

# Intersection Control Evaluation Report

## CTH UU Interchange

Project ID 1440-15-01  
Fond du Lac - Plymouth Road  
(CTH K - CTH W)  
STH 23  
Fond du Lac County

Prepared by:



5950 Seminole Centre Court Suite 200  
Madison, WI 53711

**January 2009**

# TABLE OF CONTENTS

	<b>PAGE</b>
Safety .....	1
Traffic Volumes & Operational Analysis .....	2
Construction Costs .....	3
Right-of-Way .....	4
Practical Feasibility.....	4
Operations & Maintenance Costs .....	5
Environmental.....	6
Ped's/Bikes .....	6
Recommendation .....	6

## **Appendix**

Appendix A – 2035 Traffic Volumes and Distribution Percentages .....	8
Appendix B – 2035 Warrant Analysis Check (8-Hour Warrant – Peak Hour Only) .....	11
Appendix C – TWSC Operational Analysis Results .....	13
Appendix D – Roundabout Operational Analysis Results.....	18
Appendix E – Cost Estimates .....	23
Appendix F – Sketch of Alternatives.....	25
Appendix G – STH 23 Corridor Crash Report .....	29

# Intersection Control Evaluation

## Factors to Analyze (Intersection of STH 23 & CTH UU)

	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> <b>2-WAY STOP (TWSC)</b> <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input checked="" type="checkbox"/> <b>ROUNDABOUT</b> <input type="checkbox"/> 4-WAY STOP <input type="checkbox"/> 2-WAY STOP (TWSC) <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>GENERAL COMMENTS</b>
<b>SAFETY</b>	<p>The reconstruction of the existing at-grade intersection with a diamond interchange and two two-way stop controlled (TWSC) intersections will likely result in a reduction in the existing crash rate. This will be due to two factors:</p> <ol style="list-style-type: none"> <li>1. High volume and fast moving mainline STH 23 traffic will be removed from the intersections entirely.</li> <li>2. Traffic turning from STH 23 onto CTH UU will be divided between two intersections.</li> </ol> <p>Reconstructing the intersection to operate under TWSC will not reduce the number of theoretical conflict points (32), nor will it eliminate the opportunity for “angle” type crashes as vehicles make left turning maneuvers across oncoming traffic.</p>	<p>The reconstruction of the existing at-grade intersection with a diamond interchange and two roundabout intersections will likely result in a reduction in the existing crash rate. This will be due to the same two factors as noted in the TWSC alternative.</p> <p>When compared to a TWSC, roundabouts reduce the number of conflict points for an intersection to 8 (4 diverge and 4 merge, 0 crossing) and greatly lessen the opportunity for “angle” type crashes. Recent studies show that conversion to roundabout control has reduced 39% of the total crashes and 76% of the injury crashes.</p> <p>Roundabout crashes are also typically less severe than those occurring at other types of intersections due to lower vehicle operating speeds.</p> <p>The safety of the mainline will also benefit from the installation of roundabouts. The increased capacity of roundabouts (when compared to TWSC) intersections will decrease the likelihood of traffic interacting with the mainline or rear-end crashes resulting from long queues.</p>	<p>The current crash rate for the existing at-grade intersection (based on crash data between 2001-2005) is 0.46 crashes per million entering vehicles.</p> <p>An intersection crash rate of 1.5 or higher indicates a level of concern that needs to be addressed.</p> <p>In general, both alternatives are likely to provide a safer intersection than the current conditions. Refer to the Crash Report in Appendix G for additional crash related information on the existing intersection. This Crash Report was completed for the entire STH 23 corridor in 2007.</p>

# Intersection Control Evaluation

## Factors to Analyze (Intersection of STH 23 & CTH UU)

	ALTERNATIVE CONTROL	ALTERNATIVE CONTROL	GENERAL COMMENTS																																																																																																						
	<input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> <b>2-WAY STOP (TWSC)</b> <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input checked="" type="checkbox"/> <b>ROUNDABOUT</b> <input type="checkbox"/> 4-WAY STOP <input type="checkbox"/> 2-WAY STOP (TWSC) <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP																																																																																																							
TRAFFIC VOLUMES & OPERATIONAL ANALYSIS	<p>Traffic signal warrants were not met for either of the ramp intersections with CTH UU. It is anticipated that TWSC with CTH UU free flowing is the likely best choice of intersection operation for this alternative. Refer to Appendix B for traffic signal warrant analysis results.</p> <p>Under TWSC, both intersections will operate at a LOS A under both AM and PM design year peak hours.</p> <p style="text-align: center;"><b>CTH UU &amp; EB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2">Delay Per Vehicle/LOS (Seconds)</th> </tr> <tr> <th>AM Peak</th> <th>PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td>0.0/A</td> <td>0.0/A</td> </tr> <tr> <td>SB Approach</td> <td>1.7/A</td> <td>1.7/A</td> </tr> <tr> <td>EB Approach</td> <td>8.8/A</td> <td>8.7/A</td> </tr> <tr> <td>WB Approach</td> <td>NA</td> <td>NA</td> </tr> </tbody> </table> <p style="text-align: center;"><b>CTH UU &amp; EB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2">Queue Length (Feet)</th> </tr> <tr> <th>AM Peak</th> <th>PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td>0'</td> <td>0'</td> </tr> <tr> <td>SB Approach</td> <td>0'</td> <td>0'</td> </tr> <tr> <td>EB Approach</td> <td>1'</td> <td>3'</td> </tr> <tr> <td>WB Approach</td> <td>NA</td> <td>NA</td> </tr> </tbody> </table> <p style="text-align: center;"><b>CTH UU &amp; WB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2">Delay Per Vehicle/LOS (Seconds)</th> </tr> <tr> <th>AM Peak</th> <th>PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td>6.2/A</td> <td>3.3/A</td> </tr> <tr> <td>SB Approach</td> <td>0.0/A</td> <td>0.0/A</td> </tr> <tr> <td>EB Approach</td> <td>NA</td> <td>NA</td> </tr> <tr> <td>WB Approach</td> <td>9.1/A</td> <td>8.9/A</td> </tr> </tbody> </table>		Delay Per Vehicle/LOS (Seconds)		AM Peak	PM Peak	NB Approach	0.0/A	0.0/A	SB Approach	1.7/A	1.7/A	EB Approach	8.8/A	8.7/A	WB Approach	NA	NA		Queue Length (Feet)		AM Peak	PM Peak	NB Approach	0'	0'	SB Approach	0'	0'	EB Approach	1'	3'	WB Approach	NA	NA		Delay Per Vehicle/LOS (Seconds)		AM Peak	PM Peak	NB Approach	6.2/A	3.3/A	SB Approach	0.0/A	0.0/A	EB Approach	NA	NA	WB Approach	9.1/A	8.9/A	<p>Analysis of the traffic shows that single lane roundabouts will operate acceptably for both ramp intersections with CTH UU. Both roundabouts will have an inscribed circular diameter of 136-feet.</p> <p>With a single lane roundabout, both intersections operate at LOS A under both AM and PM design year peak hours.</p> <p style="text-align: center;"><b>CTH UU &amp; EB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2">Delay Per Vehicle/LOS (Seconds)</th> </tr> <tr> <th>AM Peak</th> <th>PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td>3.1/A</td> <td>3.0/A</td> </tr> <tr> <td>SB Approach</td> <td>2.9/A</td> <td>2.9/A</td> </tr> <tr> <td>EB Approach</td> <td>2.9/A</td> <td>3.0/A</td> </tr> <tr> <td>WB Approach</td> <td>NA</td> <td>NA</td> </tr> </tbody> </table> <p style="text-align: center;"><b>CTH UU &amp; EB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2">Queue Length (Feet)</th> </tr> <tr> <th>AM Peak</th> <th>PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td>0'</td> <td>0'</td> </tr> <tr> <td>SB Approach</td> <td>0'</td> <td>0'</td> </tr> <tr> <td>EB Approach</td> <td>0'</td> <td>0'</td> </tr> <tr> <td>WB Approach</td> <td>NA</td> <td>NA</td> </tr> </tbody> </table> <p style="text-align: center;"><b>CTH UU &amp; WB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2">Delay Per Vehicle/LOS (Seconds)</th> </tr> <tr> <th>AM Peak</th> <th>PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td>3.1/A</td> <td>3.0/A</td> </tr> <tr> <td>SB Approach</td> <td>3.0/A</td> <td>3.0/A</td> </tr> <tr> <td>EB Approach</td> <td>NA</td> <td>NA</td> </tr> <tr> <td>WB Approach</td> <td>3.0/A</td> <td>2.9/A</td> </tr> </tbody> </table>		Delay Per Vehicle/LOS (Seconds)		AM Peak	PM Peak	NB Approach	3.1/A	3.0/A	SB Approach	2.9/A	2.9/A	EB Approach	2.9/A	3.0/A	WB Approach	NA	NA		Queue Length (Feet)		AM Peak	PM Peak	NB Approach	0'	0'	SB Approach	0'	0'	EB Approach	0'	0'	WB Approach	NA	NA		Delay Per Vehicle/LOS (Seconds)		AM Peak	PM Peak	NB Approach	3.1/A	3.0/A	SB Approach	3.0/A	3.0/A	EB Approach	NA	NA	WB Approach	3.0/A	2.9/A	<p><u>Traffic Patterns</u> The design year (2035) peak hour turning movement traffic data for the intersections is shown in Appendix A.</p> <p>Vehicles using the two CTH UU intersections are generally shown to follow the pattern of commuter traffic entering STH 23 westbound towards Fond du Lac during the AM peak hour; and exiting STH 23 to return to the origination point during the PM peak hour. The percentage of traffic using CTH UU as a through movement and not entering or exiting STH 23 is higher by nearly 50% during the AM peak hour compared to the PM peak hour. The peak hour traffic distribution percentages are shown in Appendix A.</p> <p>Traffic volumes northbound on CTH UU are higher during the AM peak hour by nearly 50%. Traffic volumes southbound on CTH UU are higher during the PM peak hour between 20% and 50%. Traffic volumes on the eastbound off ramp are 50% higher during the PM peak hour. Traffic volumes on the westbound off ramp remain relatively constant between either peak hour. There are no significant seasonal traffic volume fluctuations along STH 23 according to the continuous count recorder data sources.</p> <p><u>Operational Analysis</u> Both alternatives exceed the threshold for an intersection to be considered operationally acceptable. Neither alternative is shown to have access blockage resulting from queuing.</p> <p>A roundabout can have slightly higher delay when compared to a high functioning TWSC intersection.</p>
	Delay Per Vehicle/LOS (Seconds)																																																																																																								
	AM Peak	PM Peak																																																																																																							
NB Approach	0.0/A	0.0/A																																																																																																							
SB Approach	1.7/A	1.7/A																																																																																																							
EB Approach	8.8/A	8.7/A																																																																																																							
WB Approach	NA	NA																																																																																																							
	Queue Length (Feet)																																																																																																								
	AM Peak	PM Peak																																																																																																							
NB Approach	0'	0'																																																																																																							
SB Approach	0'	0'																																																																																																							
EB Approach	1'	3'																																																																																																							
WB Approach	NA	NA																																																																																																							
	Delay Per Vehicle/LOS (Seconds)																																																																																																								
	AM Peak	PM Peak																																																																																																							
NB Approach	6.2/A	3.3/A																																																																																																							
SB Approach	0.0/A	0.0/A																																																																																																							
EB Approach	NA	NA																																																																																																							
WB Approach	9.1/A	8.9/A																																																																																																							
	Delay Per Vehicle/LOS (Seconds)																																																																																																								
	AM Peak	PM Peak																																																																																																							
NB Approach	3.1/A	3.0/A																																																																																																							
SB Approach	2.9/A	2.9/A																																																																																																							
EB Approach	2.9/A	3.0/A																																																																																																							
WB Approach	NA	NA																																																																																																							
	Queue Length (Feet)																																																																																																								
	AM Peak	PM Peak																																																																																																							
NB Approach	0'	0'																																																																																																							
SB Approach	0'	0'																																																																																																							
EB Approach	0'	0'																																																																																																							
WB Approach	NA	NA																																																																																																							
	Delay Per Vehicle/LOS (Seconds)																																																																																																								
	AM Peak	PM Peak																																																																																																							
NB Approach	3.1/A	3.0/A																																																																																																							
SB Approach	3.0/A	3.0/A																																																																																																							
EB Approach	NA	NA																																																																																																							
WB Approach	3.0/A	2.9/A																																																																																																							



# Intersection Control Evaluation

## Factors to Analyze (Intersection of STH 23 & CTH UU)

	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> <b>2-WAY STOP (TWSC)</b> <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input checked="" type="checkbox"/> <b>ROUNDABOUT</b> <input type="checkbox"/> 4-WAY STOP <input type="checkbox"/> 2-WAY STOP (TWSC) <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>GENERAL COMMENTS</b>																																		
<b>TRAFFIC VOLUMES &amp; OPERATIONAL ANALYSIS (CONT.)</b>	<p><b>CTH UU &amp; WB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2" style="text-align: center;">Queue Length (Feet)</th> </tr> <tr> <th style="text-align: center;">AM Peak</th> <th style="text-align: center;">PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td style="text-align: center;">5'</td> <td style="text-align: center;">1'</td> </tr> <tr> <td>SB Approach</td> <td style="text-align: center;">0'</td> <td style="text-align: center;">0'</td> </tr> <tr> <td>EB Approach</td> <td style="text-align: center;">NA</td> <td style="text-align: center;">NA</td> </tr> <tr> <td>WB Approach</td> <td style="text-align: center;">1'</td> <td style="text-align: center;">1'</td> </tr> </tbody> </table> <p>Refer to Appendix C for the full operational analysis results including the results by individual movement.</p>		Queue Length (Feet)		AM Peak	PM Peak	NB Approach	5'	1'	SB Approach	0'	0'	EB Approach	NA	NA	WB Approach	1'	1'	<p><b>CTH UU &amp; EB STH 23 RAMPS</b></p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th rowspan="2"></th> <th colspan="2" style="text-align: center;">Queue Length (Feet)</th> </tr> <tr> <th style="text-align: center;">AM Peak</th> <th style="text-align: center;">PM Peak</th> </tr> </thead> <tbody> <tr> <td>NB Approach</td> <td style="text-align: center;">0'</td> <td style="text-align: center;">0'</td> </tr> <tr> <td>SB Approach</td> <td style="text-align: center;">0'</td> <td style="text-align: center;">0'</td> </tr> <tr> <td>EB Approach</td> <td style="text-align: center;">NA</td> <td style="text-align: center;">NA</td> </tr> <tr> <td>WB Approach</td> <td style="text-align: center;">0'</td> <td style="text-align: center;">0'</td> </tr> </tbody> </table>		Queue Length (Feet)		AM Peak	PM Peak	NB Approach	0'	0'	SB Approach	0'	0'	EB Approach	NA	NA	WB Approach	0'	0'	<p>This is due to the slower entering speeds at roundabouts compared to the free flowing mainline traffic movement for the TWSC alternative. However, roundabouts have higher capacity potential than a TWSC type intersection.</p> <p>It is important to note that the methods used to analyze roundabouts and traditional intersections differ. Rodel software is used to analyze roundabout alternatives. Rodel utilizes an empirical method. Synchro software was used to analyze the TWSC alternative and utilizes a GAP acceptance method. These differences in the delay calculation will contribute to slight variations in the results.</p> <p>Refer to Appendices C and D for the full results of the TWSC and roundabout operational analysis.</p>
	Queue Length (Feet)																																				
	AM Peak	PM Peak																																			
NB Approach	5'	1'																																			
SB Approach	0'	0'																																			
EB Approach	NA	NA																																			
WB Approach	1'	1'																																			
	Queue Length (Feet)																																				
	AM Peak	PM Peak																																			
NB Approach	0'	0'																																			
SB Approach	0'	0'																																			
EB Approach	NA	NA																																			
WB Approach	0'	0'																																			
<b>CONSTRUCTION COSTS</b>	<p>Construction Cost = \$573,492 (Approx. Construction Year – 2013)</p> <p>Cost estimate area includes:</p> <ol style="list-style-type: none"> <li>1. CTH UU from 800' north of STH 23 to 750' south of STH 23.</li> <li>2. 800' of all four ramps</li> </ol>	<p>Construction Cost = \$916,761 (Approx. Construction Year – 2013)</p> <p>Cost estimate area includes:</p> <ol style="list-style-type: none"> <li>1. CTH UU from 1350' north of STH 23 to 1130' south of STH 23.</li> <li>2. 800' of all four ramps</li> </ol>	<p>These intersections will be constructed as part of the STH 23 expansion project from USH 151 – Log Tavern Road. These construction cost estimates include all major work items with the exception of earthwork and structures. The cost for earthwork and structures is expected to be nearly the same for either option and therefore was not included as part of a comparison of the two intersection alternatives.</p> <p>It is important to note that these intersection construction cost estimates were calculated assuming that they are one section of a much larger overall project. These estimates would likely be higher if this interchange was constructed as a stand-alone contract. This is due to the benefits of spreading contract costs such as mobilization, earthwork, traffic control, etc. over a larger project area.</p> <p>Refer to Appendix E for detailed cost estimate information and a sketch of the construction cost estimate area.</p>																																		

# Intersection Control Evaluation

## Factors to Analyze (Intersection of STH 23 & CTH UU)

	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> <b>2-WAY STOP (TWSC)</b> <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input checked="" type="checkbox"/> <b>ROUNDABOUT</b> <input type="checkbox"/> 4-WAY STOP <input type="checkbox"/> 2-WAY STOP (TWSC) <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>GENERAL COMMENTS</b>
<b>RIGHT-OF-WAY</b>	N/A	N/A	These intersections will be constructed with the STH 23 expansion project from USH 151 – Log Tavern Road. In general, roundabouts will require more right-of-way than a TWSC; however in the context of the overall STH 23 expansion project, the difference in right-of-way required will be negligible.
<b>PRACTICAL FEASIBILITY</b>	<p>Reconstructing the intersections to operate as TWSC should be considered a feasible alternative. In the context of a larger STH 23 expansion project there are no major adverse impacts associated with this intersection alternative that would not exist without the majority of work to upgrade the mainline and construct interchanges.</p> <p>This alternative does have safety and future capacity concerns when compared to the roundabout alternative. However there is no reason this alternative should not be considered feasible if considered on its own merits.</p>	<p>Reconstructing the intersections to operate as roundabouts should be considered a feasible alternative. In the context of a larger STH 23 expansion project there are no major adverse impacts associated with this intersection alternative that would not exist without the majority of work to upgrade the mainline and construct interchanges.</p>	

# Intersection Control Evaluation

## Factors to Analyze (Intersection of STH 23 & CTH UU)

	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> <b>2-WAY STOP (TWSC)</b> <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input checked="" type="checkbox"/> <b>ROUNDABOUT</b> <input type="checkbox"/> 4-WAY STOP <input type="checkbox"/> 2-WAY STOP (TWSC) <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>GENERAL COMMENTS</b>
<b>OPERATIONS &amp; MAINTENANCE COSTS</b>	<p>There are no operation and maintenance concerns unique to this alternative.</p>	<p>Operation and maintenance concerns unique to this alternative to be considered:</p> <p><u>Street Lighting</u> – It is reasonable to expect that 12 street light units will be required at each of the two ramp intersections.</p> <p>Opinion of probable energy costs:            Street Lighting =            16 – 250 Watt HPS Fixtures,            106 kWh/month/fixture =            1,696 kWh/month</p> <p>Plus 8 – 150 Watt HPS Fixtures,            61 kWh/month/fixture =            488 kWh/month            Total = 2,184 kWh/month.</p> <p>(based on 8 lights (250W) within the circulating roadway and 1 light (150W) at each approach)</p> <p>Yearly Usage            2,184 kWh/month x 12 months            = 26,208 kWh/year</p> <p>Yearly Cost @ \$0.10/kWh            = \$2,620.80/year</p> <p><u>Central Island Landscaping</u> – It is reasonable to expect that a greenscaped central island will require routine maintenance. A specific cost is not easily attributed without determining the exact design of the roundabout and landscaping items.</p> <p><u>Pavement Marking</u> – Roundabouts will generally require more pavement marking than traditional intersections. A specific cost is not easily attributed without determining the exact design of the roundabout.</p>	<p>The maintenance of either alternative will include periodic pavement and other roadway infrastructure rehabilitation. The difference between the two is likely to be negligible with the exception of the specific items listed under each alternative.</p>

# Intersection Control Evaluation

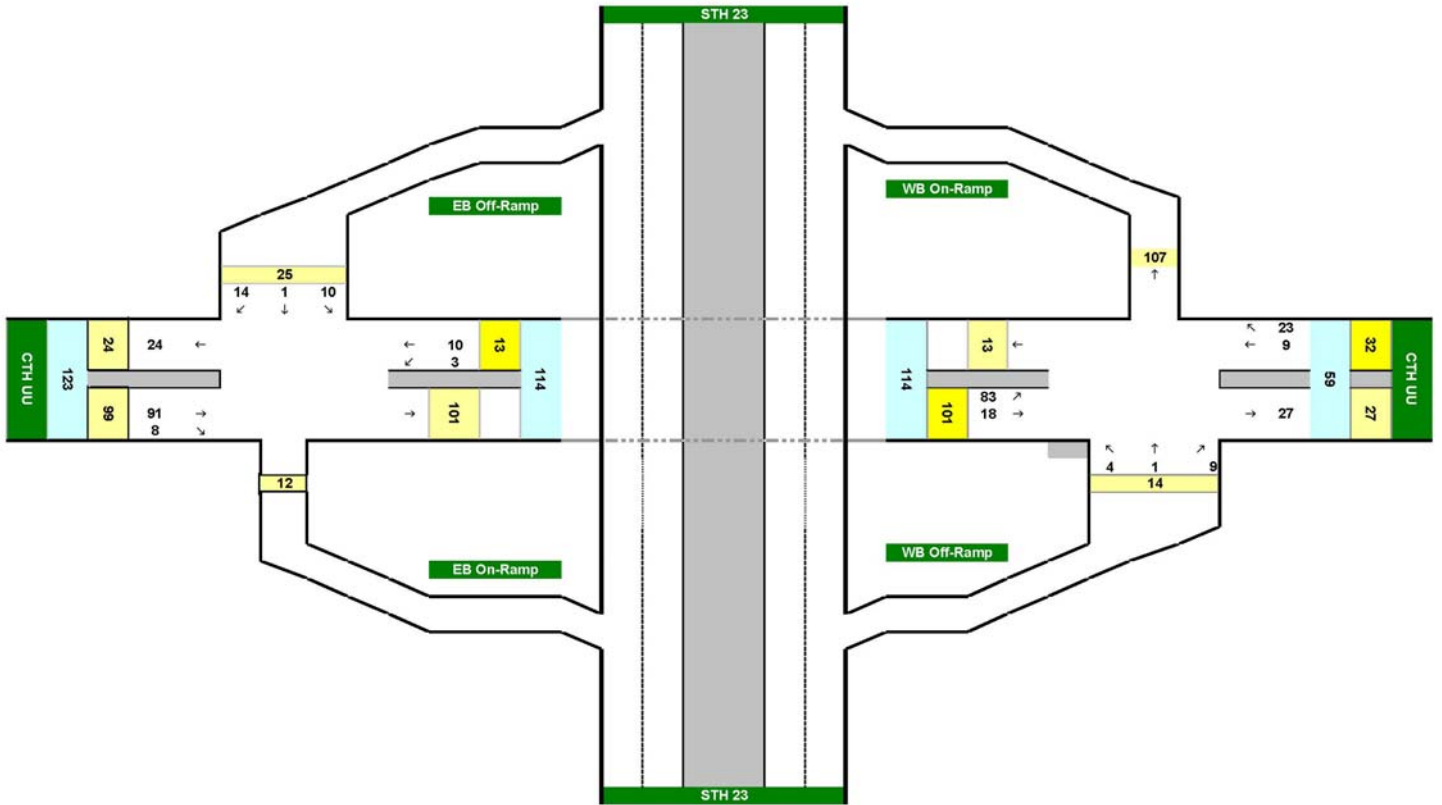
## Factors to Analyze (Intersection of STH 23 & CTH UU)

	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> <b>2-WAY STOP (TWSC)</b> <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>ALTERNATIVE CONTROL</b> <input type="checkbox"/> TRAFFIC SIGNAL, ANTICIPATING TRAFFIC SIGNAL <input checked="" type="checkbox"/> <b>ROUNDABOUT</b> <input type="checkbox"/> 4-WAY STOP <input type="checkbox"/> 2-WAY STOP (TWSC) <input type="checkbox"/> EXISTING CONTROL 2-WAY STOP	<b>GENERAL COMMENTS</b>
<b>ENVIRONMENTAL</b>	N/A	N/A	In the context of a larger STH 23 expansion project there are no major adverse impacts associated with either intersection alternative that would not exist without the majority of work to upgrade the mainline and construct interchanges.
<b>PED'S/BIKES</b>	On-street bike lanes and sidewalks will be constructed for pedestrian and bicycle usage with this alternative. Pedestrians will be able to cross the intersections by use of at-grade crosswalks.	On-street bike lanes as well as sidewalk/shared use paths will be constructed for pedestrian and bicycle usage with this alternative. Pedestrians will be able to cross the intersections by use of at-grade crosswalks.  Roundabouts will likely provide a safer crossing for pedestrians due to the slower entry speeds and needing to cross any one direction of traffic at a time when compared to the TWSC alternative.	
<b>RECOMMENDATION</b>	A TWSC intersection should be considered a viable alternative for both the east and west bound ramp intersections. It will operate at an acceptable level of service and can be upgraded to signalized control if traffic volumes increase to a level where warrants are met.  This alternative will likely have a lower long term operation and maintenance cost, but may have higher crash potential and less capacity.	A roundabout intersection should be considered a viable alternative for both the east and west bound ramp intersections. It will operate at an acceptable level of service with high residual capacity beyond the design year life.  This alternative will likely have a higher long term operation and maintenance cost, but may have lower crash potential and higher capacity.  A roundabout type intersection is recommended at both the ramp terminals at this location. The higher long term capacity potential of this alternative is a key factor since the area is adjacent to the rapidly growing Fond du Lac area. This higher capacity potential combined with the expected safety benefits further support the use of roundabouts at this location.	Refer to Appendix F for a sketch of each intersection alternative.

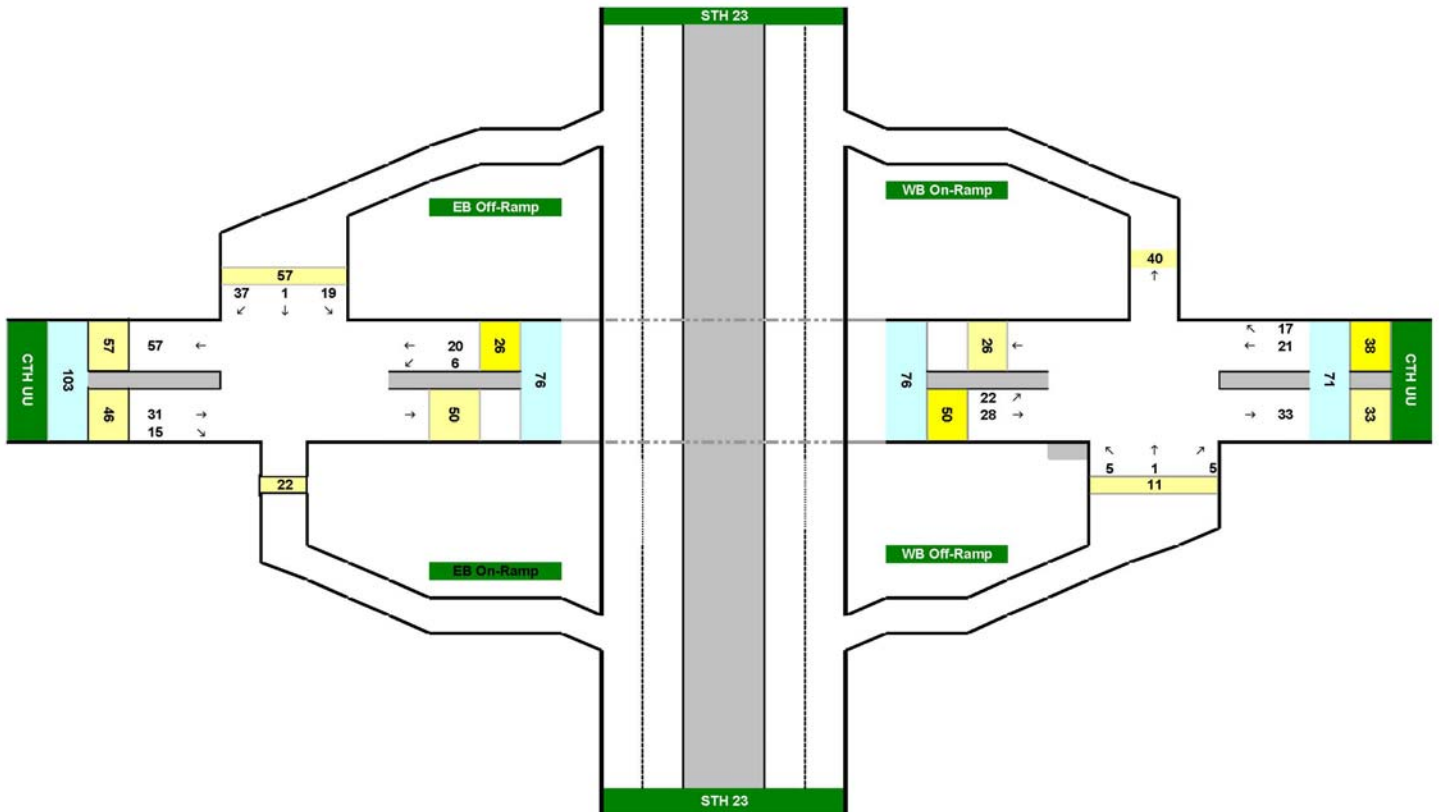
# **Appendix A**

## **2035 Traffic Volumes & Distribution Percentages**

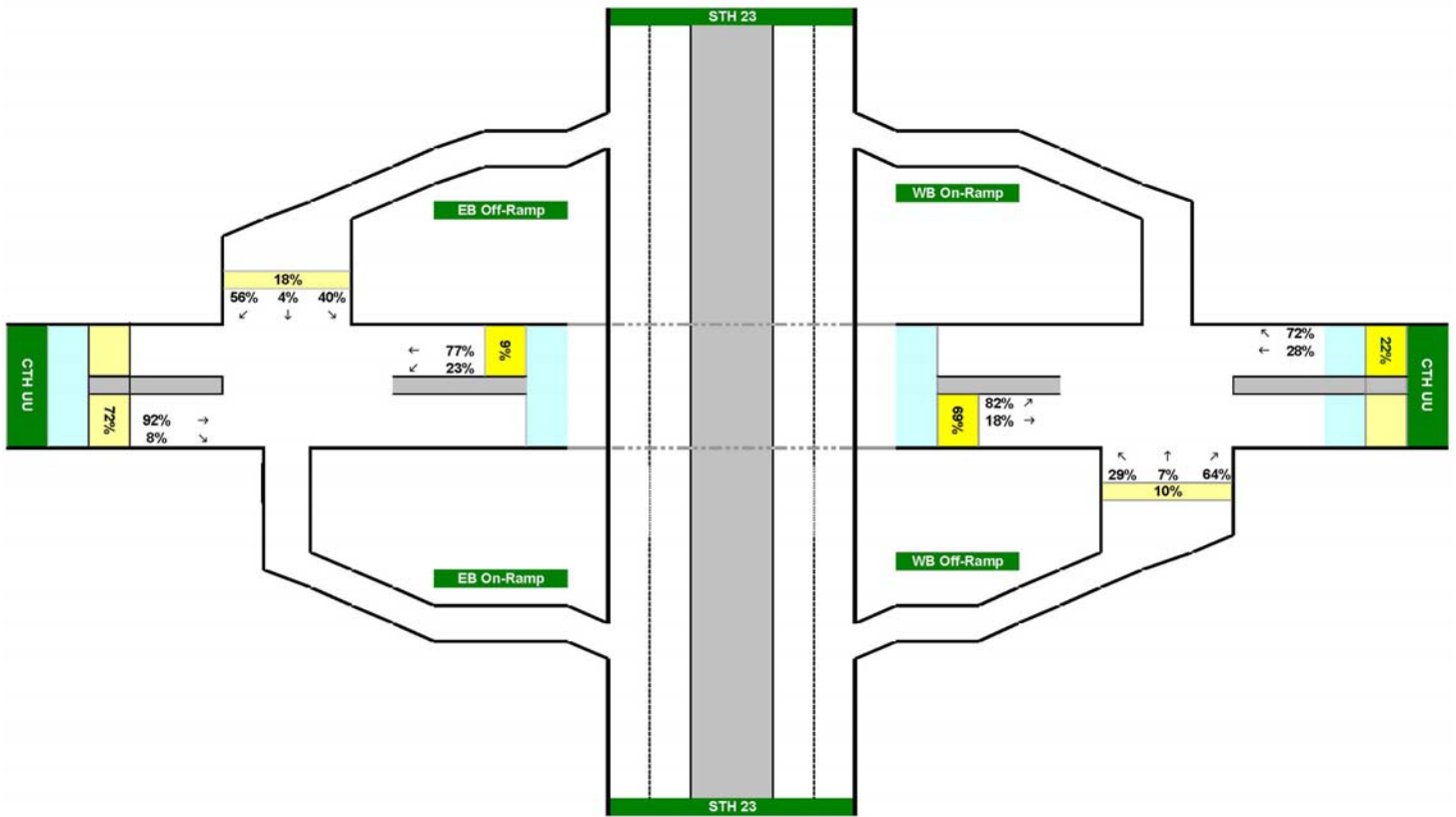
# 2035 AM Turning Movement Data



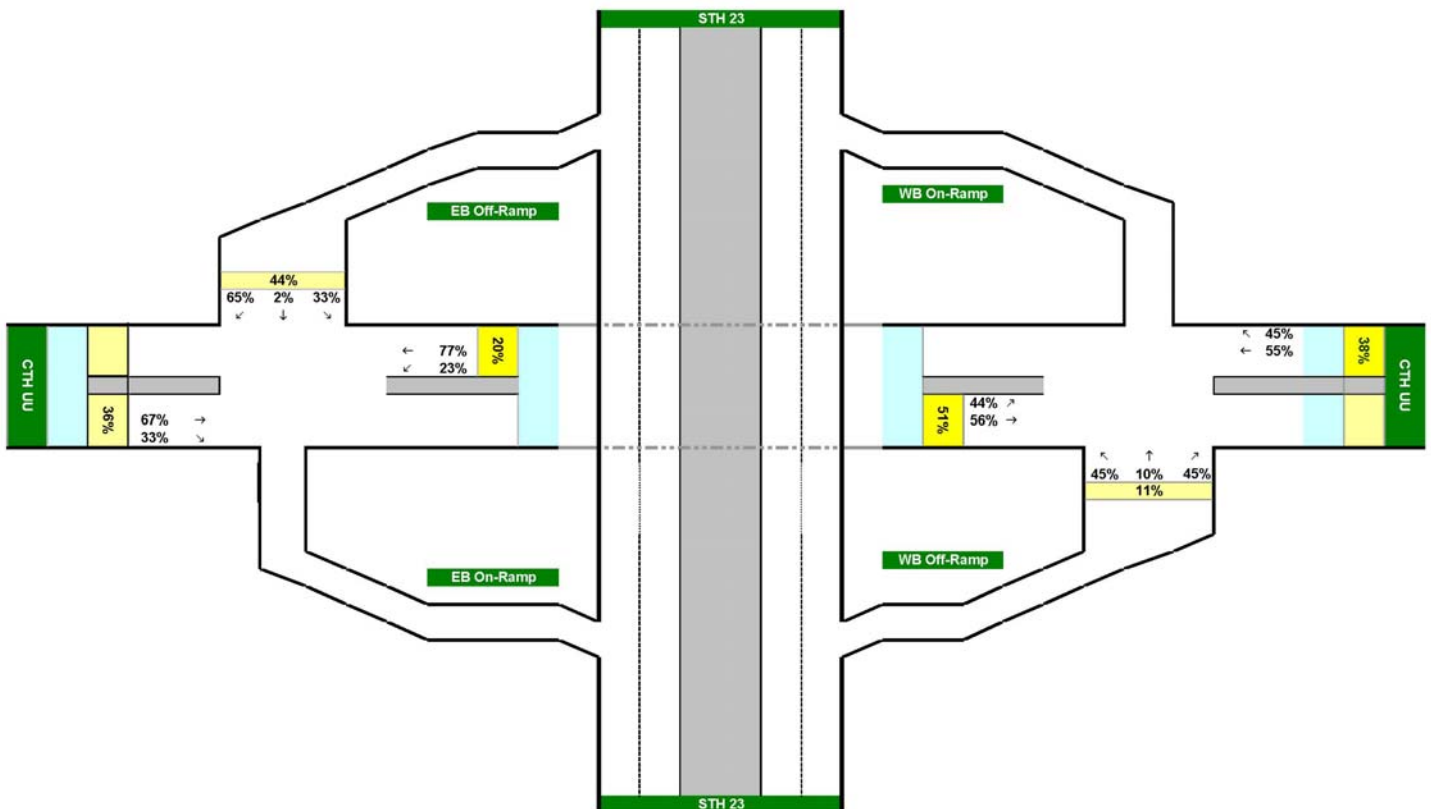
# 2035 PM Turning Movement Data



# 2035 AM Peak Traffic Distribution Percentages



# 2035 PM Peak Traffic Distribution Percentages



# **Appendix B**

## **2035 Warrant Analysis Check** (8-Hour Warrant – Peak Hour Only)





## **Appendix C**

### **TWSC Operational Analysis Results**



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖	↗					↑	↗		↖	
Volume (veh/h)	10	1	14	0	0	0	0	91	8	3	10	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	1	15	0	0	0	0	99	9	3	11	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)			4									
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	116	125	11	124	116	99	11			108		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	116	125	11	124	116	99	11			108		
tC, single (s)	7.2	6.6	6.2	7.2	6.6	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	99	100	99	100	100	100	100			100		
cM capacity (veh/h)	852	758	1061	828	767	949	1589			1465		

Direction, Lane #	EB 1	NB 1	NB 2	SB 1
Volume Total	27	99	9	14
Volume Left	11	0	0	3
Volume Right	15	0	9	0
cSH	1895	1700	1700	1465
Volume to Capacity	0.01	0.06	0.01	0.00
Queue Length 95th (ft)	1	0	0	0
Control Delay (s)	8.8	0.0	0.0	1.7
Lane LOS	A			A
Approach Delay (s)	8.8	0.0		1.7
Approach LOS	A			

Intersection Summary			
Average Delay		1.8	
Intersection Capacity Utilization		20.0%	ICU Level of Service A
Analysis Period (min)		15	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	19	1	37	0	0	0	0	31	15	6	20	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	21	1	40	0	0	0	0	34	16	7	22	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	68	85	22	89	68	34	22			50		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	68	85	22	89	68	34	22			50		
tC, single (s)	7.2	6.6	6.2	7.2	6.6	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	100	96	100	100	100	100			100		
cM capacity (veh/h)	914	796	1047	851	813	1031	1575			1537		
Direction, Lane #												
	EB 1	NB 1	NB 2	SB 1								
Volume Total	62	34	16	28								
Volume Left	21	0	0	7								
Volume Right	40	0	16	0								
cSH	1613	1700	1700	1537								
Volume to Capacity	0.04	0.02	0.01	0.00								
Queue Length 95th (ft)	3	0	0	0								
Control Delay (s)	8.7	0.0	0.0	1.7								
Lane LOS	A			A								
Approach Delay (s)	8.7	0.0		1.7								
Approach LOS	A											
Intersection Summary												
Average Delay			4.2									
Intersection Capacity Utilization			20.0%		ICU Level of Service					A		
Analysis Period (min)			15									





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↖	↗		↖			↕	↗
Volume (veh/h)	0	0	0	4	1	9	83	18	0	0	9	23
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	4	1	10	90	20	0	0	10	25
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)						4						
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	215	210	10	210	235	20	35			20		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	215	210	10	210	235	20	35			20		
tC, single (s)	7.2	6.6	6.2	7.2	6.6	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	99	100	99	94			100		
cM capacity (veh/h)	695	643	1063	708	622	1050	1557			1577		

Direction, Lane #	WB 1	NB 1	SB 1	SB 2
Volume Total	15	110	10	25
Volume Left	4	90	0	0
Volume Right	10	0	0	25
cSH	1633	1557	1700	1700
Volume to Capacity	0.01	0.06	0.01	0.01
Queue Length 95th (ft)	1	5	0	0
Control Delay (s)	9.1	6.2	0.0	0.0
Lane LOS	A	A		
Approach Delay (s)	9.1	6.2	0.0	
Approach LOS	A			

Intersection Summary			
Average Delay		5.1	
Intersection Capacity Utilization		22.2%	ICU Level of Service A
Analysis Period (min)		15	



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↖	↗		↖			↖	↗
Volume (veh/h)	0	0	0	5	1	5	22	28	0	0	21	17
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	5	1	5	24	30	0	0	23	18
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)						4						
Median type							None			None		
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	104	101	23	101	120	30	41			30		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	104	101	23	101	120	30	41			30		
tC, single (s)	7.2	6.6	6.2	7.2	6.6	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	99	100	99	98			100		
cM capacity (veh/h)	853	771	1045	863	753	1035	1549			1563		
<b>Direction, Lane #</b>	<b>WB 1</b>	<b>NB 1</b>	<b>SB 1</b>	<b>SB 2</b>								
Volume Total	12	54	23	18								
Volume Left	5	24	0	0								
Volume Right	5	0	0	18								
cSH	1548	1549	1700	1700								
Volume to Capacity	0.01	0.02	0.01	0.01								
Queue Length 95th (ft)	1	1	0	0								
Control Delay (s)	8.9	3.3	0.0	0.0								
Lane LOS	A	A										
Approach Delay (s)	8.9	3.3	0.0									
Approach LOS	A											
<b>Intersection Summary</b>												
Average Delay			2.7									
Intersection Capacity Utilization		20.0%		ICU Level of Service						A		
Analysis Period (min)			15									

## **Appendix D**

### **Roundabout Operational Analysis Results**

# RODEL Results / CTH UU (EAST BOUND)

RODEL											
10:2:08 CTH UU EB STH 23					3						
E	<m>	4.25	4.25	4.25	4.25	TIME PERIOD	min	90			
L'	<m>	40.00	40.00	40.00	40.00	TIME SLICE	min	15			
U	<m>	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15 75			
RAD	<m>	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00			
PHI	<d>	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75			
DIA	<m>	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	UEH			
GRAD	SEP	0	0	0	0	FLOW PEAK	am/op/pm	AM			
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOP	CL	FLOW RATIO			FLOW TIME
SB CTH UU	1.05	0	010	003	001	1.00	50	0.75	1.125	0.75	15 45 75
EB 23 OFF	1.05	014	001	010	001	1.00	50	0.75	1.125	0.75	15 45 75
NB CTH UU	1.05	008	091	0	001	1.00	50	0.75	1.125	0.75	15 45 75
EB 23 ON	1.05	0	0	0	0	1.00	50	0.75	1.125	0.75	15 45 75
MODE 2											
FLOW	veh	14	26	100	0	AUEDEL	s	3.1			
CAPACITY	veh	1239	1231	1231	1182	LOS SIG	A				
AVE DELAY	secs	2.9	2.9	3.1	0.0	LOS UNSIG	A				
MAX DELAY	secs	3.6	3.7	3.9	0.0	VEHIC HRS	0.1				
AVE QUEUE	veh	0	0	0	0	COST	\$	2			
MAX QUEUE	veh	0	0	0	0						
F1mode F2direct F3peak CtrlF3rev F4fact F6stats F8econ F9prnt F10run Esc											

AM 50

RODEL											
10:2:08 CTH UU EB STH 23					5						
E	<m>	4.25	4.25	4.25	4.25	TIME PERIOD	min	90			
L'	<m>	40.00	40.00	40.00	40.00	TIME SLICE	min	15			
U	<m>	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15 75			
RAD	<m>	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00			
PHI	<d>	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75			
DIA	<m>	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	UEH			
GRAD	SEP	0	0	0	0	FLOW PEAK	am/op/pm	AM			
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOP	CL	FLOW RATIO			FLOW TIME
SB CTH UU	1.05	0	010	003	001	1.00	85	0.75	1.125	0.75	15 45 75
EB 23 OFF	1.05	014	001	010	001	1.00	85	0.75	1.125	0.75	15 45 75
NB CTH UU	1.05	008	091	0	001	1.00	85	0.75	1.125	0.75	15 45 75
EB 23 ON	1.05	0	0	0	0	1.00	85	0.75	1.125	0.75	15 45 75
MODE 2											
FLOW	veh	14	26	100	0	AUEDEL	s	3.7			
CAPACITY	veh	1041	1034	1034	985	LOS SIG	A				
AVE DELAY	secs	3.4	3.5	3.8	0.0	LOS UNSIG	A				
MAX DELAY	secs	4.3	4.4	4.8	0.0	VEHIC HRS	0.1				
AVE QUEUE	veh	0	0	0	0	COST	\$	2			
MAX QUEUE	veh	0	0	0	0						
F1mode F2direct F3peak CtrlF3rev F4fact F6stats F8econ F9prnt F10run Esc											

AM 85



# RODEL Results / CTH UU (EAST BOUND)

RODEL													
10:7:08 CTH UU EB STH 23 4													
E (m)	4.25	4.25	4.25	4.25	TIME PERIOD	min	90						
L' (m)	40.00	40.00	40.00	40.00	TIME SLICE	min	15						
U (m)	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15 75						
RAD (m)	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00						
PHI (d)	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75						
DIA (m)	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	UEH						
GRAD SEP	0	0	0	0	FLOW PEAK	am/op/pm	PM						
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOF	CL	FLOW RATIO			FLOW TIME		
SB CTH UU	1.05	0	020	006	001	1.00	50	0.75	1.125	0.75	15	45	75
EB 23 OFF	1.05	037	001	019	001	1.00	50	0.75	1.125	0.75	15	45	75
NB CTH UU	1.05	015	031	0	001	1.00	50	0.75	1.125	0.75	15	45	75
EB 23 ON	1.05	0	0	0	0	1.00	50	0.75	1.125	0.75	15	45	75
MODE 2													
FLOW	veh	27	58	47	0	AUEDEL	s	3.0					
CAPACITY	veh	1239	1224	1224	1210	LOS	SIG	A					
AUE DELAY	secs	2.9	3.0	3.0	0.0	LOS	UNSIG	A					
MAX DELAY	secs	3.6	3.8	3.8	0.0	VEHIC HRS		0.1					
AUE QUEUE	veh	0	0	0	0	COST	\$	2					
MAX QUEUE	veh	0	0	0	0								
F1mode F2direct F3peak CtrlF3rev F4fact F6stats F8econ F9prnt F10run Esc													

PM 50

RODEL													
10:7:08 CTH UU EB STH 23 6													
E (m)	4.25	4.25	4.25	4.25	TIME PERIOD	min	90						
L' (m)	40.00	40.00	40.00	40.00	TIME SLICE	min	15						
U (m)	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15 75						
RAD (m)	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00						
PHI (d)	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75						
DIA (m)	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	UEH						
GRAD SEP	0	0	0	0	FLOW PEAK	am/op/pm	PM						
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOF	CL	FLOW RATIO			FLOW TIME		
SB CTH UU	1.05	0	020	006	001	1.00	85	0.75	1.125	0.75	15	45	75
EB 23 OFF	1.05	037	001	019	001	1.00	85	0.75	1.125	0.75	15	45	75
NB CTH UU	1.05	015	031	0	001	1.00	85	0.75	1.125	0.75	15	45	75
EB 23 ON	1.05	0	0	0	0	1.00	85	0.75	1.125	0.75	15	45	75
MODE 2													
FLOW	veh	27	58	47	0	AUEDEL	s	3.6					
CAPACITY	veh	1041	1027	1027	1013	LOS	SIG	A					
AUE DELAY	secs	3.5	3.6	3.6	0.0	LOS	UNSIG	A					
MAX DELAY	secs	4.3	4.6	4.5	0.0	VEHIC HRS		0.1					
AUE QUEUE	veh	0	0	0	0	COST	\$	2					
MAX QUEUE	veh	0	0	0	0								
F1mode F2direct F3peak CtrlF3rev F4fact F6stats F8econ F9prnt F10run Esc													

PM 85

# RODEL Results / CTH UU (WEST BOUND)

RODEL													
10:7:08 CTH UU WB STH 23													
E (m)	4.25	4.25	4.25	4.25	TIME PERIOD	min	90						
L' (m)	40.00	40.00	40.00	40.00	TIME SLICE	min	15						
U (m)	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15	75					
RAD (m)	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00						
PHI (d)	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75						
DIA (m)	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	UEH						
GRAD SEP	0	0	0	0	FLOW PEAK	am/op/pm	AM						
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOP	CL	FLOW RATIO			FLOW TIME		
SB CTH UU	1.05	023	009	0	001	1.00	50	0.75	1.125	0.75	15	45	75
WB 23 ON	1.05	0	0	0	0	1.00	50	0.75	1.125	0.75	15	45	75
NB CTH UU	1.05	0	018	083	001	1.00	50	0.75	1.125	0.75	15	45	75
WB 23 OFF	1.05	009	001	004	001	1.00	50	0.75	1.125	0.75	15	45	75
MODE 2													
FLOW	veh	33	0	102	15	AVEDEL	s	3.1					
CAPACITY	veh	1190	1231	1239	1182	LOS SIG	A						
AVE DELAY	secs	3.0	0.0	3.1	3.0	LOS UNSIG	A						
MAX DELAY	secs	3.8	0.0	3.9	3.8	VEHIC HRS	0.1						
AVE QUEUE	veh	0	0	0	0	COST	\$/	2					
MAX QUEUE	veh	0	0	0	0								
F1mode	F2direct	F3peak	CtrlF3rev	F4fact	F6stats	F8econ	F9prnt	F10run Esc					

AM 50

RODEL													
10:7:08 CTH UU WB STH 23													
E (m)	4.25	4.25	4.25	4.25	TIME PERIOD	min	90						
L' (m)	40.00	40.00	40.00	40.00	TIME SLICE	min	15						
U (m)	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15	75					
RAD (m)	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00						
PHI (d)	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75						
DIA (m)	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	UEH						
GRAD SEP	0	0	0	0	FLOW PEAK	am/op/pm	AM						
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOP	CL	FLOW RATIO			FLOW TIME		
SB CTH UU	1.05	023	009	0	001	1.00	85	0.75	1.125	0.75	15	45	75
WB 23 ON	1.05	0	0	0	0	1.00	85	0.75	1.125	0.75	15	45	75
NB CTH UU	1.05	0	018	083	001	1.00	85	0.75	1.125	0.75	15	45	75
WB 23 OFF	1.05	009	001	004	001	1.00	85	0.75	1.125	0.75	15	45	75
MODE 2													
FLOW	veh	33	0	102	15	AVEDEL	s	3.7					
CAPACITY	veh	993	1034	1041	985	LOS SIG	A						
AVE DELAY	secs	3.7	0.0	3.8	3.6	LOS UNSIG	A						
MAX DELAY	secs	4.6	0.0	4.7	4.6	VEHIC HRS	0.2						
AVE QUEUE	veh	0	0	0	0	COST	\$/	2					
MAX QUEUE	veh	0	0	0	0								
F1mode	F2direct	F3peak	CtrlF3rev	F4fact	F6stats	F8econ	F9prnt	F10run Esc					

AM 85



# RODEL Results / CTH UU (WEST BOUND)

RODEL											
10:7:08 CTH UU WB STH 23 3											
E	(m)	4.25	4.25	4.25	4.25	TIME PERIOD	min	90			
L'	(m)	40.00	40.00	40.00	40.00	TIME SLICE	min	15			
U	(m)	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15 75			
RAD	(m)	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00			
PHI	(d)	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75			
DIA	(m)	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	VEH			
GRAD SEP		0	0	0	0	FLOW PEAK	am/op/pm	PM			
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOF	CL	FLOW RATIO		FLOW TIME	
SB CTH UU	1.05	017	021	0	001	1.00	50	0.75	1.125	0.75	15 45 75
WB 23 ON	1.05	0	0	0	0	1.00	50	0.75	1.125	0.75	15 45 75
NB CTH UU	1.05	0	028	022	001	1.00	50	0.75	1.125	0.75	15 45 75
WB 23 OFF	1.05	005	001	005	001	1.00	50	0.75	1.125	0.75	15 45 75
MODE 2											
FLOW	veh	39	0	51	12	AUEDEL	s	3.0			
CAPACITY	veh	1223	1224	1239	1211	LOS SIG	A				
AUE DELAY	secs	3.0	0.0	3.0	2.9	LOS UNSIG	A				
MAX DELAY	secs	3.7	0.0	3.7	3.7	VEHIC HRS	0.1				
AUE QUEUE	veh	0	0	0	0	COST	\$	1			
MAX QUEUE	veh	0	0	0	0						
F1mode F2direct F3peak CtrIF3rev F4fact F6stats F8econ F9prnt F10run Esc											

PM 50

RODEL											
10:7:08 CTH UU WB STH 23 6											
E	(m)	4.25	4.25	4.25	4.25	TIME PERIOD	min	90			
L'	(m)	40.00	40.00	40.00	40.00	TIME SLICE	min	15			
U	(m)	3.65	3.65	3.65	3.65	RESULTS PERIOD	min	15 75			
RAD	(m)	20.00	20.00	20.00	20.00	TIME COST	\$/hr	15.00			
PHI	(d)	25.00	25.00	25.00	25.00	FLOW PERIOD	min	15 75			
DIA	(m)	45.00	45.00	45.00	45.00	FLOW TYPE	pcu/veh	VEH			
GRAD SEP		0	0	0	0	FLOW PEAK	am/op/pm	PM			
LEG NAME	PCU	TURNS (1st exit, 2nd..U)				FLOF	CL	FLOW RATIO		FLOW TIME	
SB CTH UU	1.05	017	021	0	001	1.00	85	0.75	1.125	0.75	15 45 75
WB 23 ON	1.05	0	0	0	0	1.00	85	0.75	1.125	0.75	15 45 75
NB CTH UU	1.05	0	028	022	001	1.00	85	0.75	1.125	0.75	15 45 75
WB 23 OFF	1.05	005	001	005	001	1.00	85	0.75	1.125	0.75	15 45 75
MODE 2											
FLOW	veh	39	0	51	12	AUEDEL	s	3.6			
CAPACITY	veh	1026	1026	1041	1014	LOS SIG	A				
AUE DELAY	secs	3.6	0.0	3.6	3.5	LOS UNSIG	A				
MAX DELAY	secs	4.5	0.0	4.5	4.4	VEHIC HRS	0.1				
AUE QUEUE	veh	0	0	0	0	COST	\$	2			
MAX QUEUE	veh	0	0	0	0						
F1mode F2direct F3peak CtrIF3rev F4fact F6stats F8econ F9prnt F10run Esc											

PM 85

**Appendix E**

**Cost Estimates**

**STH 23 PROJECT 1440-15-01**

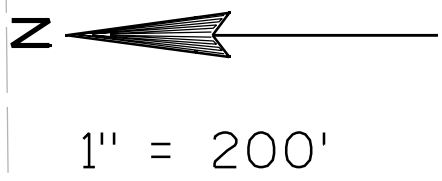
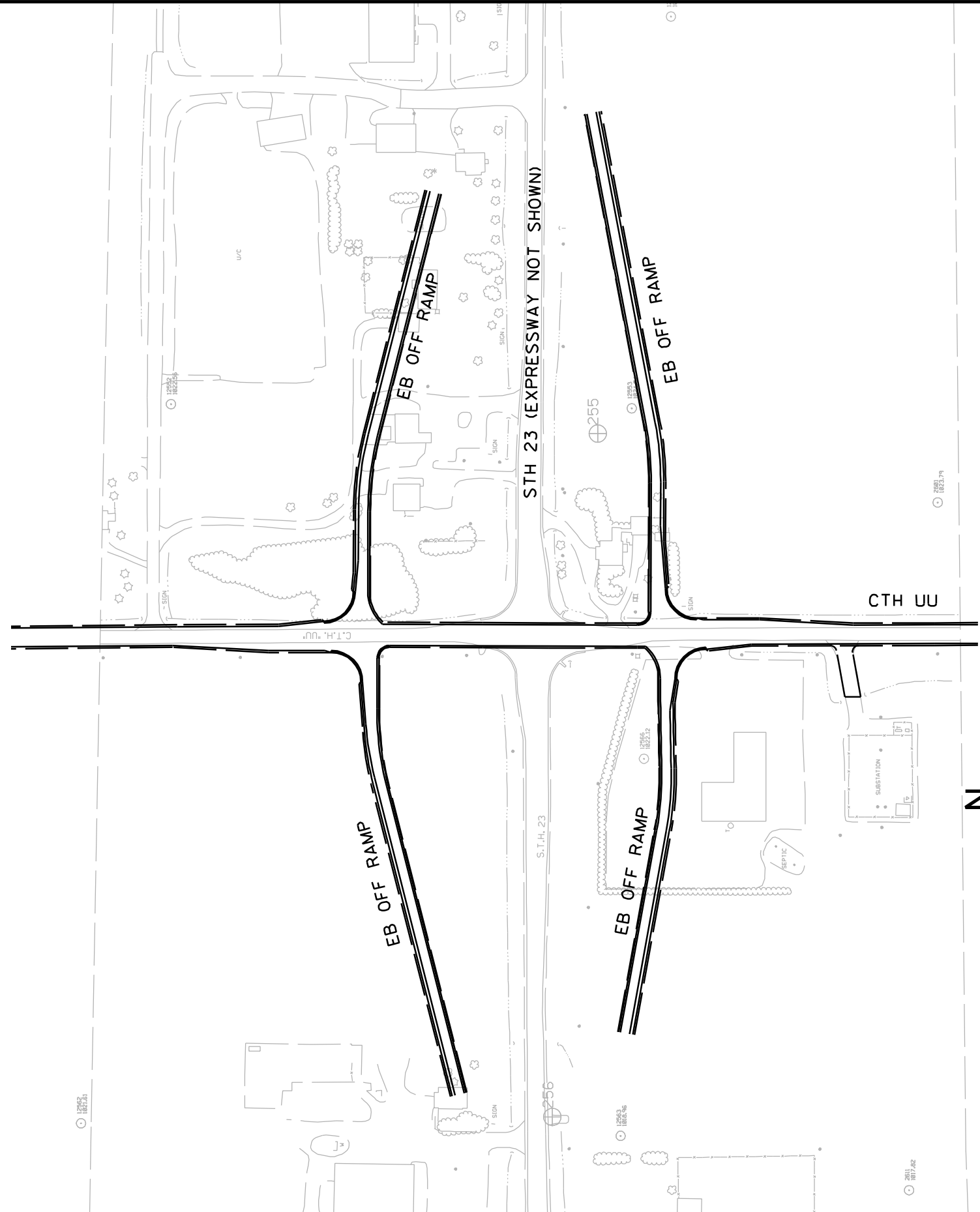
**COST ESTIMATE FOR:**

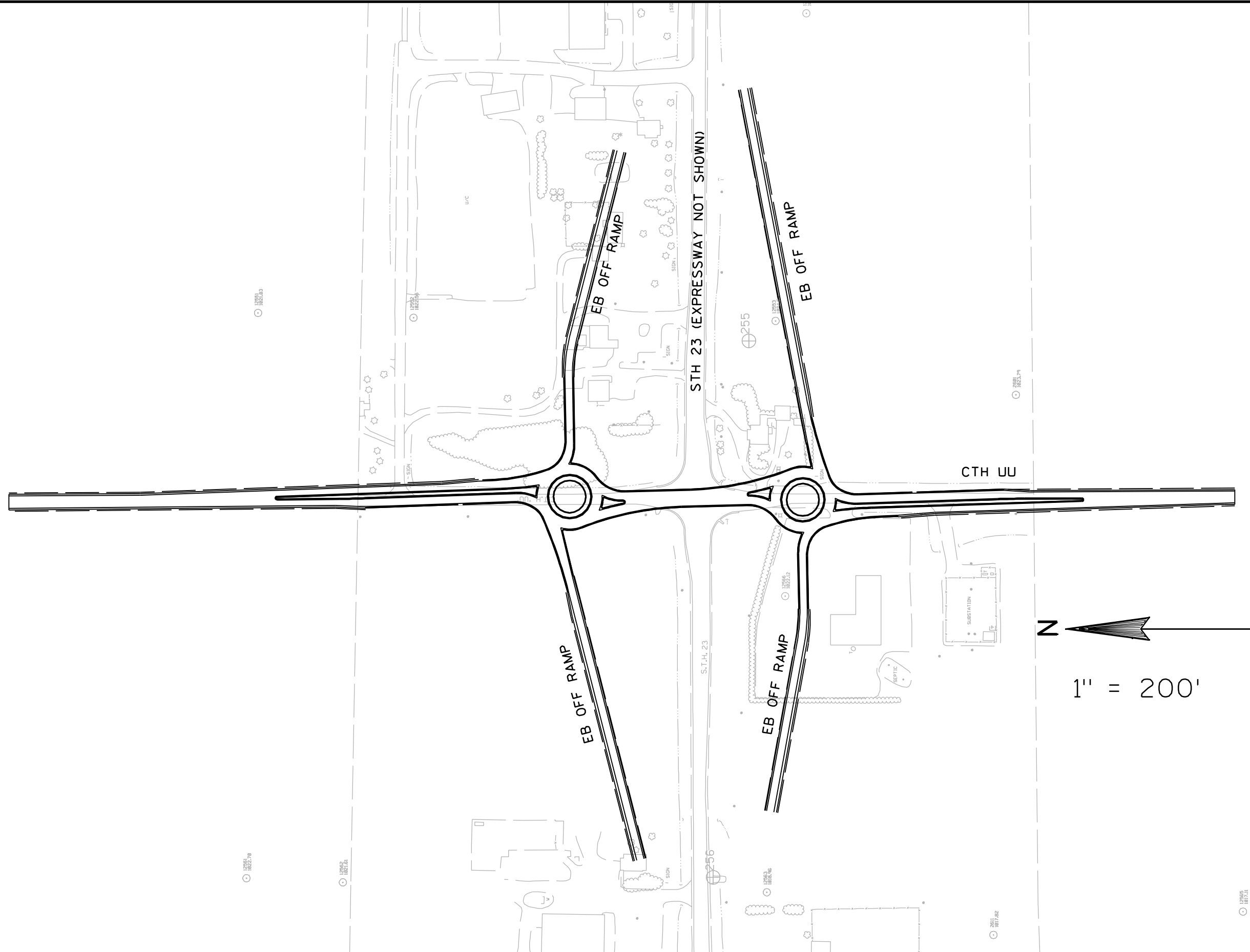
**STH 23 & CTH UU - ROUNDABOUT ALTERNATIVE**

ITEM	ITEM DESCRIPTION	UNIT	QUANTITY	UNIT PRICE	TOTAL
1	CONCRETE PAVEMENT 8-INCH	SY	3565	\$35.00	\$124,775
2	HMA PAVEMENT TYPE E-0.3	TON	2580	\$50.00	\$129,000
3	HMA PAVEMENT TYPE E-1	TON	430	\$65.00	\$27,950
4	ASPHALTIC MATERIAL	TON	181	\$450.00	\$81,450
5	TACK COAT	GAL	326	\$2.50	\$815
6	BASE AGGREGATE DENSE 3/4-INCH	TON	782	\$12.50	\$9,775
7	BASE AGGREGATE DENSE 1 1/4-INCH	TON	16010	\$10.00	\$160,100
8	CONCRETE SIDEWALK	SF	17624	\$3.00	\$52,872
9	CONCRETE CURB & GUTTER 30-INCH	LF	6496	\$10.50	\$68,208
10	CONCRETE CURB & GUTTER 36-INCH	LF	0	\$12.00	\$0
11	CONCRETE TRUCK APRON	SY	531	\$55.00	\$29,205
12	DRAINAGE ITEMS	LS	15% Of Items 1 - 11		\$102,623
13	LANDSCAPING ITEMS	LS	2% Of Items 1 - 11		\$13,683
14	EROSION CONTROL ITEMS	LS	2% Of Items 1 - 11		\$13,683
15	PAVEMENT MARKING ITEMS	LS	3% Of Items 1 - 11		\$20,525
16	SIGNING ITEMS	LS	2% Of Items 1 - 11		\$13,683
17	LIGHTING ITEMS	LS	10% Of Items 1 - 11		\$68,415
<b>GRAND TOTAL COST</b>					<b>\$916,761</b>

**STH 23 & CTH UU - TWO WAY STOP CONTROL ALTERNATIVE**

ITEM	ITEM DESCRIPTION	UNIT	QUANTITY	UNIT PRICE	TOTAL
1	CONCRETE PAVEMENT 8-INCH	SY	4734	\$35.00	\$165,690
2	HMA PAVEMENT TYPE E-0.3	TON	1545	\$50.00	\$77,250
3	HMA PAVEMENT TYPE E-1	TON	542	\$65.00	\$35,230
4	ASPHALTIC MATERIAL	TON	125	\$450.00	\$56,250
5	TACK COAT	GAL	226	\$2.50	\$565
6	BASE AGGREGATE DENSE 3/4-INCH	TON	804	\$12.50	\$10,050
7	BASE AGGREGATE DENSE 1 1/4-INCH	TON	11820	\$10.00	\$118,200
8	CONCRETE SIDEWALK	SF	0	\$3.00	\$0
9	CONCRETE CURB & GUTTER 30-INCH	LF	0	\$10.50	\$0
10	CONCRETE CURB & GUTTER 36-INCH	LF	570	\$12.00	\$6,840
11	DRAINAGE ITEMS	LS	15% Of Items 1 - 10		\$70,511
12	LANDSCAPING ITEMS	LS	2% Of Items 1 - 10		\$9,402
13	EROSION CONTROL ITEMS	LS	2% Of Items 1 - 10		\$9,402
14	PAVEMENT MARKING ITEMS	LS	2% Of Items 1 - 10		\$9,402
15	SIGNING ITEMS	LS	1% Of Items 1 - 10		\$4,701
16	LIGHTING ITEMS	LS	0% Of Items 1 - 10		\$0
<b>GRAND TOTAL COST</b>					<b>\$573,492</b>



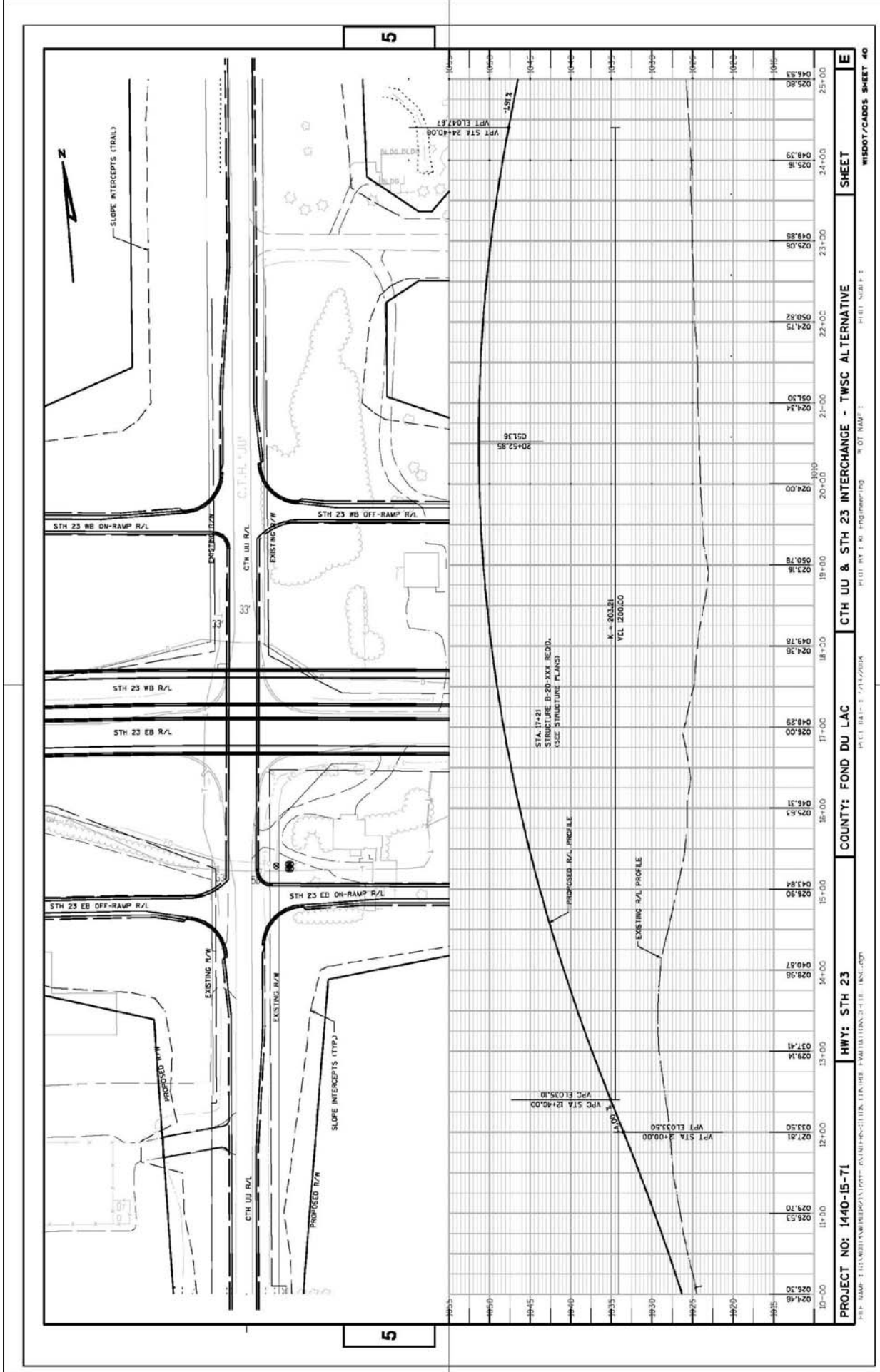


**Appendix F**

**Sketch of Alternatives**



# CTH UU & STH 23 INTERSECTION - TWSC ALTERNATIVE



STATION	PROPOSED R/L PROFILE	EXISTING R/L PROFILE
10+00	024.46	024.46
11+00	026.53	029.70
12+00	027.81	033.50
13+00	029.14	037.41
14+00	029.56	040.87
15+00	026.50	041.84
15+00	025.63	046.31
16+00	026.00	048.29
18+00	026.30	045.78
19+00	023.16	050.78
20+00	024.00	051.30
21+00	024.34	051.30
22+00	024.75	050.82
23+00	029.06	049.85
24+00	048.39	048.39
25+00	046.53	046.53

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

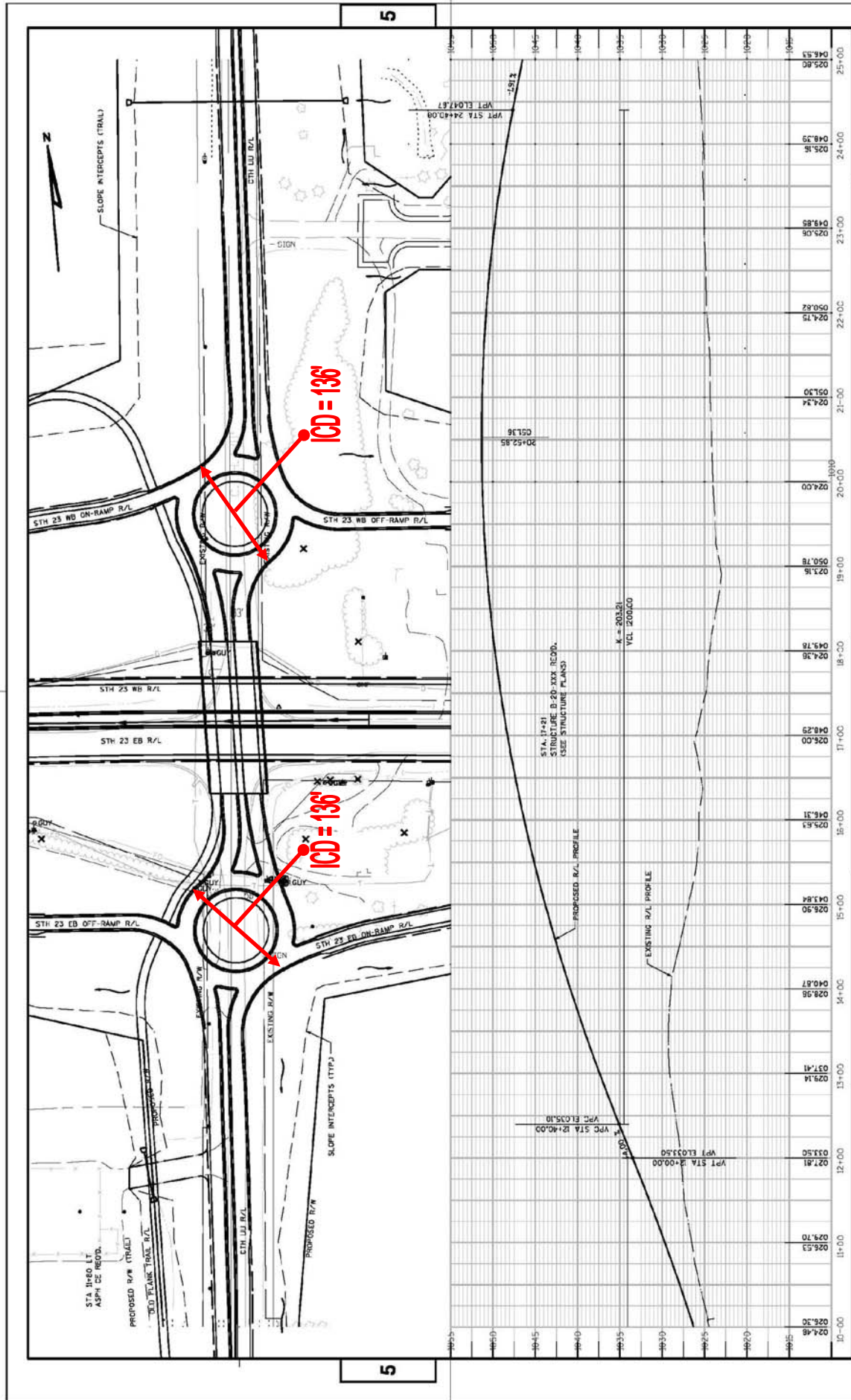
PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

PROJECT NO: 1440-15-71  
 COUNTY: FOND DU LAC  
 HWY: STH 23  
 CTH UU & STH 23 INTERCHANGE - TWSC ALTERNATIVE  
 SHEET

# CTH UU & STH 23 INTERSECTION - ROUNDABOUT ALTERNATIVE



STATION	PROPOSED R/L PROFILE	EXISTING R/L PROFILE	STATION	PROPOSED R/L PROFILE	EXISTING R/L PROFILE
10+00	026.30	026.30	13+00	029.14	029.14
11+00	026.53	026.53	14+00	028.98	028.98
12+00	027.81	027.81	15+00	026.90	026.90
13+00	033.50	033.50	16+00	025.61	025.61
14+00	029.70	029.70	17+00	026.00	026.00
15+00	026.51	026.51	18+00	024.78	024.78
16+00	026.00	026.00	19+00	023.16	023.16
17+00	026.00	026.00	20+00	024.00	024.00
18+00	024.78	024.78	21+00	024.34	024.34
19+00	023.16	023.16	22+00	024.75	024.75
20+00	024.00	024.00	23+00	029.08	029.08
21+00	024.34	024.34	24+00	028.58	028.58
22+00	024.75	024.75	25+00	028.80	028.80
23+00	029.08	029.08			
24+00	028.58	028.58			
25+00	028.80	028.80			

## **Appendix G**

### **STH 23 Corridor Crash Report**

# Crash Report

Project ID 1440-15-01  
Fond du Lac - Plymouth Road  
(CTH K - CTH W)  
STH 23  
Fond du Lac County

Prepared by:  
The logo for KL Engineering features a large, stylized 'KL' in blue with a white outline and a black drop shadow. To the right of the 'KL' is the word 'Engineering' in a bold, italicized, black serif font.

*5950 Seminole Centre Court Suite  
200  
Madison, WI 53711*

**June, 2007**

# TABLE OF CONTENTS

	<b>PAGE</b>
I. Introduction .....	1
II. Traffic and Crash Data.....	1
III. Crash Rates .....	5
IV. Conclusion .....	6

## **Appendix**

Appendix A – Project Location Map .....	8
Appendix B – AADT & Crash Rate Computations .....	10
Appendix C – Wisconsin Traffic Volume Data .....	11
Appendix D – Crash Maps .....	15
Appendix E – Intersection Related Crash Data.....	19
Appendix F – Non-Intersection Related Crash Data .....	23

## I. Introduction

The proposed project begins at USH 151 and proceeds easterly approximately 8.3 miles to Log Tavern Road.

This report summarizes crash data for STH 23 from USH 151 to Log Tavern Road. The corridor was divided into 3 segments to correlate crashes with AADT. The segments included the 0.7 mile segment from the Northbound ramps of USH 151 to CTH K which is a primarily four lane divided rural section (posted speed from 35 mph to 45mph), the 1.3 mile segment from CTH K to the CTH UU which is primarily a four lane divided rural section (posted speed 55 mph) and the 6.3 mile segment from CTH UU to Log Tavern Road which is a two lane undivided rural section (posted speed of 55 mph). See Appendix A for Project Location Map.

## II. Traffic and Crash Data

### Annual Daily Traffic (AADT)

Year 2000 AADT's from the Wisconsin Highway Traffic Volume Data and Traffic Counts for 2006 provided by WisDOT were used to interpolate AADT's for the years 2001 - 2005; using the equation  $F=P(1+i)^n$  where F is the future year, P is the present year's AADT, i is the growth rate and n is the number of years from the present year. Table 1 shows the AADT's for the period from 2001-2005. See Appendix B for AADT computations and Appendix C for Wisconsin Highway Traffic Volume Data maps.

**Table 1: AADT's**

SEGMENT	FROM	TO	2001	2002	2003	2004	2005
1	USH 151	CTH K	13600	13600	13600	13600	13600
2	CTH K	CTH UU	10395	10593	10795	11001	11211
3	CTH UU	LOG TAVERN RD	8181	8263	8346	8430	8515

### Vehicle Miles Traveled (VMT)

Vehicles miles traveled is a measure of the number of vehicles and the distance that was traveled by those vehicles. The VMT is used to compute a crash rate. Table 2 summarizes the vehicle miles traveled in millions and was computed using the following:

$$VMT = (AADT \times 365 \text{ days} \times LENGTH)/1,000,000$$

**Table 2: VMT in Millions**

	LENGTH	2001	2002	2003	2004	2005
SEGMENT 1	0.70 MILES	3.5	3.5	3.5	3.5	3.5
SEGMENT 2	1.26 MILES	4.8	4.9	5.0	5.1	5.2
SEGMENT 3	6.29 MILES	18.8	19.0	19.2	19.3	19.5

### **Total Crashes**

The crash data provided by WisDOT includes crashes from 2001 thru 2005 that occurred on the State Trunk Highway System. The database includes crashes involving \$1,000 or more damage to any one vehicle, an injury of fatality, and \$200 or more in damage to government property such as traffic sign's or guard rail. Table 3 summarizes the crashes by segment according to crash severity.

**Table 3: Total # of Crashes**

Segment Number	Total # of Crashes	Property Damage	Crash With Injury	Crash With Fatality
Segment 1	28	14	14	0
Segment 2	21	10	11	0
Segment 3	53	25	28	0
<b>Project Totals</b>	<b>102</b>	<b>49</b>	<b>53</b>	<b>0</b>

### **Intersection and Non-Intersection Related crashes**

The crash data that was used for this analysis identifies the crashes that are intersection related and those that are not related to intersections. "Intersection Related" is defined in the Law Enforcement Officer's Instruction Manual for Completing the Wisconsin Motor Vehicle Crash Report Form (MV4000) as the following:

“"Intersection Related" crashes are crashes which result from an activity, behavior, or traffic control which affects a unit's movement in relation to an intersection; whether or not the point of origin or first harmful event occurred within the intersection”

### **Intersection Related Crashes**

Each intersection along the corridor was analyzed to determine the number, type and severity of crashes that occurred. The number of crashes at the intersections accounted for 43 percent of the total crashes through the corridor and 49 percent of injury crashes. Segment 1 accounts for 50 percent of intersection related crashes.

Table 4 summarizes crash data by crash severity and crash type for each intersection. Figure 1 shows the percentage of intersection related crashes at each intersection. Figure 2 shows the breakdown of the collision type for intersection related crashes.

Table 4: Intersection Crash Summary

	TOTAL	PROP. DAMAGE	INJURIES	FATALITIES	ANGLE	UNKNOWN	HEAD-ON	FIXED-OBJ.	REAR-END	SIDESWIPE - SAME DIR.	SIDE SWIPE OPP. DIR.
<b>SEGMENT 1</b>											
COUNTY ROAD K	20	9	11	0	13	0	1	3	2	0	1
WISCONSIN AMERICAN	2	1	1	0	2	0	0	0	0	0	0
<b>SEGMENT 2</b>											
UU	6	2	4	0	3	0	0	1	2	0	0
HILLTOP DR	1	0	1	0	1	0	0	0	0	0	0
WHISPERING SPRINGS BLVD	1	0	1	0	0	0	0	0	1	0	0
<b>SEGMENT 3</b>											
SEVEN HILLS RD	2	0	2	0	1	0	0	0	1	0	0
POPLAR RD	1	0	1	0	0	0	0	0	1	0	0
COUNTY ROAD W	9	5	4	0	2	1	0	0	4	2	0
HINN RD	0	0	0	0	0	0	0	0	0	0	0
RICHARDS RD	1	1	0	0	0	0	0	0	0	1	0
LOG TAVERN RD	0	0	0	0	0	0	0	0	0	0	0
TAFT RD	0	0	0	0	0	0	0	0	0	0	0
TOWER RD.	1	0	1	0	0	0	0	0	1	0	0
TOTALS	44	18	24	0	22	1	2	4	12	3	1

Figure 1: Percentage of Crashes at Intersections

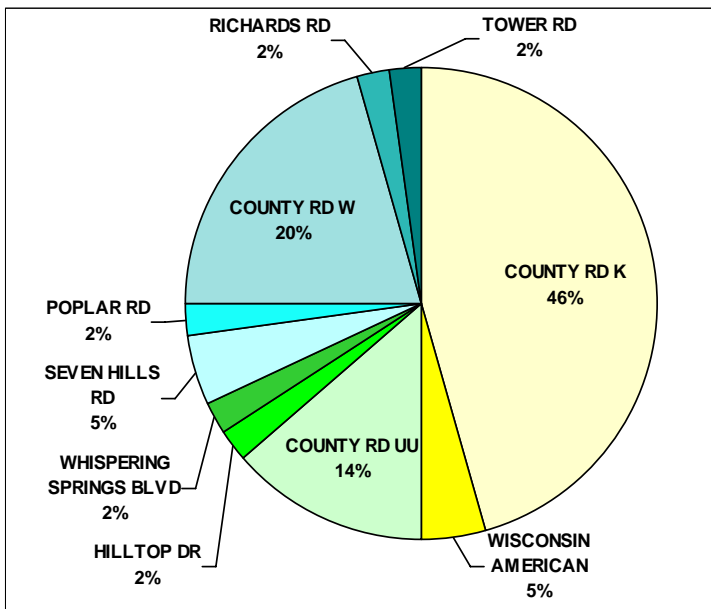
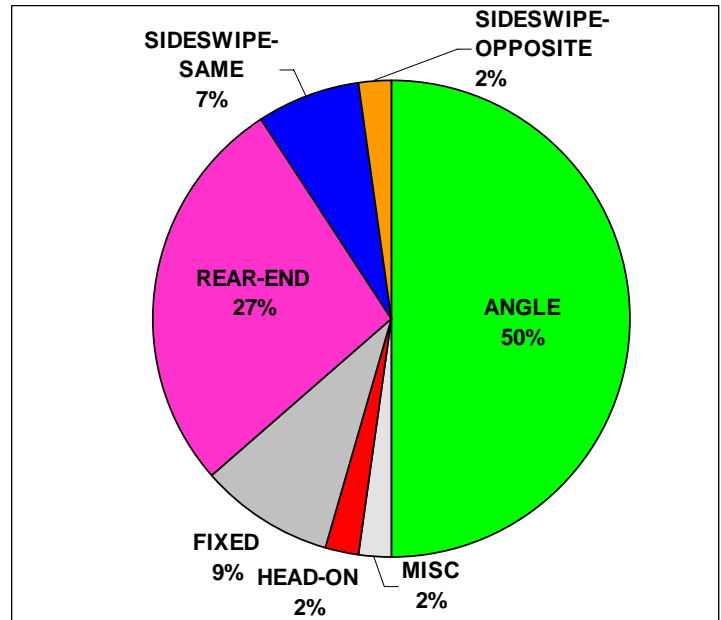


Figure 2: Collision Type at Intersections





## Non-Intersection Related Crashes

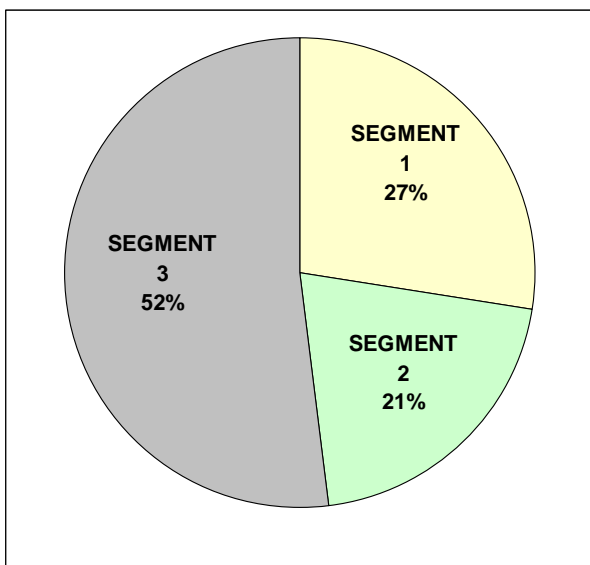
Non-Intersection related crashes were classified into segments of the roadway in which they occurred. Non-Intersection related crashes accounted for 66 percent of the total crashes. Collisions with fixed objects accounted for 43 percent of the non-intersection related crashes. 54 percent of these crashes occurred while it was snowing or raining.

See Appendix D for map of non-intersection related crashes locations. Table 5 summarizes crashes based on severity and crash type. Figure 3 shows the percentage of crashes per Segment. Figure 4 shows the collision type percentage for non-intersection related crashes.

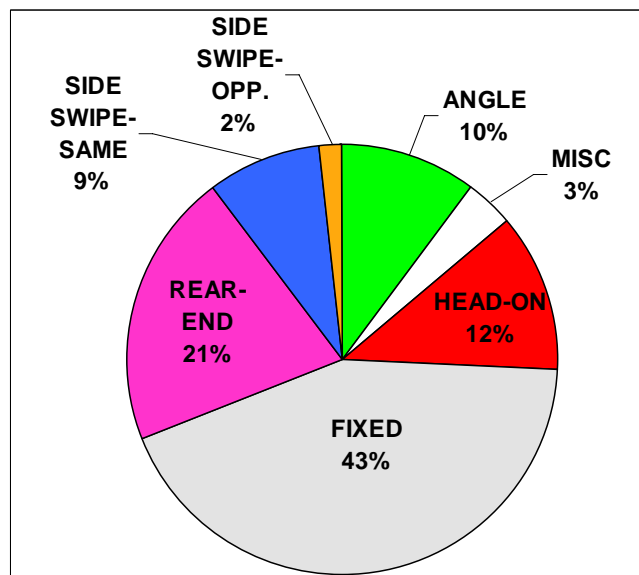
**Table 5: Non-Intersection Collision Types by Segment**

SEGMENT	TOTAL	PROP DAMAGE	INJURY	FATAL	ANGLE	HEAD-ON	FIXED OBJECT	REAR-END	SIDE SWIPE SAME	SIDE SWIPE OPPOSITE
1	6	3	3	0	1	0	2	4	1	0
2	13	8	5	0	2	2	4	4	1	0
3	39	19	20	0	3	5	21	4	3	1
<b>TOTALS</b>	<b>58</b>	<b>30</b>	<b>28</b>	<b>0</b>	<b>6</b>	<b>7</b>	<b>25</b>	<b>12</b>	<b>5</b>	<b>1</b>

**Figure 3: Percentage of Non-Intersection crashes by segment**



**Figure 4: Collision Types for Non-Intersection Related Crashes**



### III. Crash Rates

Crash rates for each segment were computed for total number of crashes, crashes resulting in an injury and crashes resulting in a fatality. These rates were then compared to the statewide crash rate for comparable rural State trunk highways. The statewide crash rate is expressed as number of crashes per hundred million vehicle miles traveled. Crash rates were computed by dividing the total number of crashes by the VMT. Table 6 summarizes the crash rates by year, segment and severity of crash. Values above the statewide average are underlined and bold.

**Table 6: Crash Rates Comparison to Statewide Average for Rural STH**

YEAR	TOTAL			INJURY			FATALITY		
	STATE	SEG2	SEG3	STATE	SEG2	SEG3	STATE	SEG2	SEG3
2001	104	42	53	42	42	27	2	0	0
2002	106	61	37	42	41	21	2	0	0
2003	117	80	89	46	40	<b><u>52</u></b>	2	0	0
2004	121	<b><u>138</u></b>	67	47	<b><u>59</u></b>	31	2	0	0
2005	115	97	31	43	39	15	2	0	0
AVG	113	84	55	44	<b><u>44</u></b>	29	2	0	0

There was a significant decrease in the accident rates between 2003 and 2004.

Segment 1 is a 0.7 mile segment that includes an interchange in a semi-urban area. There are no statewide averages for a segment of this type. Due to this and its length being less than one mile, which is considered the minimum length appropriate for comparison, this segment was not included in this section of the report.

## **IV. Conclusion**

There were 102 crashes along STH 23 from 2001 thru 2005. Crashes involving property damage accounted for 47 percent of the total while crashes resulting in personal injury accounted for 53 percent. There were no fatalities during this time period. A review of the database (minus deer related crashes) indicated the following:

- Segment 3, the longest segment, was responsible for 51 percent of the total number of crashes, 74 percent of which were non-intersection related.
- Intersection related crashes accounted for 44 percent of the total crashes, 52 percent of those were angle type crashes.
- Non-intersection related crashes that were not from a collision with a fixed object are concentrated near the intersections and driveways. See Appendix D for map showing non-intersection related crash locations.

### **SEGMENT 1**

- There were 28 crashes (29 percent of total) during the study period.
- 80 percent of crashes were intersection related crashes, 71 percent of which occurred near the County Road K intersection.
- 43 percent of the crashes were angle type crashes at the County Road K intersection.
- 2001-2003 the segment averaged 9 crashes per year, after 2003 the segment only averaged 1.5 crashes.

### **SEGMENT 2**

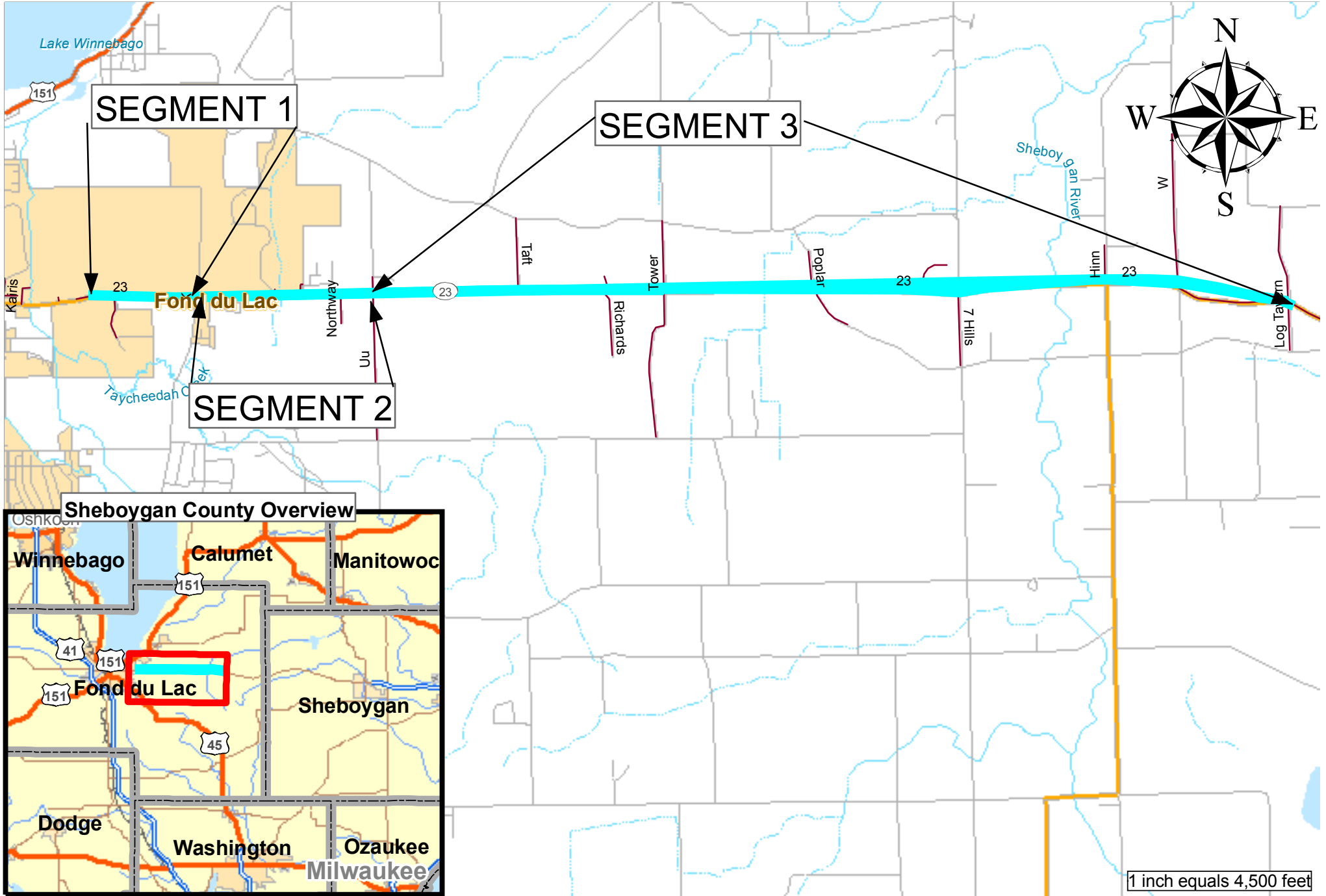
- There were 21 crashes (20 percent of total) during the study period.
- 48 percent of crashes in Segment 2 occurred near the County Road UU intersection.
- 2001-2003 the segment averaged 3 crashes per year, after 2003, 6 crashes.
- In 2004 the segment had crashes rates higher than the statewide average for crashes and crashes with injury.

### **SEGMENT 3**

- There were 53 crashes (51 percent of total) during the study period.
- 40 percent of crashes were with fixed objects such as signs, the ditch, light poles or trees.
- 9 crashes occurred at the County Road W intersection, four of which were rear-end type crashes.
- In 2003 the injury crash rate was higher than the statewide average.

**APPENDIX A**  
**PROJECT LOCATION MAP**

# Project Location Map



**APPENDIX B**

**AADT & CRASH  
RATE COMPUTATIONS**

# AADT & Crash Rate Computations

## ADT CALCULATOR

KNOWN TRAFFIC	<u>151-K</u>	<u>K-UU</u>	<u>UU-LOG TAVERN</u>	
START YEAR	<u>2000</u>	<u>2000</u>	<u>2000</u>	FROM TRAFFIC COUNTS BOOK 2002
ADT	13600	10200	8100	
END YEAR	<u>2006</u>	<u>2006</u>	<u>2006</u>	FROM TRAFFIC COUNTS COMPLETED IN 2006
ADT	13600	11425	8600	
<u>GROWTH RATE =</u>	0.00%	1.91%	1.00%	
<u>F=P(1+i)^n</u>				

## ADT PROJECTIONS

$$\text{MILLION VEHICLE MILES (MVM)} = (\text{ADT} \times 365 \times \text{LENGTH}) / 1,000,000$$

### 151-CTH K

	0.70		MILES		
YEAR	2001	2002	2003	2004	2005
ADT	13600	13600	13600	13600	13600
MVM	3.5	3.5	3.5	3.5	3.5
<b>TOTAL</b>	<b>143.5</b>	<b>315.6</b>	<b>258.2</b>	<b>28.7</b>	<b>57.4</b>
<b>INJ</b>	<b>86.1</b>	<b>172.1</b>	<b>143.5</b>	<b>0.0</b>	<b>28.7</b>
<b>FATL</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>

### CTH K - CTH UU

	1.26		MILES		
YEAR	2001	2002	2003	2004	2005
ADT	10395	10593	10795	11001	11211
MVM	4.8	4.9	5.0	5.1	5.2
<b>TOTAL</b>	<b>41.7</b>	<b>61.4</b>	<b>80.3</b>	<b>137.9</b>	<b>96.6</b>
<b>INJ</b>	<b>41.7</b>	<b>40.9</b>	<b>40.1</b>	<b>59.1</b>	<b>38.7</b>
<b>FATL</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>

### UU-LOG TAVERN

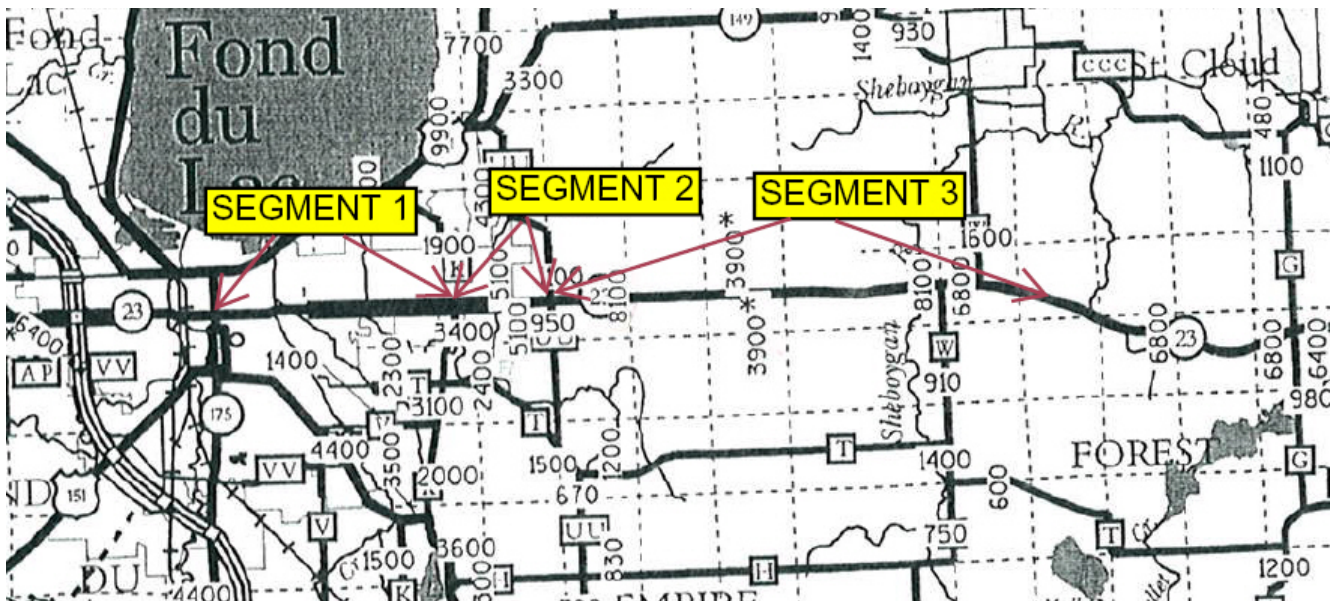
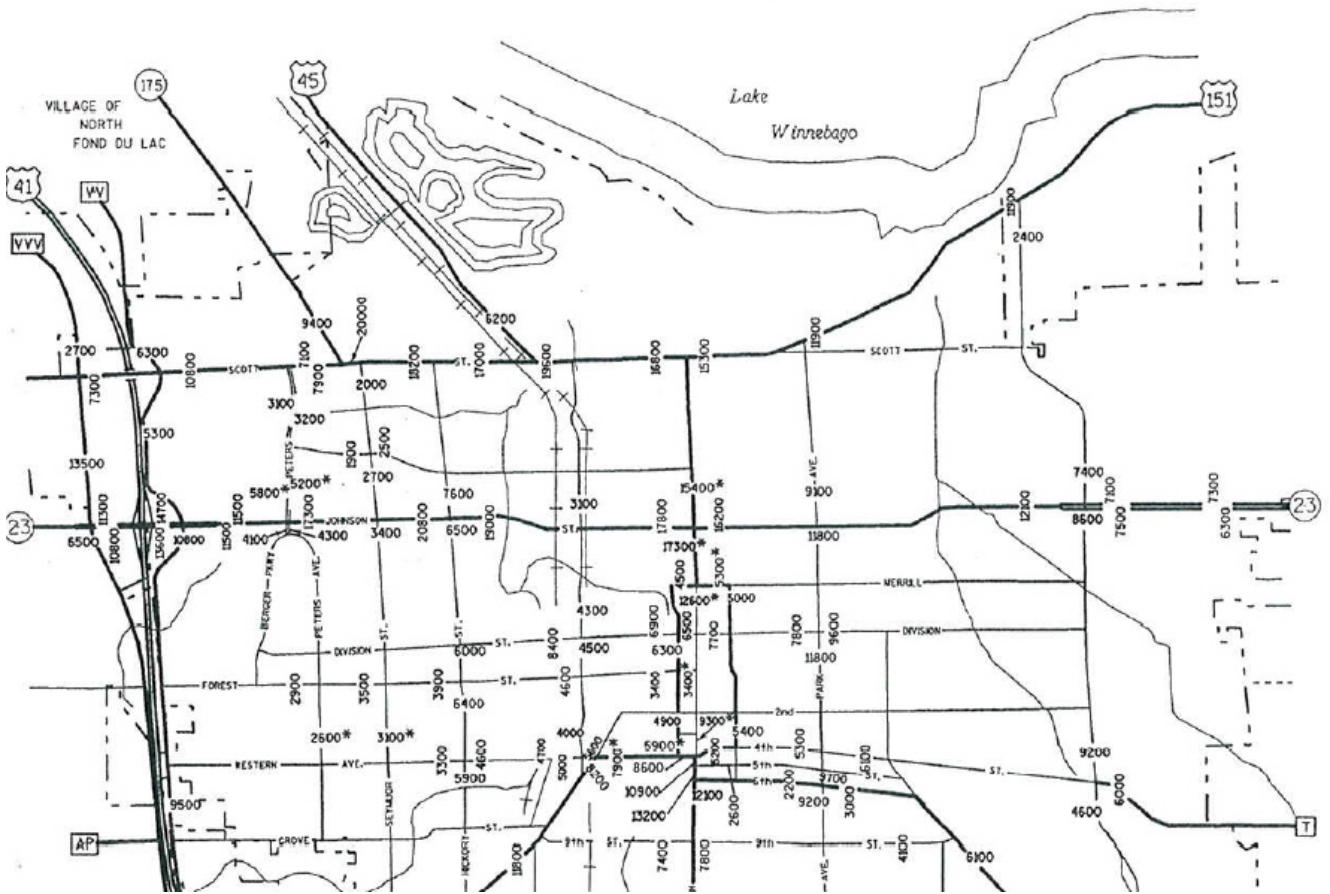
	6.29		MILES		
YEAR	2001	2002	2003	2004	2005
ADT	8181	8263	8346	8430	8515
MVM	18.8	19.0	19.2	19.3	19.5
<b>TOTAL</b>	<b>53.3</b>	<b>36.9</b>	<b>88.7</b>	<b>67.2</b>	<b>30.7</b>
<b>INJ</b>	<b>26.6</b>	<b>21.1</b>	<b>52.2</b>	<b>31.0</b>	<b>15.4</b>
<b>FATL</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>	<b>0.0</b>

# **APPENDIX C**

## **WISCONSIN TRAFFIC VOLUME DATA**



**From 2002 Wisconsin Traffic Counts**



## From Wisconsin Department of Transportation

<b>TRAFFIC FORECAST REPORT</b>	<b>REGION/COUNTY(IES):</b> Northeast Fond du Lac Co.
<b>PROJECT ID(S):</b> 1440-13-00	<b>LOCATION:</b> USH 151 to CTH "UU"
<b>ROUTE(S):</b> STH 23	<b>COMPLETED:</b> February 16, 2006

Traffic Forecasting Section, Bureau of State Highway Programs, Division of Transportation Investment Management

Developed by: Bill Gavinski  
 Phone: (608) 266-3976  
 FAX #: (608) 2367-0294  
 E-Mail ID: william.gavinski@dot.state.wi.us

**Design Values (%'s)**

ROUTE(S):	STH 23		
<b>Design Volume(s):</b>			
K100	10.2		
K30	11.2		
F(PHV)	14.2		
T(DHV)	9.3		
T(PHV)	7.3		
D (Dsgn hr)	55-45		

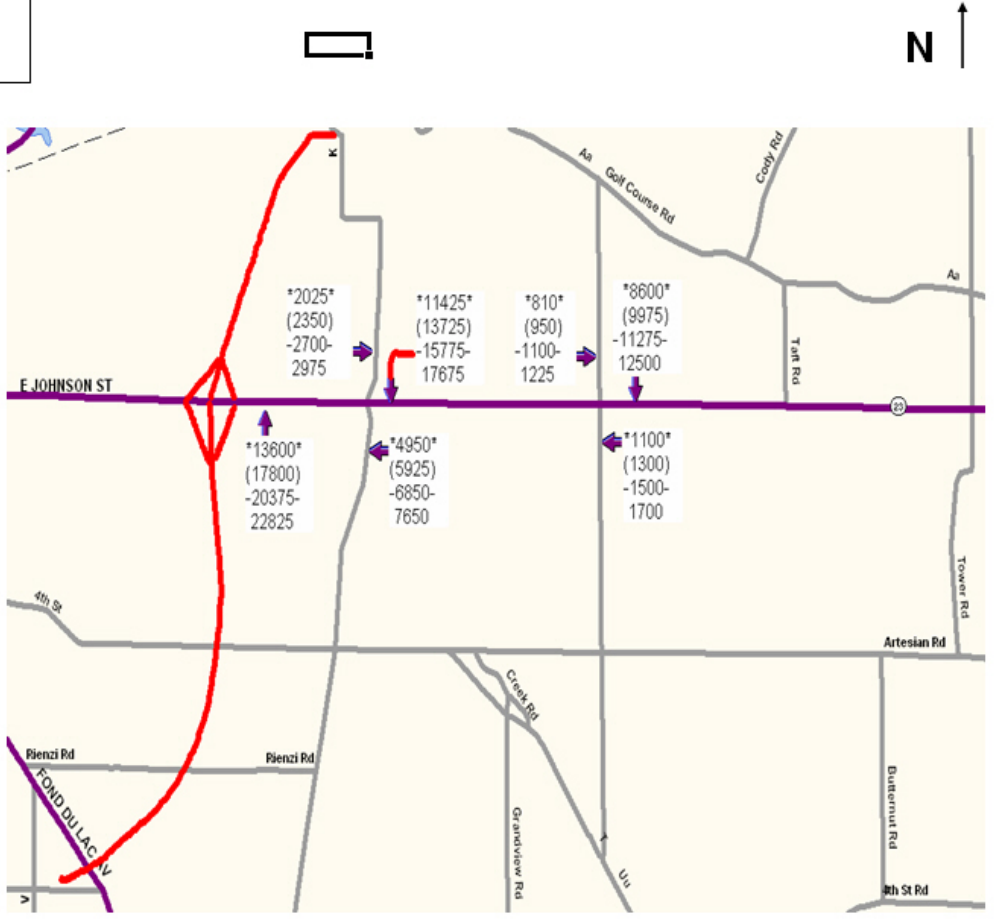
**Truck Class %'s**

Truck Class	Seg. 1	Seg. 2	Seg. 3
2D	3.4		
3AX	2.0		
2S1+2S2	2.3		
3-S2	5.5		
DBL-BTM	0.7		
<b>TOTAL</b>	<b>13.3%</b>		

Specify Last Count & Forecast Years:  
 \*000\* Last Traffic Count  
 (000) 2015 AADT  
 -000- 2025 AADT  
 000 20235 AADT

**Notes on the Forecast:**

- Mainline volumes were forecasted using historical traffic count data from the coverage count stations located in or adjacent to the project area. The Fond du Lac urban transportation model was used to forecast traffic and diversions due to highway facility modifications.
- Truck classification is based upon the vehicle classification stlocated on STH 23 (Sta# 206104).
- Design data based upon similar highway facilities which have similar traffic volumes.
- It is assumed that there will be some new traffic generators located along this highway facility during the forecast period.



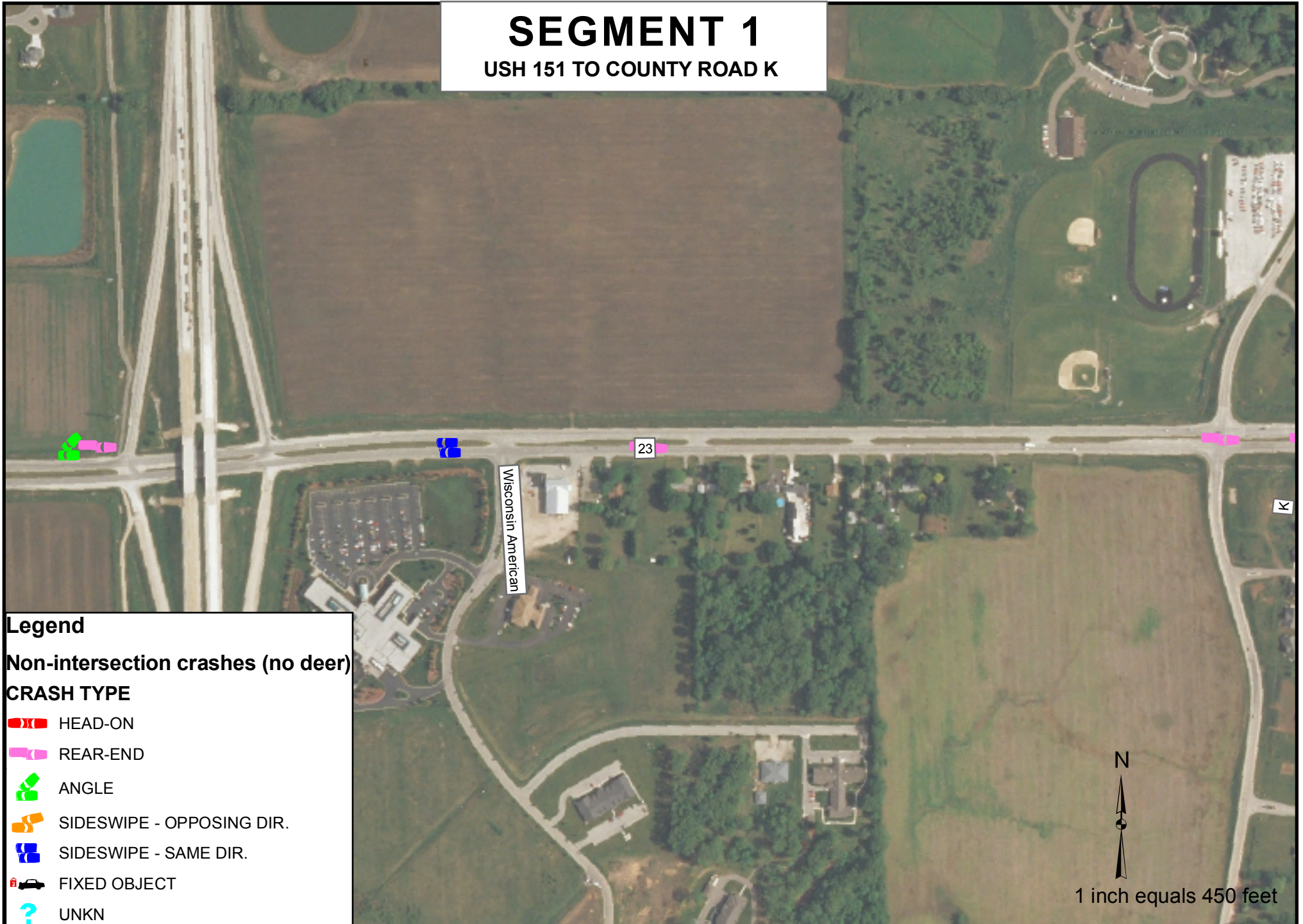
**APPENDIX D**  
**CRASH MAPS**



# STH 23 NON-INTERSECTION RELATED CRASHES

## SEGMENT 1








USH 151 TO COUNTY ROAD K



### Legend

Non-intersection crashes (no deer)

#### CRASH TYPE

-  HEAD-ON
-  REAR-END
-  ANGLE
-  SIDESWIPE - OPPOSING DIR.
-  SIDESWIPE - SAME DIR.
-  FIXED OBJECT
-  UNKN

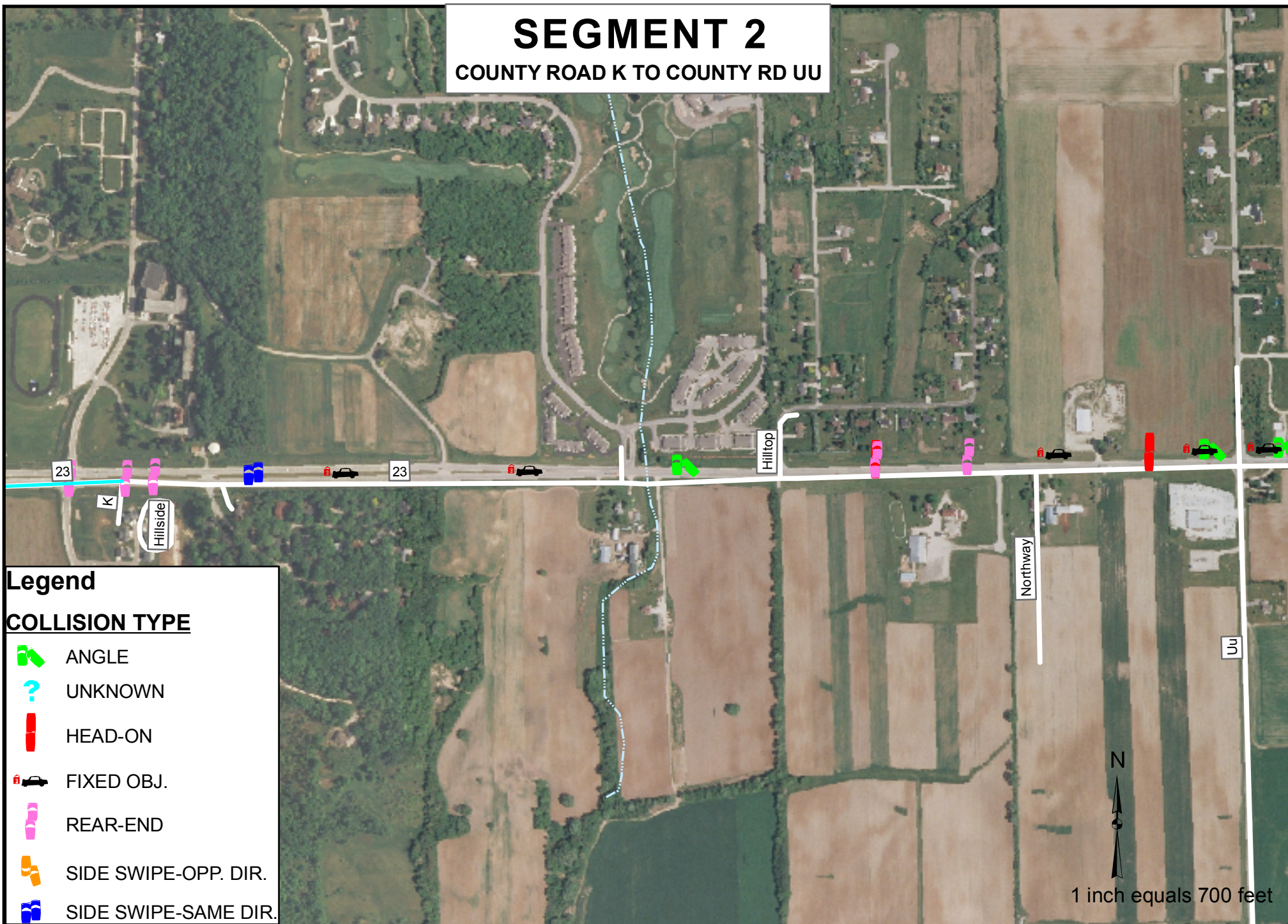


1 inch equals 450 feet



# STH 23 NON-INTERSECTION RELATED CRASHES

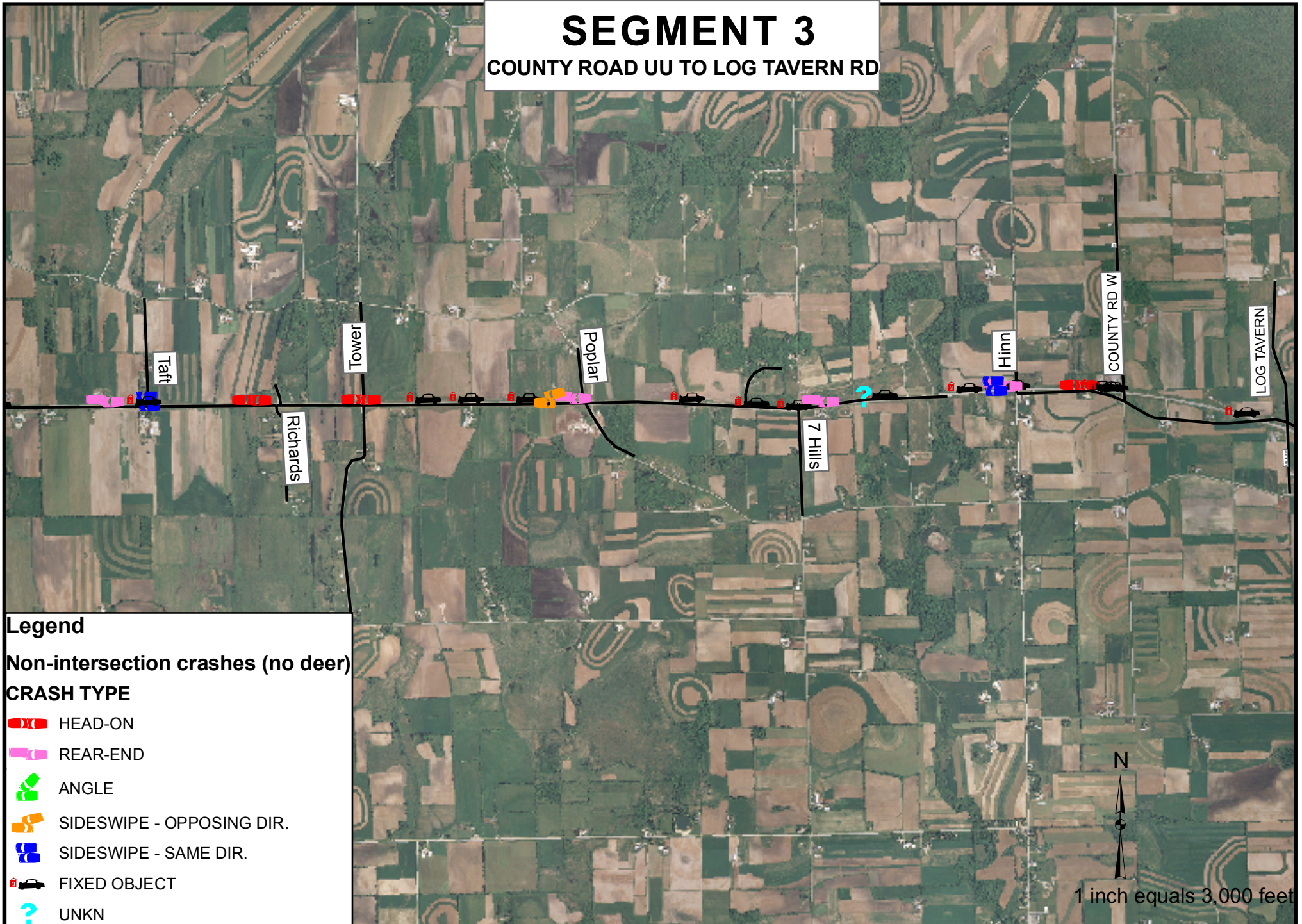
## SEGMENT 2 COUNTY ROAD K TO COUNTY RD UU





# STH 23 NON-INTERSECTION RELATED CRASHES

## SEGMENT 3 COUNTY ROAD UU TO LOG TAVERN RD



**APPENDIX E**

**INTERSECTION  
RELATED CRASH DATA**

SEGMENT #	INT_NAME	RLTNRD WY	ONHWY	ONHWY DIR	INT DIR	INT DIS	ACCDLOC	ALCFLAG	ROADCOND	WTHRCOND	LGTCOND	ACCD SVR	TOT INJ	TOT FATAL	ACCD TYPE	MNR COLL	TOT VEH	DRVR PC 1A	POST SPD 1	AGE 1	DRVR PC 2A	POST SPD 2	AGE 2	M CFL NMBR	COLLSN TYPE	REAR	HEAD	ANGL	SSOP	SSS	FIXED	MISC	WEATH	ACTIONS	Year									
1	K	ON	23	E		0	I					INJ	4	0		ANGL	3	45	59	FTY	35	85	01442041312	ANGL	-	-	1	-	-	-	-	-	-	EB GO STR  SB GO STR	2001									
1	K	ON	23	E		0	I		CLDY			INJ	2	0		HEAD	2	35	17		45	19	01351520285	HEAD	-	1	-	-	-	-	-	-	-	-	EB LT TRN  WB GO STR	2001								
1	K	ON	23	W		0	I					INJ	1	0		ANGL	2	55	31		45	53	01512360441	ANGL	-	-	1	-	-	-	-	-	-	-	-	NB GO STR  WB GO STR	2001							
1	K	ON		E		0	I		WET	CLDY		PD	0	0		REAR	2	35	52		35	45	01130510697	REAR	1	-	-	-	-	-	-	-	-	-	-	1	NB BACKNG  NB STOPED	2001						
1	K	ON	K	E		0	I			CLDY		PD	0	0		ANGL	2	35	49		45	52	01130500420	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	SB GO STR  EB GO STR	2001						
1	K	ON	23	E		0	I					INJ	1	0	BIKE	NO C	1	35	19		45	34	02542800180	FIXED	-	-	-	-	-	1	-	-	-	-	-	-	NB GO STR HIT BIKE  WB GO STR	2002						
1	K	ON	23	E		0	I		CLDY		DARK	PD	0	0		ANGL	2	35	30		45	59	02623290804	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	NB GO STR  EB GO STR	2002					
1	K	ON	23	W	E	1	I		WET	BLNK		INJ	1	0		REAR	3	45	39		45	58	02351760321	REAR	1	-	-	-	-	-	-	-	-	-	-	-	-	WB GO STR  WB STOPED	2002					
1	K	ON	23	E		0	I					PD	0	0		ANGL	2	55	16		45	42	02542800162	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	NB GO STR  EB GO STR	2002				
1	K	ON	23	W		0	I				DARK	INJ	2	0		ANGL	2	45	20	FTY	35	59	02281402641	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	WB GO STR  NB GO STR	2002				
1	K	ON	23	W		0	I		CLDY			PD	0	0		ANGL	2	45	23		45	17	02341700109	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	1	NB LT TRN  WB GO STR	2002			
1	K	RTSH	23	E		0	I	Y	CLDY		DARK	INJ	1	0	DITCH	NO C	1	45	40		.	.	02100422824	FIXED	-	-	-	-	-	1	-	-	-	-	-	-	-	-	EB LT TRN HIT DITCH	2002				
1	K	ON	23	E		0	I		CLDY			PD	0	0		ANGL	2	35	64		45	37	03613210982	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	NB GO STR  EB GO STR	2003			
1	K	ON	23	E		0	I					PD	0	0		ANGL	2	45	37	FTY	45	16	03050300044	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	-	SB GO STR  NB LT TRN	2003		
1	K	ON	23	E		0	I		WET	RAIN		INJ	2	0		ANGL	2	45	20		45	51	03633290549	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	1	SB GO STR  EB GO STR	2003		
1	K	ON	23	W		0	I					INJ	2	0		ANGL	3	35	19		45	38	03080450925	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	1	SB GO STR  EB GO STR	2003		
1	K	ON	23	E		0	I		CLDY			INJ	3	0		ANGL	2	45	26	FTY	45	81	03321811108	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	-	EB GO STR  SB GO STR	2003		
1	K	ON	23	E		0	I		WET	CLDY		PD	0	0		ANGL	2	45	28		45	27	04652720532	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	-	1	WB LT TRN  EB GO STR	2004	
1	K	ON	23	W		0	I		SNOW	SNOW	LIGT	PD	0	0	GR FAC	NO C	1	45	63		.	.	05003491018	FIXED	-	-	-	-	-	1	-	-	-	-	-	-	-	-	-	-	1	NB RT TRN HIT GR FAC	2005	
1	K	ON	23	E		0	I		CLDY			INJ	7	0		SSOP	3	35	35		45	45	05001390234	SSOP	-	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	1	SB GO STR  EB GO STR	2005	
1	WISCONSIN AMERI	ON	23	E		0	I		CLDY		LIGT	INJ	1	0		ANGL	2	45	54		45	23	03181010400	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	-	1	WB LT TRN  EB GO STR	2003	
1	WISCONSIN AMERI	ON	23	E		0	I					PD	0	0		ANGL	2	25	16		45	61	03120700781	ANGL	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1	NB LT TRN  EB GO STR	2003
							I					PD	13												INT.	1	-	13	1	-	3	-	4											



SEGMENT #	INT_NAME	RLTNRD WY	ONHWY	ONHWY DIR	INT DIR	INT DIS	ACCDLOC	ALCFLAG	ROADCOND	WTHRCOND	LGTCOND	ACCD SVR	TOT INJ	TOT FATAL	ACCD TYPE	MNRCOLL	TOT VEH	DRV R PCA	POST SPD 1	AGE 1	DRV R PCA	POST SPD 2	AGE 2	M CFL NMBR	COLLSN TYPE	REAR	HEAD	ANGL	SSOP	SSS	FIXED	MISC	WEATH	ACTIONS	Year
2	HILLTOP DR	ON	23	E		0	I			CLDY		INJ	1	0		ANGL	2		55	49	FTY	55	47	04451910331	ANGL	-	-	1	-	-	-	-	-	EB GO STR   SB GO STR	2004
2	UU	ON	23	E		0	I					INJ	2	0		REAR	2	ID	55	47		55	49	01190850456	REAR	1	-	-	-	-	-	-	EB GO STR   EB STOPED	2001	
2	UU	ON	23	E		0	I	WET		CLDY		INJ	3	0		ANGL	2	FTY	55	23		55	71	02050181034	ANGL	-	-	1	-	-	-	-	1	NB GO STR   EB GO STR	2002
2	UU	ON	23	E		0	I	WET		CLDY		INJ	2	0		ANGL	2	FTY	55	18		55	22	03160910632	ANGL	-	-	1	-	-	-	-	1	NB LT TRN   EB GO STR	2003
2	UU	RTSH	23	E	W	1	I					PD	0	0	DITCH	NO C	1	ID	55	28		.	.	03462580144	FIXED	-	-	-	-	-	1	-	-	WB GO STR   HIT DITCH	2003
2	UU	ON	23	E		0	I					PD	0	0		ANGL	2		55	61	FTY	55	49	05000840529	ANGL	-	-	1	-	-	-	-	EB GO STR   NB GO STR	2005	
2	UU	ON	23	E		0	I	WET	SNOW			INJ	1	0		REAR	2	FTC	55	17		55	16	05003611286	REAR	1	-	-	-	-	-	-	EB GO STR   EB LT TRN	2005	
2	WHISPERING SPRI	ON	23	E	E	1	I					INJ	2	0		REAR	2	FTC	45	40		45	53	05002270241	REAR	1	-	-	-	-	-	-	EB GO STR   EB STOPED	2005	
							I					PD	10												INT.	3	-	4	-	-	1	-	3		

SEGMENT #	INT_NAME	RLTNRD	ONHWY	ONHWYDIR	INTDIR	INTDIS	ACCDLOC	ALCFLAG	ROADCOND	WTHRCOND	LGTCOND	ACCDSEVR	TOTINJ	TOTFATL	ACCDTYPE	MNRCOLL	TOTVEH	DRVRPCA	POSTSPD1	AGE1	DRVRPCA	POSTSPD2	AGE2	MCFLNMBR	COLLSN-TYPE	REAR	HEAD	ANGL	SSOP	SSS	FIXED	MISC	WEATH	ACTIONS	Year
3	POPLAR RD	ON	23	E	E	2	I		SNOW	SNOW		INJ	1	0		REAR	2	55	22	FTC	55	16	04060272265	REAR	1	-	-	-	-	-	-	1	WB SL/ST  WB GO STR	2004	
3	RICHARDS RD	ON	23	E		0	I	Y	WET	RAIN	DARK	PD	0	0		SSS	2	55	33	IO	55	33	01733460506	SSS	-	-	-	-	1	-	-	1	WB LT TRN  WB OVT LT	2001	
3	SEVEN HILLS RD	SHLD	23	E		0	I		WET			INJ	1	0		REAR	2	55	28	FTC	55	53	03060341783	REAR	1	-	-	-	-	-	-	1	EB OTHER  EB OVT RT	2003	
3	SEVEN HILLS RD	ON	23	E	E	1	I					INJ	1	0		ANGL	2	55	48		55	56	04632641620	ANGL	-	-	1	-	-	-	-	-	EB OVT RT  WB GO STR	2004	
3	TOWER RD	ON	23	E		0	I			CLDY		INJ	3	0		REAR	3	55	22		55	44	01251141023	REAR	1	-	-	-	-	-	-	-	EB GO STR  EB STOPED	2001	
3	W	ON	23	E	W	5	I		SNOW	WIND		INJ	1	0		REAR	2	55	27		55	30	01060231698	REAR	1	-	-	-	-	-	-	1	WB GO STR  WB GO STR	2001	
3	W	ON	23	E		0	I			CLDY		PD	0	0		NO C	3	55	43		55	71	01241070574	MISC	-	-	-	-	-	-	1	-	WB LT TRN  WB GO STR	2001	
3	W	ON	23	E		0	I		WET	RAIN		INJ	1	0		REAR	2	55	32	TFC	55	71	03351951301	REAR	1	-	-	-	-	-	-	1	EB LT TRN  EB LT TRN	2003	
3	W	ON	23	E	E	20	I					INJ	1	0		REAR	2	55	46		55	79	03281560575	REAR	1	-	-	-	-	-	-	-	EB GO STR  EB SL/ST	2003	
3	W	ON	W	E		0	I			CLDY	DUSK	INJ	1	0		REAR	2	55	17		55	26	03281560577	REAR	1	-	-	-	-	-	-	-	SB BACKNG  SB STOPED	2003	
3	W	ON	23	E		0	I					PD	0	0		ANGL	2	55	18		55	17	03542930965	ANGL	-	-	1	-	-	-	-	-	SB GO STR  WB SL/ST	2003	
3	W	ON	23	E		0	I					PD	0	0		ANGL	2	55	56		55	25	03462580154	ANGL	-	-	1	-	-	-	-	-	EB LT TRN  EB GO STR	2003	
3	W	ON	23	E		0	I			LIGT		PD	0	0		SSS	2	55	16		55	48	03351951369	SSS	-	-	-	-	1	-	-	-	WB LT TRN  WB GO STR	2003	
3	W	ON	23	E		0	I			CLDY		PD	0	0		SSS	2	55	48		55	38	05002100580	SSS	-	-	-	-	1	-	-	-	NB RT TRN  WB GO STR	2005	
							I					PD	25											INT.	7	-	3	-	3	-	1	5			

**APPENDIX F**

**NON-INTERSECTION  
RELATED CRASH DATA**

SEGMENT #	INT_NAME	RLTNRD WY	ONHWY	ONHWY DIR	INT DIR	INT DIS	ACC DLOC	ALCF LAG	ROADCOND	WTHRCOND	LGTCOND	ACCD SVR	TOT INJ	TOT FATAL	ACCD TYPE	MNR COLL	TOT VEH	DRVR PC 1A	POST SPD 1	AGE 1	DRVR PC 2A	POST SPD 2	AGE 2	M CFL NMBR	COLLSN -TYPE	REAR	HEAD	ANGL	SSOP	SSS	FIXED	MISC	WEATH	ACTIONS	Year
1	K	ON	23	W	W	6	N			CLDY	DARK	INJ	1	0		REAR	2	FTC	45	17		45	16	02351760307	REAR	1	-	-	-	-	-	-	-	WB GO STR  WB SL/ST	2002
1	WISCONSIN AMERI	ON	23	E	W	30	N					INJ	1	0		ANGL	2		45	58	FTY	77	24	02613191505	ANGL	-	-	1	-	-	-	-	-	EB GO STR  NB GO STR	2002
1	WISCONSIN AMERI	ON	23	W	W	6	N					PD	0	0		REAR	2	ID	45	41		45	16	03020090350	REAR	1	-	-	-	-	-	-	-	WB GO STR  WB STOPED	2002
1	WISCONSIN AMERI	ON	23	E	E	9	N			CLDY		PD	0	0		REAR	2	FTC	45	47		45	42	02542800202	REAR	1	-	-	-	-	-	-	-	EB GO STR  EB RT TRN	2002
1	WISCONSIN AMERI	BLNK	23	E	W	4	N	BLNK	BLNK			PD	0	0		SSS	2	ID	3	36		35	16	03362020530	SSS	-	-	-	-	1	-	-	-	WB CHG LN  WB GO STR	2003
1	WISCONSIN AMERI	ON	23	E	W	28	N					INJ	1	0		REAR	2	ID	45	42		45	60	03301670194	REAR	1	-	-	-	-	-	-	-	EB GO STR  EB SL/ST	2003
							N																	NON-INT	4	-	1	-	1	-	-	-			

SEGMENT #	INT_NAME	RLTNRD WY	ONHWY	ONHWY DIR	INT DIR	INT DIS	ACCDLOC	ALCFLAG	ROADCOND	WTHRCOND	LGTCOND	ACCD SVR	TOT INJ	TOT FATAL	ACCD TYPE	MNR COLL	TOT VEH	DRV R PCA	POST SPD 1	AGE 1	DRV R PCA	POST SPD 2	AGE 2	M CFL NMBR	COLLSN - TYPE	REAR	HEAD	ANGL	SSOP	SSS	FIXED	MISC	WEATH	ACTIONS	Year	
2	HILLTOP DR	ON	23	E	E	40	N					INJ	2	0		REAR	2		55	16	ID	55	57	01321380148	REAR	1	-	-	-	-	-	-	-	EB SL/ST  EB GO STR	2001	
2	HILLTOP DR	ON	23	E	E	10	N	Y				INJ	1	0		HEAD	2		55	32	DC	55	65	04662780502	HEAD	-	1	-	-	-	-	-	-	WB BACKNG  WB GO STR	2004	
2	HILLTOP DR	ON	23	E	E	20	N		SNOW	SNOW		INJ	3	0		REAR	2	FVC	45	19		45	21	04110470845	REAR	1	-	-	-	-	-	-	-	WB GO STR  ?B GO STR	2004	
2	HILLTOP DR	ON	23	E	E	10	N					PD	0	0		REAR	2	ID	55	21		55	43	05002000717	REAR	1	-	-	-	-	-	-	-	WB GO STR  WB STOPED	2005	
2	K	ON	23	E	E	30	N				DARK	PD	0	0	OBNFX	NO C	1		45	23		.	.	02663510434	FIXED	-	-	-	-	-	1	-	-	WB GO STR HIT OBNFX	2002	
2	K	RTSH	23	E	E	50	N		ICE	SNOW		PD	0	0	OVRTRN	NO C	1		45	34		.	.	04003520180	FIXED	-	-	-	-	-	1	-	-	EB GO STR HIT OVRTRN	2004	
2	K	ON	23	E	E	20	N					PD	0	0		SSS	2	IO	45	56		45	59	04251050134	SSS	-	-	-	-	1	-	-	1	WB NPASZN  WB GO STR	2004	
2	K	ON	23	W	E	9	N		ICE	SLET		PD	0	0		REAR	2	TFC	45	43		45	47	05000280356	REAR	1	-	-	-	-	-	-	1	WB GO STR  WB GO STR	2005	
2	UU	ON	23	E	W	3	N		ICE	SNOW		INJ	2	0		ANGL	2		55	48	TFC	55	57	02221020545	ANGL	-	-	1	-	-	-	-	-	1	WB GO STR  EB GO STR	2002
2	UU	RTSH	23	E	W	4	N		ICE	CLDY		INJ	1	0	DITCH	NO C	1	TFC	55	23		.	.	03211150199	FIXED	-	-	-	-	-	1	-	-	1	EB GO STR HIT DITCH	2003
2	UU	RTSH	23	E	W	20	N		SNOW	SNOW		PD	0	0	EMBKMT	NO C	1	TFC	55	58		.	.	04010051741	FIXED	-	-	-	-	-	1	-	-	1	WB GO STR HIT EMBKMT	2003
2	UU	ON	23	E	W	10	N		SNOW	WIND		PD	0	0		HEAD	2	TFC	55	17		55	25	04110480477	HEAD	-	1	-	-	-	-	-	-	1	WB GO STR  EB GO STR	2004
2	WHISPERING SPRI	ON	23	E	E	10	N					PD	0	0		ANGL	2	ID	45	18		45	49	04582430915	ANGL	-	-	1	-	-	-	-	-	-	WB GO STR  WB GO STR	2004
							N																		NON-INT	4	2	2	-	1	4	-	7			





# INTERSECTION LOCATION - STH 23 EB & WB JUB HANDLE & CTH UU

Project ID - 144D-15-70 Intersection Location – STH 23 EB & WB Jug Handle & CTH UU	
Factor	ALTERNATIVE CONTROL <input type="checkbox"/> TRAFFIC SIGNAL, <input type="checkbox"/> ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> 2-WAY STOP <input type="checkbox"/> EXISTING CONTROL
Safety	ALTERNATIVE CONTROL <input type="checkbox"/> TRAFFIC SIGNAL, <input type="checkbox"/> ANTICIPATING TRAFFIC SIGNAL <input type="checkbox"/> ROUNDABOUT <input type="checkbox"/> 4-WAY STOP <input checked="" type="checkbox"/> 2-WAY STOP <input type="checkbox"/> EXISTING CONTROL
Operational Analysis	
Construction Cost	
Right-of-Way	
Practical Feasibility	
Operation & Maintenance Cost	
Environmental	
Pedestrian and Bicycles	
Recommendation	
Responsibility	PIP Team
	PDS Team
	PDS Team
	PIP Team
	PDS Team