSTRUCTION FORMS, THE CONTRACTOR SHALL SUBMIT FALSEWORK PLANS FOR APPROVAL OF THE STRUCTURES DESIGN SECTION.

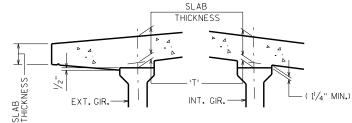
ALL GIRDERS SHALL BE CAST FULL LENGTH AS SHOWN.

SPACING SHOWN FOR #4 STIRRUPS IS FOR GRADE 60 REINFORCEMENT.IF THE FABRICATOR WANTS TO BUILD A BAR STEEL CAGE BY WELDING LONGITUDINAL REINFORCEMENT TO THE #4 STIRRUPS, 2 OPTIONS ARE AVAILABLE:

- USE ASTM A706, GRADE 60 REINFORCEMENT AND THE STIRRUP SPACING AS SHOWN ON THE PLANS.
- 2. USE ASTM A615, GRADE 40 REINFORCEMENT AND A MODIFIED STIRRUP SPACING SUBMITTED TO AND APPROVED BY THE STRUCTURES DEVELOPMENT SECTION.

AN ALTERNATE EQUIVALENT OF WELDED WIRE FABRIC (WWF) MAY BE SUBSTITUTED FOR THE STIRRUP REINFORCEMENT SHOWN, UPON APPROVAL OF THE STRUCTURES DEVELOPMENT

WELDED WIRE FABRIC SHALL CONFORM TO THE REQUIREMENTS



SLAB HAUNCH DETAIL

IF $1^{\prime}\!\!/\!\!/^{"}$ MINIMUM HAUNCH HEIGHT AT EDGE OF GIRDER CANNOT BE MAINTAINED, THE GRADE LINE MAY BE REVISED BY THE ENGINEER AT THE OPTION OF THE CONTRACTOR, THE PLAN SLAB THICKNESS SHALL BE HELD, NOTIFY BRIDGE

TO DETERMINE 'T', ELEV. OF TOP OF GIR'S. AT $\frac{Q}{8}$ OF SUBSTRUCTURE UNITS & AT $\frac{1}{8}$ POINTS OF EACH SPAN SHALL BE TAKEN. THEN FOLLOW THIS PROCESS:

TOP OF DECK ELEV. AT FINAL GRADE

→ MII	NIMUM CYL	INDER	STRE	NGTH	OF CC	NCRETE	@ TIM	E OF TRA	NSFER OF	PRESTE	RESS F	ORCE.						т
								GIRDE	R DA	TΑ]	DATE
DEAD LOAD DEFL. (IN.) CONC. DIA. OF DRAPED PATTERN												UNDRAPED	PATTERN	THE.	DAIL			
SPAN	GIRDER LENGTH	17	Τ.,	7,	1.7	STRGTH.		STRAND	TOTAL	f'ci	(IN.)			TOTAL	f'ci	1	_	
	"L"	1/8	1/4	3/8	1/2	f'c (P.S.I.)	"P"		NO.OF STRANDS	(P.S.I.)	"A"	"B" MIN.	"B" MAX.	"C"	NO.OF STRANDS	(P.S.I.)	—	<u> </u>
1	143'-1''	1.03	1.77	2.21	2.36	7000	Δ	0.60	42	6745	65.00		23.00	5.00	0	0	1 5	STR
																	CON: SPE	
																	7	0"

— 2" X 1" BEVEL

HOLD DOWN POINT

DRAPED STRANDS-

GIRDER

SYM. ABOUT MIDSPAN

GIRDER

3'-6" MAX. 2'-0" MIN.

3/4" X 3/4" BEVEL

GIRDER LENGTH = "L"

SIDE VIEW & TYPICAL SECTION IN SPAN

1 /4" CL

MIN.

101 SP. @ 1'-4" = 134'-8"

#4 BAR

-#4 STIRRUPS

***7** STIRRUPS

#4 BARS

IN PAIRS

-9 #3 BARS-

(EPOXY COATED)

#3 BARS @ 6" (EPOXY COATED)

17'-0" 🖎

/ 5 @ 4¹/2¹

___#4 BΔR

28-1230

DRAPED PATTERN

3'-2 ½" 📵

TOP FLG.

41/8"

36-1582

21/2" 1 2 @ 41/2"

DIA. EACH OF

1/2". | 7P.

20-8**7**9

41/2"

— DRAPE ALL STRANDS ON THESE TWO LINES TOTAL NO. OF STRANDS ∠̃total initial PRESTRESS FORCE IN KIPS 11 SPA. @ 2'

TYP STRAND PATTERN

STRUCTURES DESIGN SECTION RUCTURE B-18-178 PLANS JPH SHEET 10 **PRESTRESSED** GIRDER DETAILS

REVISION

BY

BOT. OF GIRDER─ <-- 1/₄ PT. (0.25 L) 2'-3" MAX. DRAPED STRAND PROFILE OFFICE FOR HAUNCH HEIGHTS OVER 4". DEAD LOAD DEFL ADJUST THIS DIMENSION ON BOTH ENDS ACCORDING TO NOTE TOP OF GIRDER AFTER TOP OF GIRDER BEFORE SLAB IS POURED. 22-96**7** SLAB, SIDEWALKS AND 30-1318 38-1670 PARAPET ARE POURED. TOP VIEW OF GIRDER (A) TOP OF GIRDER ELEVATION + DEAD LOAD DEFLECTION ***** - SLAB THICKNESS = HAUNCH HEIGHT 'T' ALL PATTERNS ARE SYM. ABOUT \triangle P=8 $\frac{1}{2}$ " FOR END 23'-0" OF GIRDERS NOTE: AN AVERAGE HAUNCH ('T') OF 4" WAS USED IN THE QUANTITY "CONCRETE MASONRY BRIDGES". DEAD LOAD DEFLECTION DIAGRAM 24-1055 32-1406 40-1**7**58 ⚠ P=7" FOR MIDDLE 97'-1" OF GIRDERS STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION 26-1142 34-1494 42-1845

(A) DETAIL TYP. AT EACH END.

(B) 4, #6 BARS, FULL LENGTH, 2'-11" MIN. LAP

i i i

#4 BAR EPOXY COATED. PLACE @ STIRRUP SPACING. EMBED INTO GIRDER 1'-3".

• • • • • •

3'-2 ½" 📵

NO BEVEL

@ 1'-0" SPA.

-#4 STIRRUPS

#4 BARS X 1'-11" LG. (USE STIRRUP SPA. AND PLACE BTWN. ENDS OF GIRDER EXCEPT AT DETAIL "A".)

#4 BAR AT TOP & BOTTOM OF GIRDER

--#4 BARS @ 1'-0"

