W RO

GRE OCT 2013

ORDER OF SHEETS

Section No. 6

TOTAL SHEETS = 46

Section No. 1 Section No. 2 Typical Sections and Details Estimate of Quantities Section No. 3 Section No. 3 Miscellaneous Quantities

Section No. 8 Structure Plans

Standard Detail Drawings

STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

PLAN OF BRIDGE REHABILITATION

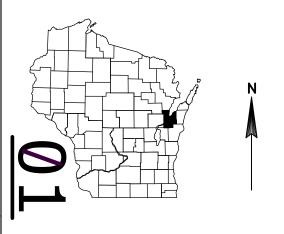
FEDERAL PROJECT STATE PROJECT PROJECT CONTRACT WISC 2014033 1220-19-72

CITY OF GREEN BAY, LEO FRIGO BRG

IRWIN AVENUE - ATKINSON DRIVE

STATE PROJECT NUMBER

IH 43 **BROWN COUNTY**



DESIGN DESIGNATION

ESALS

PI AN

Δ.Δ.D.T. (2012) = 43.000 A.A.D.T. (2032) = 57,000 D.H.V. (2032) = 5,130 = 60/40 D.D. = 8.0% DESIGN SPEED = 70 MPH

CONVENTIONAL SYMBOLS

WOODED OR SHRUB AREA

CORPORATE LIMITS PROPERTY LINE LOT LINE LIMITED HIGHWAY EASEMENT EXISTING RIGHT OF WAY PROPOSED OR NEW R/W LINE SLOPE INTERCEPT REFERENCE LINE EXISTING CULVERT PROPOSED CULVERT (Box or Pipe) COMBUSTIBLE FLUIDS MARSH AREA

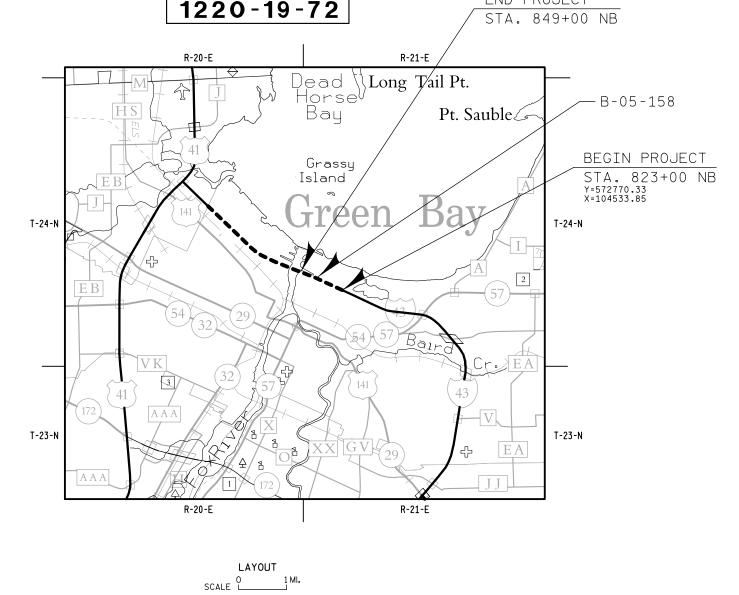
= 11,169,000

PROFILE GRADE LINE ORIGINAL GROUND MARSH OR ROCK PROFILE (To be noted as such) SPECIAL DITCH GRADE ELEVATION CULVERT (Profile View) UTILITIES

ELECTRIC FIRER OPTIC SANITARY SEWER STORM SEWER TELEPHONE UTILITY PEDESTAL POWER POLE Ġ Ø TELEPHONE POLE

_ ROCK

LABEL _ _



ORIGINAL PLANS PREPARED BY MICHAEL BAKER JR., INC. Baker 7633 GANSER WAY, STE 206, MADISON, WI 53719 HALVERSON E-34364 MADISON. 10/21/2013 (Date)

STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

PREPARED BY WISDOT / AYRES Surveyor BAKER Designer TOM BUCHHOLZ KRISSY VANHOUT Regional Examiner MIKE KING Regional Supervisor __ C.O. Examiner

APPROVED FOR THE DEPARTMENT DATE: 10/21/2013

Ε

TOTAL NET LENGTH OF CENTERLINE = 0.00 MI.

END PROJECT

GENERAL NOTES

NO TREES OR SHRUBS ARE TO BE REMOVED UNLESS INDICATED FOR REMOVAL BY THE ENGINEER.

THE LOCATION OF KNOWN EXISTING UTILITIES IN THE VICINITY OF THE PROJECT ON THE PLANS IS APPROXIMATE. THERE MAY BE OTHER UTILITIES IN THE AREA THAT ARE NOT SHOWN.

THE CONTRACTOR SHALL NOTIFY DIGGERS HOTLINE AND AFFECTED UTILITIES PRIOR TO THE START OF WORK. ANY LOCAL MUNICIPAL UTILITY WHICH IS NOT A MEMBER OF THE DIGGERS HOTLINE MUST BE CONTACTED SEPARATELY.

REMOVAL OF EROSION CONTROL DEVICES IS INCLUDED IN THE COST OF THEIR RESPECTIVE BID ITEMS.

EROSION CONTROL DEVICES SHALL BE PLACED IN SEQUENCE WITH CONSTRUCTION OPERATIONS OR AS DETERMINED BY THE ENGINEER.



Call 811 3 Work Days Before You Dig or Toll Free (800) 242-8511 Hearing Impaired TDD (800) 542-2289 www.DiggersHotline.com

UTILITY CONTACTS

MIKE OLSEN ATC - ELECTRICITY 801 O'KEEFE RD P.O. BOX 6113 DE PERE, WI 54115-6113 (920) 338-6582 molsen@atclic.com

NATHAN QUALLS
GREEN BAY METROPOLITAN SEWERAGE DISTRICT - SEWER
2231 N QUINCY ST
GREEN BAY, W154302-1248

GREEN BAY, WI 54302-124 (920) 438-1032 NQualls@newwater.us

vince.albin@twcable.com

JAMES KOSTUCH
WINDSTRAEM KDL INC. - COMMUNICATION LINE
13935 BISHOPS DRIVE
BROOKFIELD, WI 53005
(262) 792-7938
james.kostuch@windstream.com

KEVIN HUFF
QWEST COMMUNICATIONS - COMMUNICATION LINE
13057 S. MONITOR AVENUE
PALOS HEIGHTS, IL 60463
(708) 837-7927
kevin.huff@centurylink.com

VINCE ALBIN
TIME WARNER CABLE, A DELAWARE LIMITED PARTNERSHIP - COMMUNICATION LINE
3520 DESTINATION DR
APPLETON, WI 54915
(920) 831-9249

CASEY SCHWANDT
WEST SHORE PIPE LINE COMPANY - GAS/PETROLEUM
2119 NORTH QUINCY STREET
GREEN BAY, W154302
(920) 876-2462
cschwan@buckeye.com

LORIBUTRY
WISCONSIN PUBLIC SERVICE CORPORATION - ELECTRICITY
700 N ADAMS ST
P.O. BOX 19001
GREEN BAY, WI54307-9001
(920) 433-1703
LAButry@integrysgroup.com

STEVE GRENIER
CITY OF GREEN BAY - SEWER
100 N JEFFERSON ST
GREEN BAY, WI54301
(920) 448-3100
stevengr@ci.green-bay.wi.us

BRIAN POWELL
GREEN BAY WATER UTILITY - WATER
631 S ADAMS ST
GREEN BAY, WI54301
(920) 448-3480
brianpo@ci.green-bay.wi.us

WAYNE CRETTON

PACKERLAND BROADBAND - COMMUNICATION LINE 105 KENT ST P.O. BOX 190 IRON MOUNTAIN, MI 49801 (906) 282-3768 wayne.cretton@plbb.us

STEVE JAKUBIEC
TDS METROCOM - COMMUNICATION LINE
SUITE 218A
10 COLLEGE AVE
APPLETON, WI 54911
(920) 882-4166
steve.jdkubiec@tdstelecom.com

JEFF MADSON
WISCONSIN DEPARTMENT OF TRANSPORTATION - COMMUNICATION LINE
STE. 300
433 W.ST. PAUL AVE.
MILWAUKEE, WI 53203-3007
(414) 225-3723
Jeffrey.Modson@dot.wi.gov

KAREN WELLS
AT&T WISCONSIN - COMMUNICATION LINE
205 S JEFFERSON ST
GREEN BAY, WI54301
(920) 433-4226
kw9272@att.com

LORIBUTRY
WISCONSIN PUBLIC SERVICE CORPORATION - GAS/PETROLEUM
700 N ADAMS ST
P.O.BOX 19001
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(920) 433-1703
LAButry@Integrysgroup.com

<u>DNR CONTACT</u>

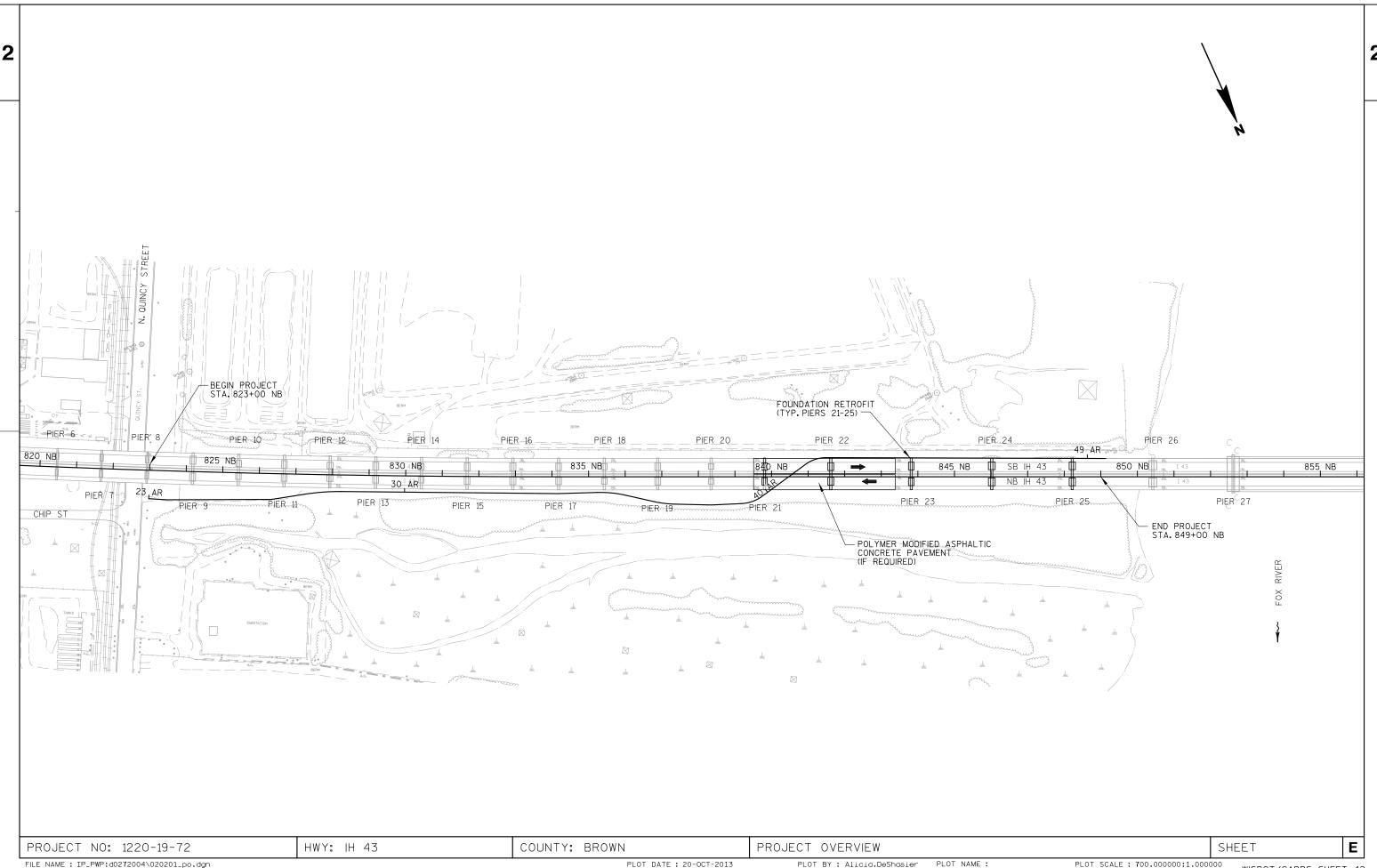
WDNR CONTACT
JIM DOPERALSKI
WISCONSIN DEPARTMENT OF NATURAL RESOURCES
NORTHEAST REGION HEADQUARTERS
2984 SHAWANO AVENUE
GREEN BAY, WI 54313
(920) 662-5119
James,Doperalski@wisconsin.gov

PROJECT NO: 1220-19-72 HWY: IH 43 COUNTY: BROWN

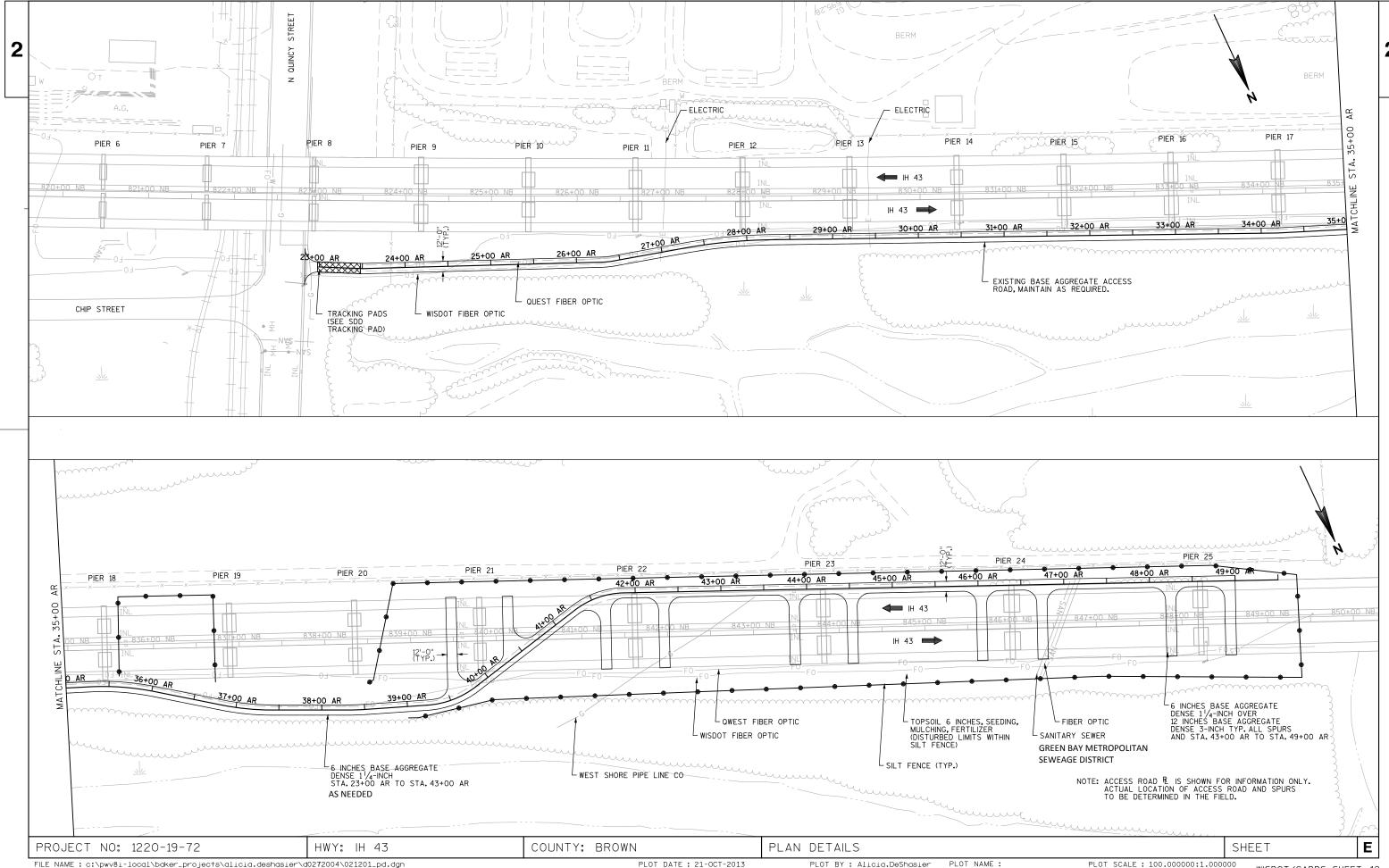
GENERAL NOTES

SHEET

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PLOT SCALE: 700.000000:1.000000 FILE NAME : IP_PWP:d0272004\020201_po.dgn PLOT BY: Alicia.DeShasier PLOT NAME: WISDOT/CADDS SHEET 42

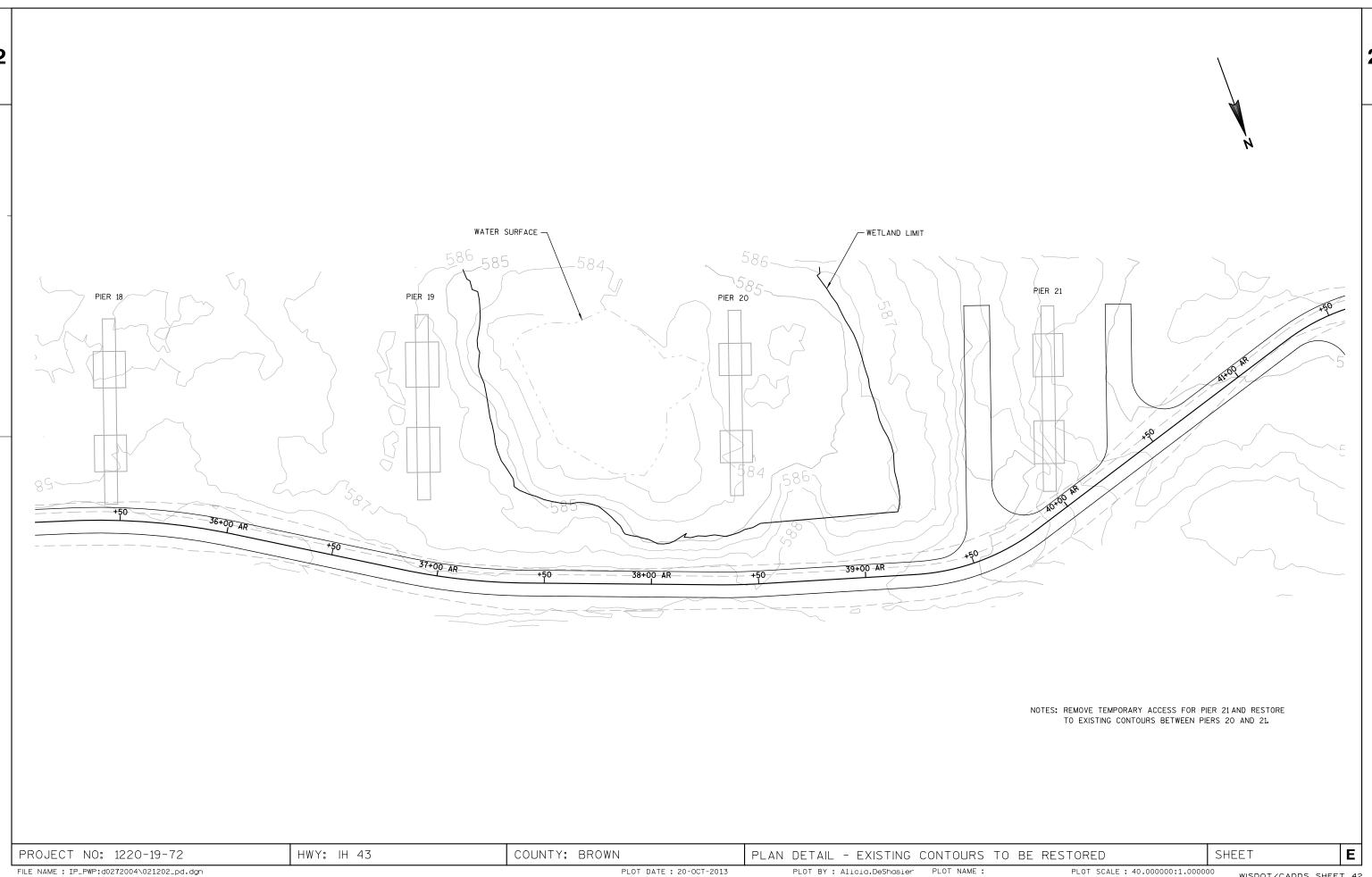


PLOT DATE: 21-OCT-2013

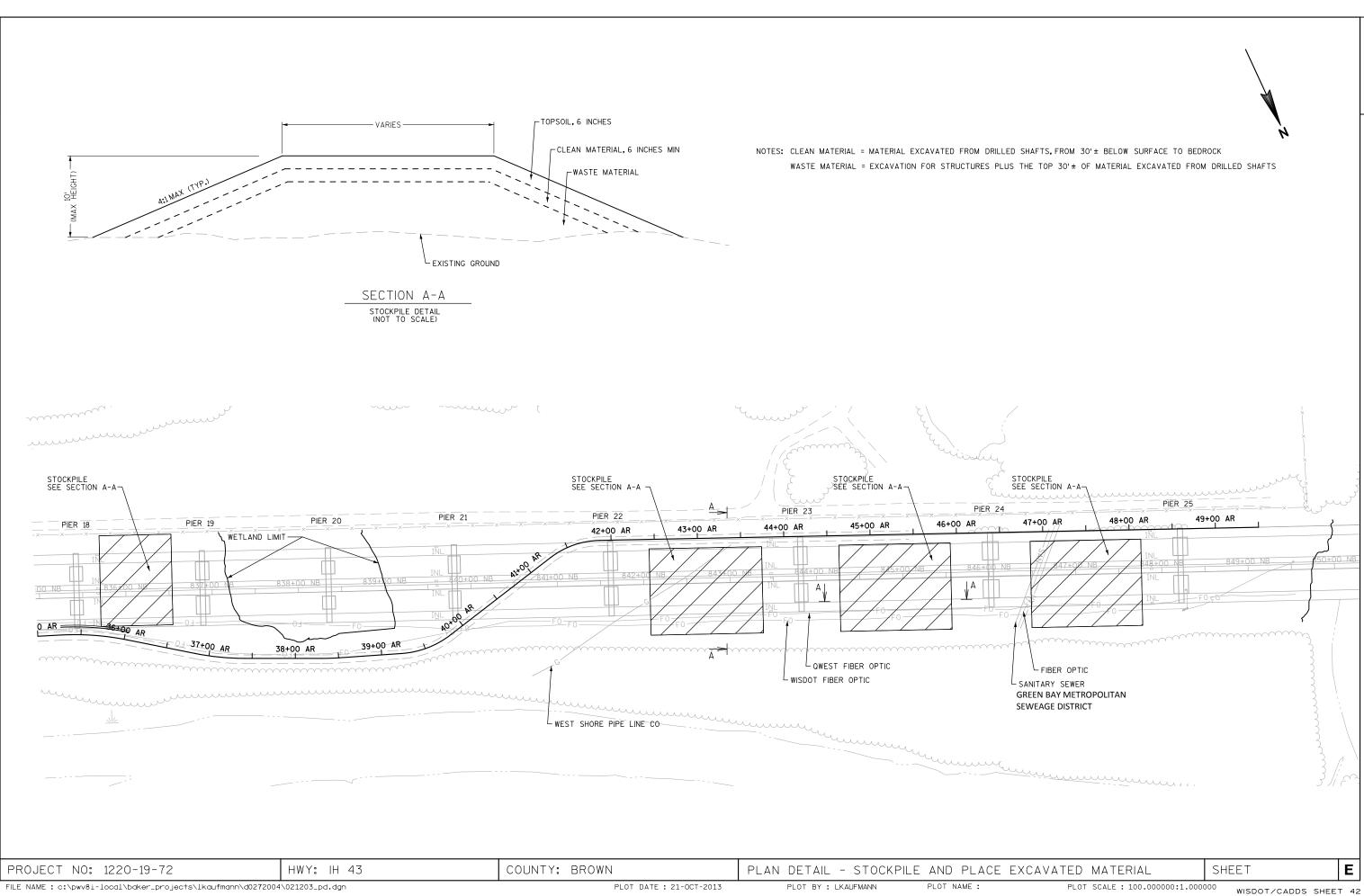
PLOT BY: Alicia.DeShasier

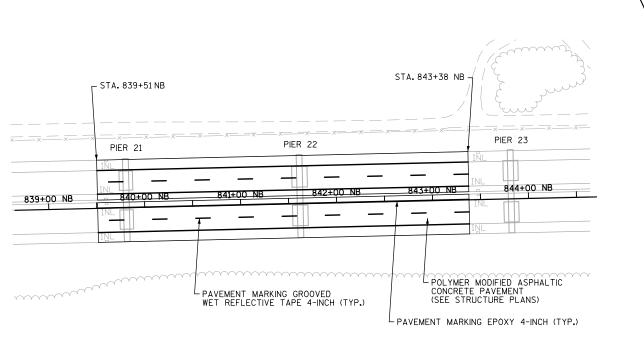
PLOT SCALE: 100.000000:1.000000

WISDOT/CADDS SHEET 42



WISDOT/CADDS SHEET 42





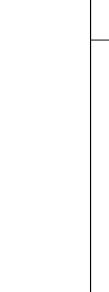
PAVEMENT MARKING DETAIL

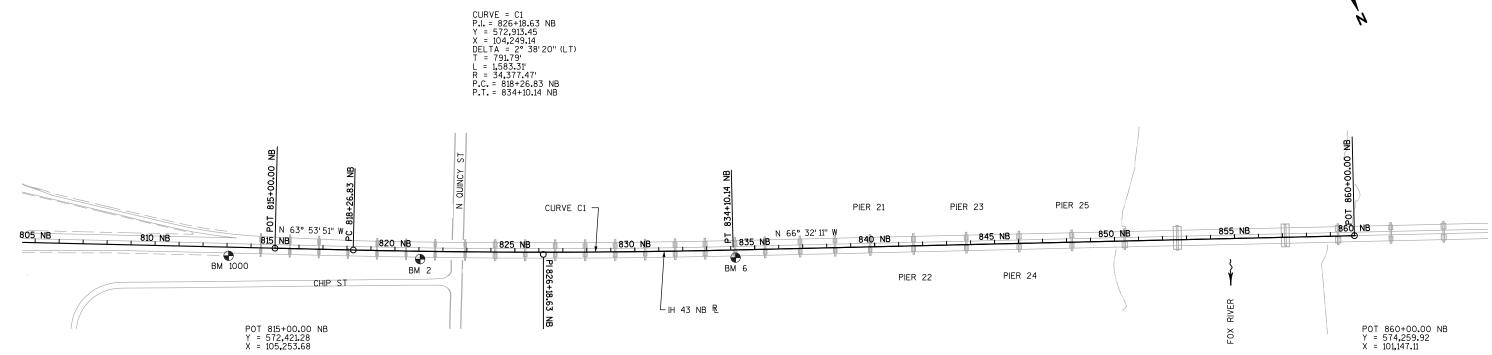
PROJECT NO: 1220-19-72 HWY: IH 43 COUNTY: BROWN PLAN DETAILS SHEET **E**

FILE NAME: IP_PWP:d0272004\021204_pd.dgn

PLOT BY: Alicia.DeShasier PLOT NAME: PLOT SCALE: 100.000000:1.000000 WISDOT/CADDS SHEET 42







BENCH MARKS

NO.	NO. STATION		DESCRIPTION	ELEV.
BM 1000	813+07.39 NB	36.06'RT	BM DISK B-5-158	617.21
BM 2	821+05.61 NB	32.75'RT		627.65
BM 6	834+19.38 NB	32.04' RT		665.53

PROJECT NO: 1220-19-72 HWY: IH 43 COUNTY: BROWN ALIGNMENT DETAIL SHEET **E**

DATE 21 LINE	10CT13	E S	TIMATE	E O F Q U A N	NTITIES 1220-19-72
NUMBER 0010		ITEM DESCRIPTION SINCENTIVE/DISINCENTIVE FOR INTERIM	UNI T CD	TOTAL 15. 000	QUANTI TY 15. 000
0020	203. 0200	COMPLETION OF WORK REMOVING OLD STRUCTURE (STATION) 01.	LS	1. 000	1. 000
0030	205. 0100	843+82 EXCAVATION COMMON	CY	930. 000	930. 000
0040	206. 1000	EXCAVATION FOR STRUCTURES BRIDGES (STRUCTURE) 01. B-5-158	LS	1. 000	1. 000
0050	210. 0100	BACKFILL STRUCTURE	CY	4, 750. 000	4, 750. 000
0060 0070	305. 0120 305. 0130	BASE AGGREGATE DENSE 1 1/4-INCH BASE AGGREGATE DENSE 3-INCH	TON TON	1, 915. 000 1, 850. 000	1, 915. 000 1, 850. 000
0080	502. 5005	MASONRY ANCHORS TYPE L NO. 5 BARS	EACH	120. 000	120. 000
0090	502. 5015	MASONRY ANCHORS TYPE L NO. 7 BARS		2, 860. 000	2, 860. 000
			EACH		
0100	505. 0605	BAR STEEL REINFORCEMENT HS COATED BRIDGES	LB 	682, 670. 000	682, 670. 000
0110	509. 9010. S	S REMOVING ASPHALTIC CONCRETE DECK OVERLAY (STRUCTURE) 01. B-5-158	SY	3, 440. 000	3, 440. 000
0120	618. 0100	MAINTENANCE AND REPAIR OF HAUL ROADS (PROJECT) 01. 1220-19-72	EACH	1. 000	1. 000
0130	619. 1000	MOBILIZATION	EACH	1. 000	1.000
0140	625. 0100	TOPSOI L	SY	22, 000. 000	22, 000. 000
0150	627. 0200	MULCHI NG	SY	22, 000. 000	22, 000. 000
0.00	527.0200	5_5/11 110	0 1	22,000.000	22,000.000
0160	628. 1104	EROSI ON BALES	EACH	10. 000	10. 000
0170	628. 1504	SILT FENCE	LF	3, 320. 000	3, 320. 000
0170	628. 1520	SILT FENCE MAINTENANCE	LF	16, 568. 000	16, 568. 000
	628. 1905	MOBILIZATIONS EROSION CONTROL	EACH	8. 000	8. 000
0190	628. 1905 628. 1910	MOBILIZATIONS ERUSION CONTROL MOBILIZATIONS EMERGENCY EROSION CONTROL			
0200	o∠ŏ. 1910	WUDILIZATIUNS EWERGENCY ERUSIUN CUNTRUL	EACH	5. 000	5. 000
0210	628. 2004	EROSION MAT CLASS I TYPE B	SY	4, 320. 000	4, 320. 000
	628. 7560	TRACKING PADS	EACH	4, 320. 000 1. 000	4, 320. 000 1. 000
0220					
0230	629. 0210	FERTILIZER TYPE B	CWT	82.000	82.000
0240	630. 0130	SEEDING MIXTURE NO. 30	LB	24. 000	24. 000
0250	630. 0200	SEEDING TEMPORARY	LB	36. 000	36. 000
0260	642 E401	FIELD OFFICE TYPE D	EVCH	1 000	1 000
0260	642. 5401		EACH	1.000	1.000
0270	643. 0100	TRAFFIC CONTROL (PROJECT) 01. 1220-19-7		1.000	1.000
0280	643. 0300	TRAFFIC CONTROL DRUMS	DAY	176. 000	176. 000
0290	643. 0420	TRAFFIC CONTROL BARRICADES TYPE III	DAY	4. 000	4. 000
0300	643. 0705	TRAFFIC CONTROL WARNING LIGHTS TYPE A	DAY	8. 000	8. 000
0310	643. 0715	TRAFFIC CONTROL WARNING LIGHTS TYPE C	DAY	64. 000	64. 000
0310	643. 0800	TRAFFIC CONTROL ARROW BOARDS	DAY	8. 000	8. 000
0320	643. 0900	TRAFFIC CONTROL SIGNS	DAY	20. 000	20. 000
0330	646. 0106	PAVEMENT MARKING EPOXY 4-INCH	LF	1, 550. 000	1, 550. 000
0340		PAVEMENT MARKING EPOXY 4-INCH S PAVEMENT MARKING GROOVED WET REFLECTIVE		1, 550, 000	1, 550, 000
	040. 0001. 3	TAPE 4-INCH	LI ⁻	174.000	174.000
0360	649. 0400	TEMPORARY PAVEMENT MARKING REMOVABLE TAPE 4-INCH	LF	3, 120. 000	3, 120. 000
0370	715. 0502	INCENTIVE STRENGTH CONCRETE STRUCTURES	DOL	8, 082. 000	8, 082. 000
0380	ASP. 1TOA	ON-THE-JOB TRAINING APPRENTICE AT \$5.	HRS	2, 400. 000	2, 400. 000
5550		00/HR	5	_, .50. 000	_, .55. 555
0390	ASP. 1TOG	ON-THE-JOB TRAINING GRADUATE AT \$5.00/H	R HRS	4, 400. 000	4, 400. 000
0400	SPV. 0035	SPECIAL 01. HIGH PERFORMANCE CONCRETE	CY	40. 000	40. 000
		(HPC) MASONRY STRUCTURES	= -		
		,			
0410	SPV. 0035	SPECIAL 02. FOUNDATION CONCRETE MASONRY	CY	1, 307. 000	1, 307. 000
0420	SPV. 0045	SPECIAL O1. VIBRATION MONITORING	DAY	25. 000	25. 000
0430	SPV. 0060	SPECIAL 01. BAR COUPLERS NO. 11 BAR	EACH	1, 200. 000	1, 200. 000
0.50	2 0000	SPECIAL	_,.511	., 250. 000	., 250. 000
0440	SPV. 0060	SPECIAL O2. SEDIMENTATION BASIN	EACH	1.000	1. 000
0440	SPV. 0000 SPV. 0075	SPECIAL 02. SEDIMENTATION BASIN SPECIAL 01. DRILLED SHAFT OBSTRUCTIONS	HRS	30. 000	30. 000
0450	3F V. 00/3	SELCIAL UI. DRILLLU SHAFI UDSIKUCIIUNS	пкэ	30.000	30.000

DATE 210CT13		E S	ГІМАТЕ	E OF QUAN		
LI NE NUMBER	ITEM	ITEM DESCRIPTION	UNI T	TOTAL	1220-19-72 QUANTI TY	
0460	SPV. 0075	SPECIAL 02. STREET SWEEPING	HRS	33.000	33.000	
0470	SPV. 0085	SPECIAL 01. POST-TENSIONING PIER FOOTING	LB	65, 957. 000	65, 957. 000	
0480	SPV. 0085	SPECIAL 02. STRUCTURAL STEEL CARBON	LB	27, 960. 000	27, 960. 000	
		SPECI AL				
0490	SPV. 0090	SPECIAL 01. DRILLED SHAFT FOUNDATION	LF	2, 472. 000	2, 472. 000	
		60-I NCH				
0500	SPV. 0105	SPECIAL 01. BRIDGE JACKING SPECIAL	LS	1. 000	1. 000	
		STRUCTURE B-5-158				
0510	SPV. 0105	SPECIAL 02. TRIAL DRILLED SHAFT	LS	1. 000	1. 000	
		FOUNDATION 60-INCH				
0520	SPV. 0105	SPECIAL 03. STOCKPILE AND PLACE	LS	1. 000	1. 000	
		EXCAVATED MATERIAL				
0530	SPV. 0195	SPECIAL 01. POLYMER MODIFIED ASPHALTIC	TON	385. 000	385. 000	
		CONCRETE PAVEMENT				

205.0100 EXCAVATION COMMON

						COMMON
STATION		то	STATION		LOCATION	CY
39+50	AR		40+25	AR	PIER 21	144
41+50	AR		42+50	AR	PIER 22	130
43+75	AR		44+75	AR	PIER 23	120
46+00	AR		46+75	AR	PIER 24	120
48+10	AR		48+85	AR	PIER 25	120
					UNDISTRIBUTED	296
PROJECT	TOTA	L				930

305.0120	305.0130
BASE AGGREGATE	BASE AGGREGATE
DENSE 1 1/4-INCH	DENSE 3-INCH

STATIO	TATION TO STATION LOCA		LOCATION	TON	TON		
23+00	AR		49+50	AR	LENGTH OF ACCESS ROAD	1228	
43+00	AR		49+50	AR	ACCESS ROAD PIER 23-25		607
39+50	AR		40+25	AR	PIER 21	101	201
41+50	AR		42+50	AR	PIER 22	87	173
43+75	AR		44+75	AR	PIER 23	84	168
46+00	AR		46+75	AR	PIER 24	83	166
48+10	AR	• •	48+85	AR	PIER 25	82	165
					UNDISTRIBUTED	250	370
ROJECT T	OTAL					1,915	1,850

EROSION CONTROL

						625.0100 TOPSOIL	627.0200 MULCHING	628.1104 EROSION BALES	628.1504 SILT FENCE	628.1520 SILT FENCE MAINTENANCE	628.1905 MOBILIZATIONS EROSION CONTROL	628.1910 MOBILIZATIONS EMERGENCY EROSION CONTROL	628.2004 EROSION MAT CLASS I TYPE B	628.7560 Tracking Pads	629.0210 FERTILIZER TYPE B	630.0130 SEEDING MIXTURE NO. 30	630.0200 SEEDING TEMPORARY
STATION		то	STATION		LOCATION	SY	SY	EACH	LF	LF	EACH	EACH	SY	EACH	CWT	LB	LB
23+00	AR		49+50	AR	LENGTH OF ACCESS ROAD				2650					1			
39+49	AR		40+24	AR	STOCKPILES	350	350			2208					10.9	3.1	4.7
39+50	AR		40+25	AR	PIER 21	3450	3450			2208					10.9	3.1	4.7
41+50	AR		42+50	AR	PIER 22	3450	3450			2208					10.9	3.1	4.7
43+75	AR		44+75	AR	PIER 23	3450	3450			2208					10.9	3.1	4.7
46+00	AR		46+75	AR	PIER 24	3450	3450			2208					10.9	3.1	4.7
48+10	AR		48+85	AR	PIER 25	3450	3450			2208					10.9	3.1	4.7
					UNDISTRIBUTED	4400	4400	10	670	3320	8	5	4320		16.3	4.7	7.1
PROJECT TO	TAL					22,000	22,000	10	3,320	16,570	8	5	4,320	1	82	24	36

SUMMARY OF EACH & LUMP SUM BID ITEMS

CATEGORY	ITEM NUMBER	DESCRIPTION	UNITS	QUANTITY	
***************************************					-
	618.0100	MAINTENANCE AND REPAIR OF HAUL ROADS 1220-19-72	EACH	1	
0020	619.1000	MOBILIZATION	EACH	1	
	642.5401	FIELD OFFICE TYPE D	EACH	1	
	SPV.0060.02	SEDIMENTATION BASIN	EACH	1	
	SPV.0105.03	STOCKPILE AND PLACE EXCAVATED MATERIAL	LS	1	

HWY: IH 43

SPV.0075.02 STREET SWEEPING

LOCATION	FREQUENCY	HRS		
QUINCY STREET	3 TIMES PER WEEK	33		
	PROJECT TOTAL	33		

PROJECT NO: 1220-19-72

COUNTY: BROWN

MISCELLANEOUS QUANTITIES

PLOT BY : LKAUFMANN

SHEET

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TRAFFIC CONTROL

	643.0100 TRAFFIC CONTROL PROJECT	643.0300 TRAFFIC CONTROL DRUMS	643.0420 TRAFFIC CONTROL BARRICADES TYPE III	643.0705 TRAFFIC CONTROL WARNING LIGHTS TYPE A	643.07105 TRAFFIC CONTROL Warning Lights Type C	643.0800 TRAFFIC CONTROL ARROW BOARDS	643.0900 TRAFFIC CONTROL SIGNS	TEMPORAR MARKING REI	.0400 Y PAVEMENT MOVABLE TAPE INCH
LOCATION	EACH	DAYS	DAYS	DAYS	DAYS	DAYS	DAYS	(WHITE) LF	(YELLOW) LF
STA 839+51 NB - 843+38 NB									
NB IH 43 LANE CLOSURE		88	2	4	32	4	10	780	780
SB IH 43 LANE CLOSURE		88	2	4	32	4	10	780	780
PROJECT TOTAL	1	176	4	8	64	8	20	1560	1560

PAVEMENT MARKING

646.0106 646.0881.S PAVEMENT MARKING EPOXY PAVEMENT 4-INCH MARKING **GROOVED WET** REFLECTIVE TAPE 4-INCH (YELLOW) (WHITE)

STATIO	N	то	STATION	LOCATION	LF	LF	LF
839+51	NB	-	843+38 NB	IH 43 NB & SB	775	775	194
				PROJECT TOTAL	775	775	194

SPV.0045.01 VIBRATION MONITORING

STATION	TO	STATION	DAY
841+00 NB	-	843+00 NB	25
	PRO	JECT TOTAL	25

PROJECT NO: 1220-19-72 HWY: IH 43 COUNTY: BROWN

MISCELLANEOUS QUANTITIES

PLOT NAME:

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SHEET

PLOT DATE: 21-OCT-2013

PLOT BY : LKAUFMANN

Standard Detail Drawing List

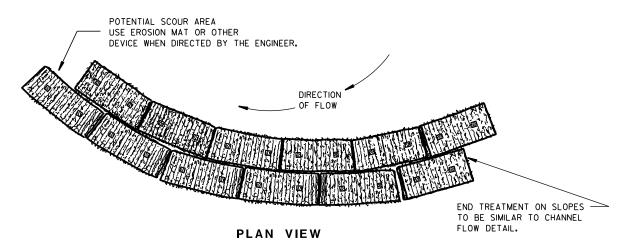
TYPICAL INSTALLATIONS OF EROSION BALES / TEMPORARY DITCH CHECKS SILT FENCE
TRACKING PAD
PAVEMENT MARKING (MAINLINE)
TRAFFIC CONTROL, LANE CLOSURE, SPEEDS GREATER THAN 40 M.P.H.

08E08-03 08E09-06 08E14-01 15C08-16A 15D12-03

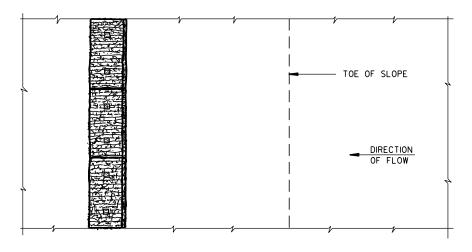
GENERAL NOTES

DETAILS OF CONSTRUCTION, MATERIALS AND WORKMANSHIP NOT SHOWN ON THIS DRAWING SHALL CONFORM TO THE PERTINENT REQUIREMENTS OF THE STANDARD SPECIFICATIONS AND THE APPLICABLE SPECIAL PROVISIONS.

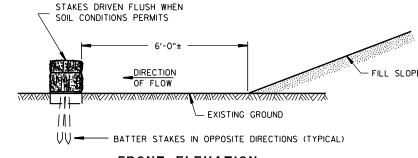
TEMPORARY DITCH CHECKS EITHER EROSION BALES OR MANUFACTURED SHALL BE PAID FOR UNDER THE BID ITEM OF TEMPORARY DITCH CHECK. THE DEPARTMENT WILL NOT PAY FOR TEMPORARY DITCH CHECKS CONSTRUCTED OF A SINGLE ROW OF EROSION BALES.



WHEN ALTERING THE DIRECTION OF FLOW



PLAN VIEW



FRONT ELEVATION

WHEN EXISTING GROUND SLOPES AWAY FROM FILL SLOPE

EROSION BALES FOR SHEET FLOW

TYPICAL INSTALLATIONS OF **EROSION BALES / TEMPORARY** DITCH CHECKS

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STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

APPROVED

6/04/02 /S/ Beth Connestro
CHIEF ROADWAY DEVELOPMENT ENGINEER

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TYPICAL APPLICATION OF SILT FENCE

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PLAN VIEW SILT FENCE AT MEDIAN SURFACE DRAINS



GENERAL NOTES

DETAILS OF CONSTRUCTION NOT SHOWN ON THIS DRAWING SHALL CONFORM TO THE PERTINENT REQUIREMENTS OF THE STANDARD SPECIFICATIONS AND APPLICABLE SPECIAL PROVISIONS.

- ① HORIZONTAL BRACE REQUIRED WITH 2" X 4" WOODEN FRAME OR EQUIVALENT AT TOP OF POSTS.
- 2 FOR MANUAL INSTALLATIONS THE TRENCH SHALL BE A MINIMUM OF 4" WIDE & 6" DEEP TO BURY AND ANCHOR THE GEOTEXTILE FABRIC. FOLD MATERIAL TO FIT TRENCH AND BACKFILL & COMPACT TRENCH WITH EXCAVATED SOIL.
- 3 WOOD POSTS SHALL BE A MINIMUM SIZE OF 11/8" X 11/8" OF OAK OR HICKORY.
- 4) SILT FENCE TO EXTEND ACROSS THE TOP OF THE PIPE.
- (5) CONSTRUCT SILT FENCE FROM A CONTINUOUS ROLL IF POSSIBLE BY CUTTING LENGTHS TO AVOID JOINTS. IF A JOINT IS NECESSARY USE ONE OF THE FOLLOWING TWO METHODS; A) OVERLAP THE END POSTS AND TWIST, OR ROTATE, AT LEAST 180 DEGREES, B) HOOK THE END OF EACH SILT FENCE LENGTH.



TRENCH DETAIL



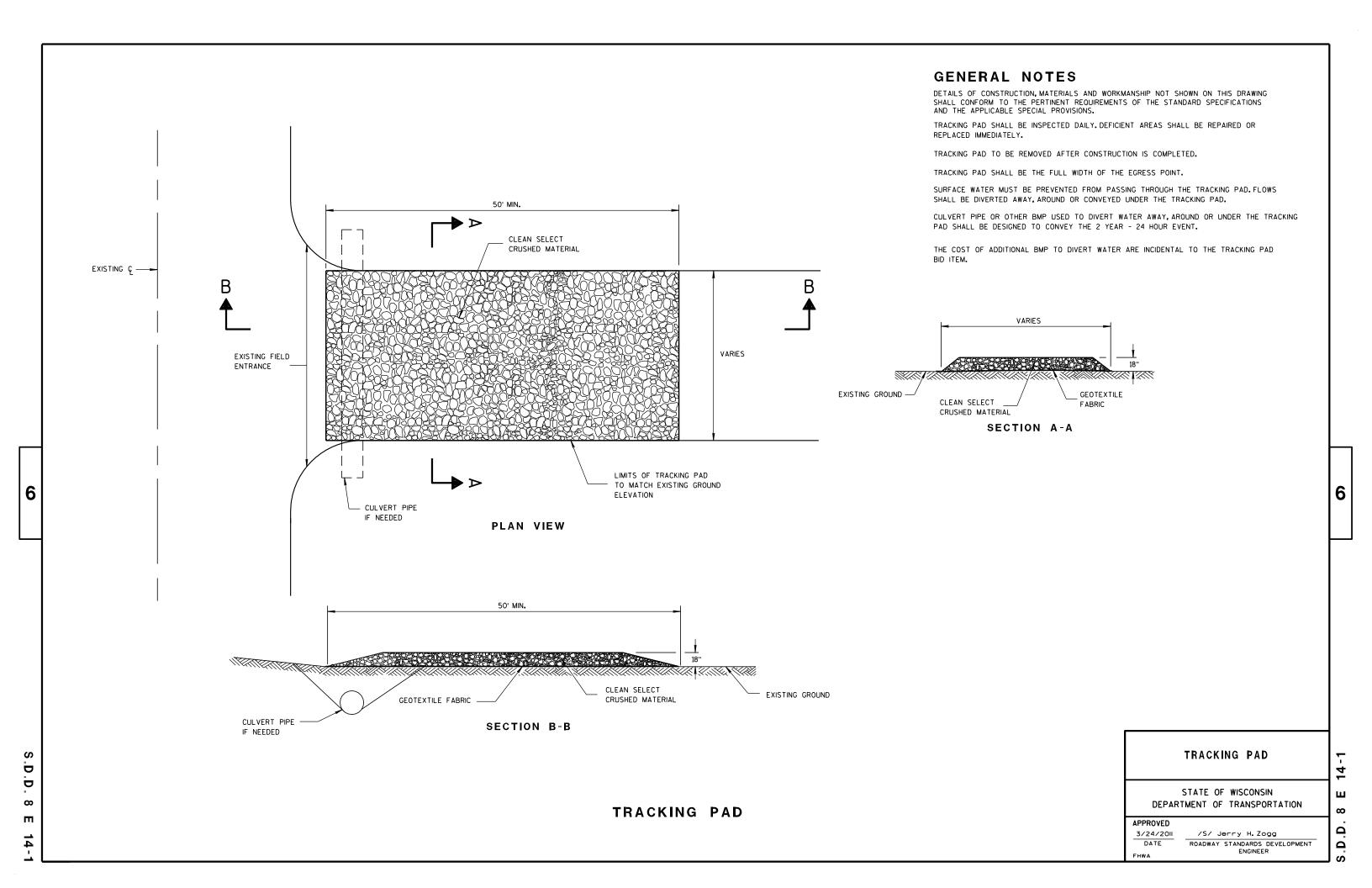
SILT FENCE TIE BACK

(WHEN REQUIRED BY THE ENGINEER)

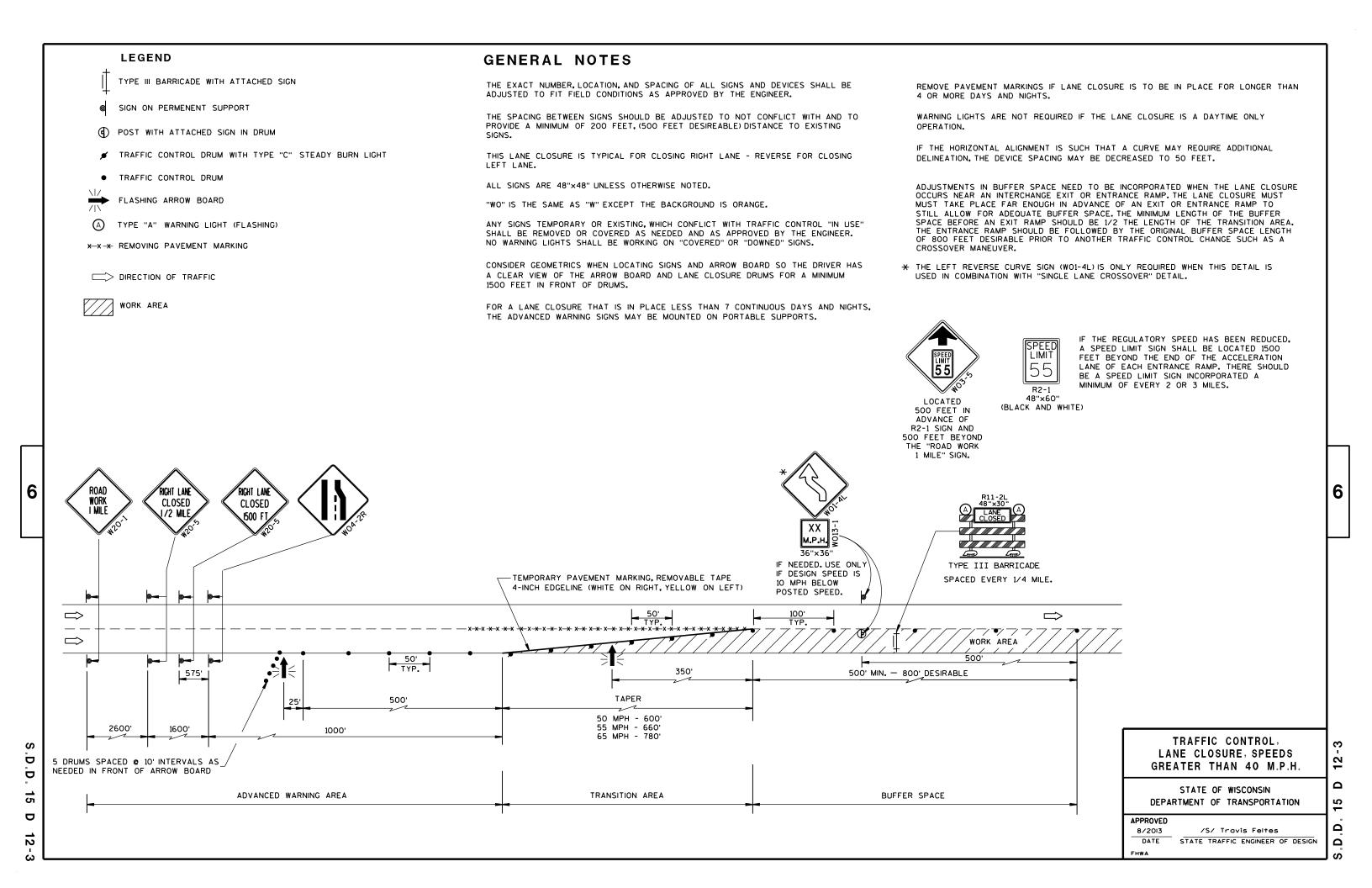


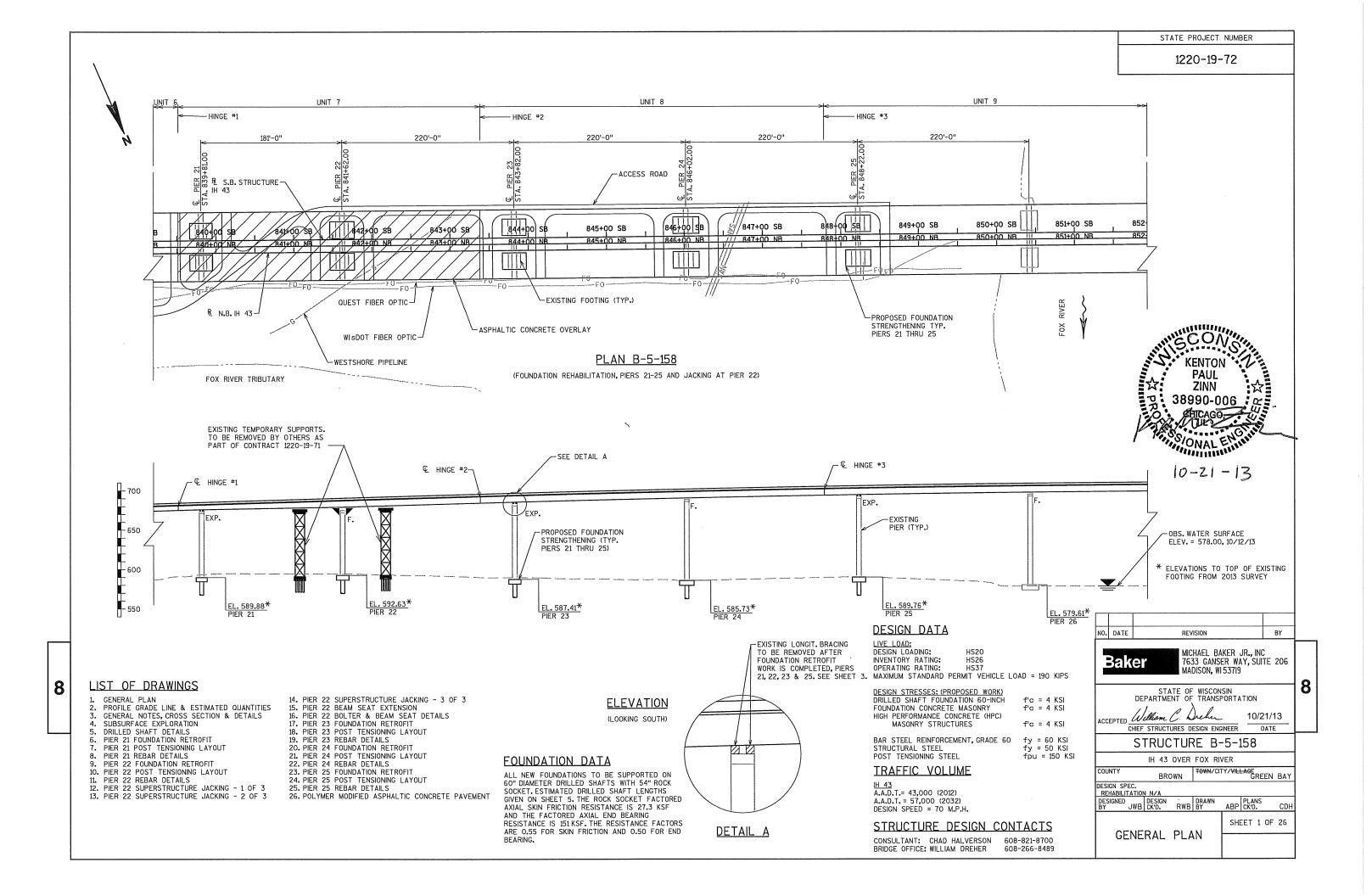
SILT FENCE

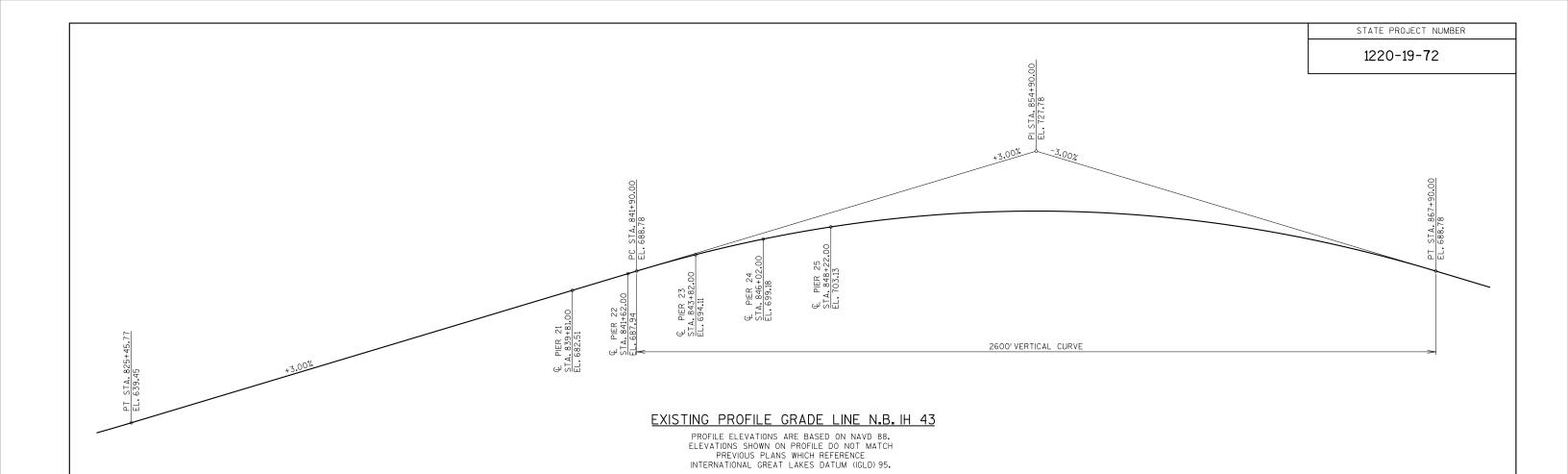
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TOTAL ESTIMATED QUANTITIES

ITEM NUMBER	ITEM DESCRIPTION	UNIT	PIER 21	PIER 22	PIER 23	PIER 24	PIER 25	SUPER	TOTALS
203.0200	REMOVING OLD STRUCTURE STATION 843+82	LS	-	-	-	-	-	-	1
206.1000	EXCAVATION FOR STRUCTURES BRIDGES B-5-158	LS	-	-	-	-	-	-	1
210.0100	BACKFILL STRUCTURE	CY	820	1,270	950	490	1,220	-	4,750
502,5005	MASONRY ANCHORS TYPE L NO.5 BARS	EACH	24	24	24	24	24	-	120
502,5015	MASONRY ANCHORS TYPE L NO.7 BARS	EACH	536	596	576	576	576	-	2,860
505.0605	BAR STEEL REINFORCEMENT HS COATED BRIDGES	LB	134,020	139,250	136,140	136,640	136,620	-	682,670
509.9010.S	REMOVING ASPHALTIC CONCRETE DECK OVERLAY	SY	-	-	-	-	-	3,440	3,440
SPV.0035.01	HIGH PERFORMANCE CONCRETE (HPC) MASONRY STRUCTURES	CY	-	40	-	-	-	-	40
SPV.0035.02	FOUNDATION CONCRETE MASONRY	CY	230.4	269.6	258.0	279.0	270.0	-	1,307
SPV.0060.01	BAR COUPLERS NO. 11 SPECIAL	EACH	240	240	240	240	240	-	1,200
SPV.0075.01	DRILLED SHAFT OBSTRUCTIONS	HRS	6	6	6	6	6	-	30
SPV.0085.01	POST-TENSIONING PIER FOOTING	LB	12,230	13,104	12,886	14,196	13,541	=	65,957
SPV.0085.02	STRUCTURAL STEEL CARBON SPECIAL	LB	-	19,810	-	-	-	8,150	27,960
SPV.0090.01	DRILLED SHAFT FOUNDATION 60-INCH	LF	512	508	476	476	500	-	2,472
SPV.0105.01	BRIDGE JACKING SPECIAL STRUCTURE B-5-158	LS	-	1	-	-	-	-	1
SPV.0105.02	TRIAL DRILLED SHAFT FOUNDATION 60-INCH	LS	-	1	-	-	-	-	1
SPV.0195.01	POLYMER MODIFIED ASPHALTIC CONCRETE PAVEMENT	TON	-	-	-	-	-	385	385

BENCH MARKS

NO.	STATION	OFFSET	DESCRIPTION	ELEV.
BM 1000	813+07.39 NB	36.06'RT	BM DISK B-5-158	617.21
BM 2	821+05.61 NB	32.75'RT		627.65
ВМ 6	834+19.38 NB	32.04' RT		665.53

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STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158

PROFILE GRADE SHEET 2 OF 26

LINE & ESTIMATED QUANTITIES

8

STATE PROJECT NUMBER

THE PROPOSED WORK TO INCLUDE FOUNDATION STRENGTHENING AND SUPERSTRUCTURE JACKING.

DRAWINGS SHALL NOT BE SCALED.

BAR STEEL REINFORCEMENT SHALL BE EMBEDDED 2" CLEAR UNLESS SHOWN OR NOTED OTHERWISE.

ALL FIELD CONNECTIONS SHALL BE MADE WITH $\frac{3}{4}$ " DIAMETER FRICTION TYPE HIGH-TENSILE STRENGTH BOLTS UNLESS SHOWN OR NOTED OTHERWISE.

THE FIRST OR FIRST TWO DIGITS OF THE BAR MARK SIGNIFIES THE BAR SIZE.

THE EXISTING GROUND LINE SHALL BE USED AS THE UPPER LIMITS OF EXCAVATION AT THE PIERS.

DIMENSIONS SHOWN ARE BASED ON THE ORIGINAL STRUCTURE PLANS.

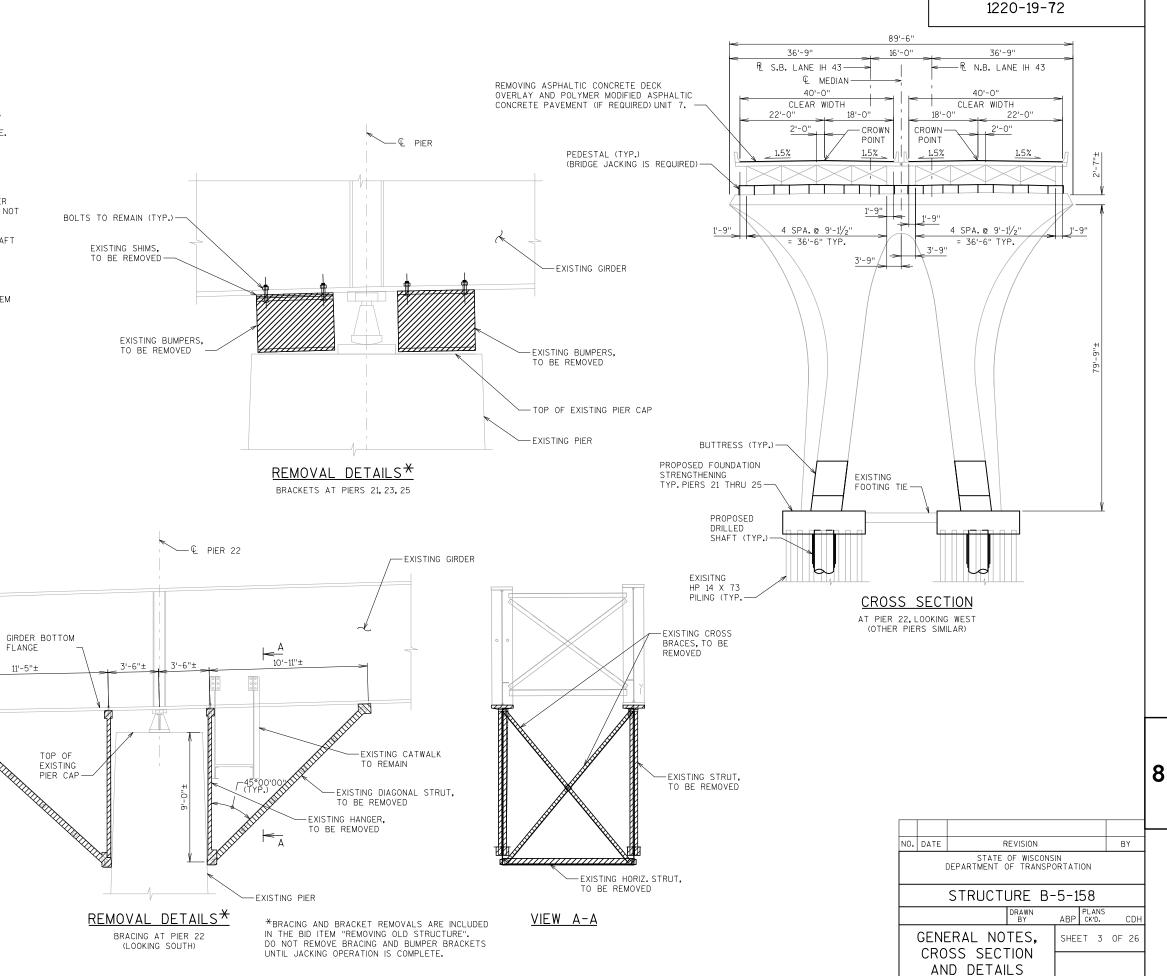
REMOVING ASPHALTIC CONCRETE DECK OVERLAY AND CONSTRUCTING POLYMER MODIFIED ASPHALTIC CONCRETE PAVEMENT (IF REQUIRED) FOR UNIT 7 SHALL NOT BEGIN UNTIL AUTHORIZATION FROM THE DEPARTMENT.

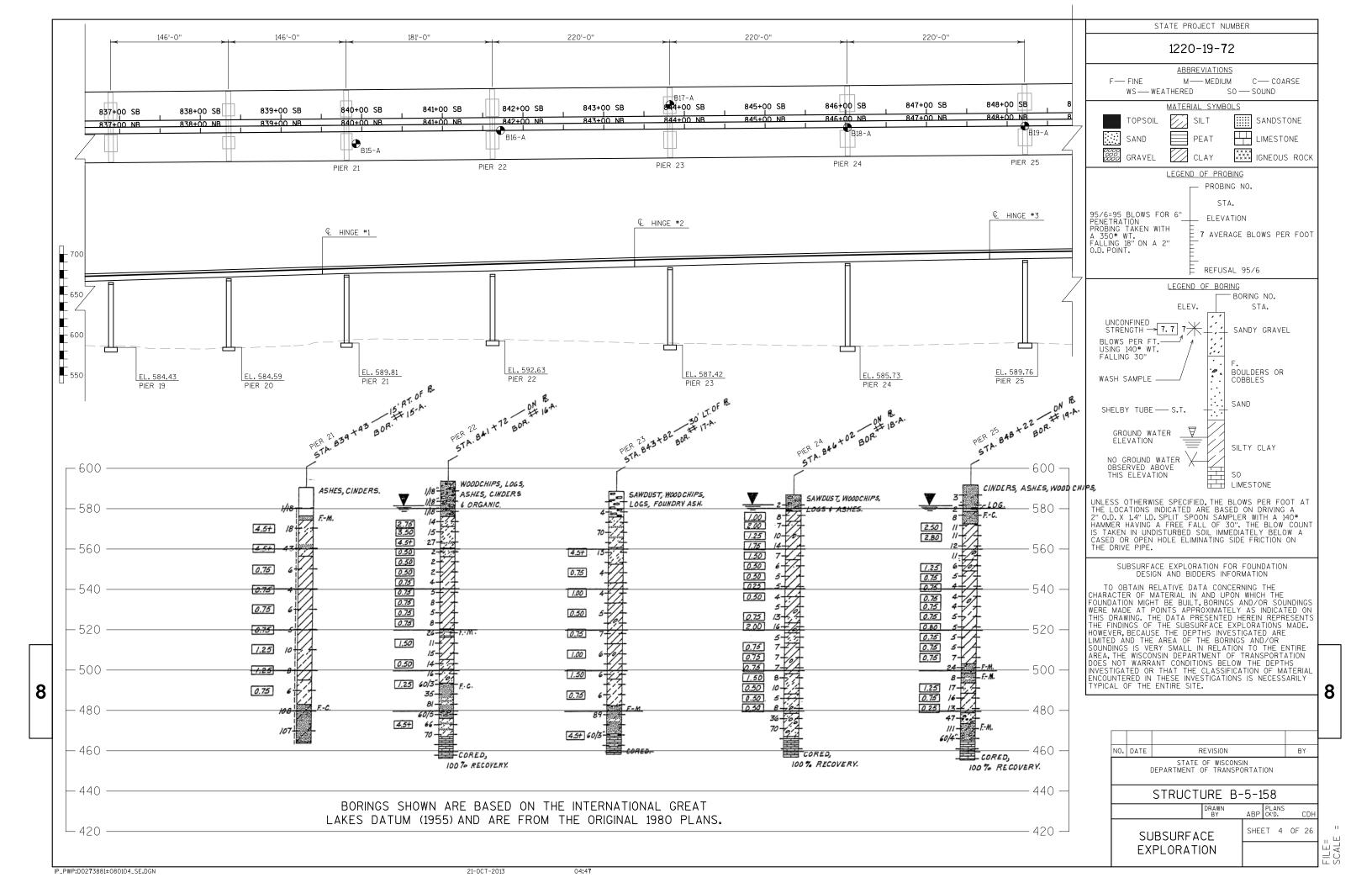
CONCRETE FOR THE DRILLED SHAFTS IS INCLUDED IN BID ITEM "DRILLED SHAFT FOUNDATION 60-INCH".

CONCRETE FOR THE FOOTING EXTENSION AND BUTTRESSES IS INCLUDED IN BID ITEM "FOUNDATION CONCRETE MASONRY".

CONCRETE FOR THE PIER CAP (PIER 22) BEAM SEATS IS INCLUDED IN BID ITEM "HIGH PERFORMANCE CONCRETE (HPC) MASONRY STRUCTURES".

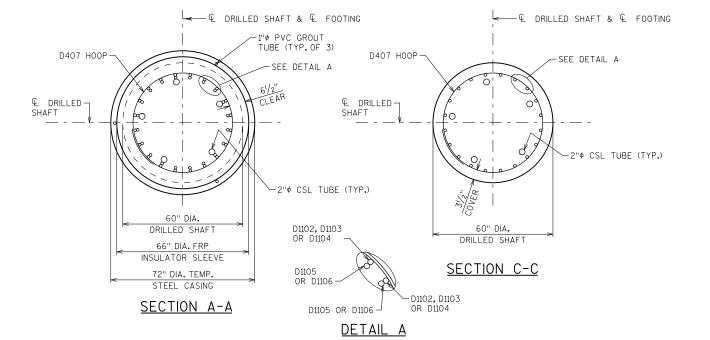
BOLTS TO REMAIN (TYP.)-

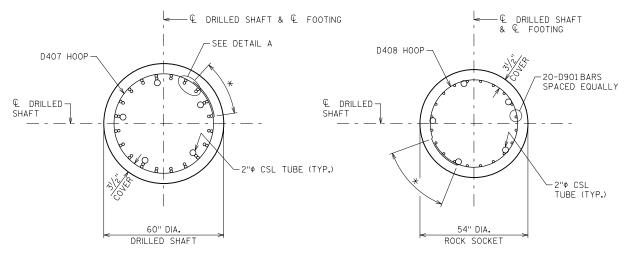




NOTES

- 1. STEEL CASING, STEEL PIPES FOR CROSSHOLE SONIC LOGGING (CSL), FRP SLEEVE, AND CONCRETE SHALL BE INCLUDED IN THE COST FOR "DRILLED SHAFT FOUNDATION 60-INCH".
- 2. EPOXY COATED STEEL PIPES SHALL BE FURNISHED AND INSTALLED FOR THE FULL LENGTH OF THE DRILLED SHAFTS FOR CROSSHOLE SONIC LOGGING (CSL).
- 3. THE CONTRACTOR SHALL NOT YIELD OR DEFORM THE REINFORCEMENT BAR CAGE DURING ITS LIFTING, HANDLING OR PLACEMENT.
- 4. STAGGER LOCATIONS OF BAR COUPLERS NO.11 SPECIAL. CONTRACTOR TO PROVIDE LAYOUT OF SPLICE LOCATIONS WITH SHOP DRAWINGS.
- 5. CUT SHAFT BARS AS SHOWN TO LIMITS ABOVE TOP OF FOOTING. QUANTITY FOR PAYMENT IS BASED ON REINFORCEMENT AS DETAILED.





DRILLED SHAFT DATA TABLE

PIER	BOTTOM OF FOOTING ELEVATION (1)	APPROX. BOTTOM OF DRILLED SHAFT ELEVATION (2)	APPROX. BOTTOM OF ROCK SOCKET ELEVATION	TOTAL DRILLED SHAFT LENGTH (FEET) (3)
21	584.63	465	458	128
22	586.63	468	461	127
23	581.41	470	463	119
24	579.48	468	461	119
25	583.51	467	460	125

- (1) BOTTOM OF FOOTING ELEVATIONS ARE FROM 2013 SURVEY (NAVD 88) (2) BOTTOM OF DRILLED SHAFT ELEVATIONS ARE AT TOP OF COMPETENT ROCK AS DETERMINED FROM 1975 CORE BORINGS, ADJUSTED BY 1'± TO
- CORRELATE WITH NAVD 88.

 (3) TOTAL DRILLED SHAFT LENGTH INCLUDES SHAFT LENGTH PLUS 1 EMBEDMENT INTO BOTTOM OF FOOTING PLUS 7' ROCK SOCKET

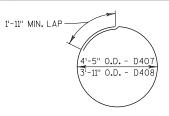
BILL OF BARS - DRILLED SHAFTS

COATED (PER PIER) = 97,730 LBS TOTAL COATED = 488,650 LBS

SECTION D-D

PIER	BAR MARK	NO. REQ'D PER SHAFT	TOTAL NO. REQ'D.	LENGTH	BENT	COAT	LOCATION	COMMENTS
	D901	20	400	11'-7''		Х	ROCK SOCKET	
	D1102	20	400	60'-0"		Χ	DRILLED SHAFT VERTICAL	BAR COUPLER AT ONE END
	D1103	20	400	60'-0"		Х	DRILLED SHAFT VERTICAL	BAR COUPLER AT BOTH ENDS
PIERS 21-25	D1104	20	400	14'-0''		Х	DRILLED SHAFT VERTICAL	BAR COUPLER AT ONE END - CUT TO FIT IN FIELD
21 23	D1105	20	400	19'-0''		Х	DRILLED SHAFT VERTICAL	BAR COUPLER AT ONE END - CUT TO FIT IN FIELD
	D1106	20	400	55'-0"		Х	DRILLED SHAFT VERTICAL	BAR COUPLER AT ONE END
	D407	134	2680	15'-10"	Х	Х	DRILLED SHAFT TIE	
	D408	13	260	14'-3"	X	Х	ROCK SOCKET TIE	

NOTE: THE FIRST OR FIRST TWO DIGITS OF THE BAR MARK SIGNIFIES THE BAR SIZE. ALL BAR BEND DIMENSIONS ARE OUT-TO-OUT OF BAR.



D407 D408 **LEGEND**

* STAGGER SPLICE 180° EVERY OTHER BAR

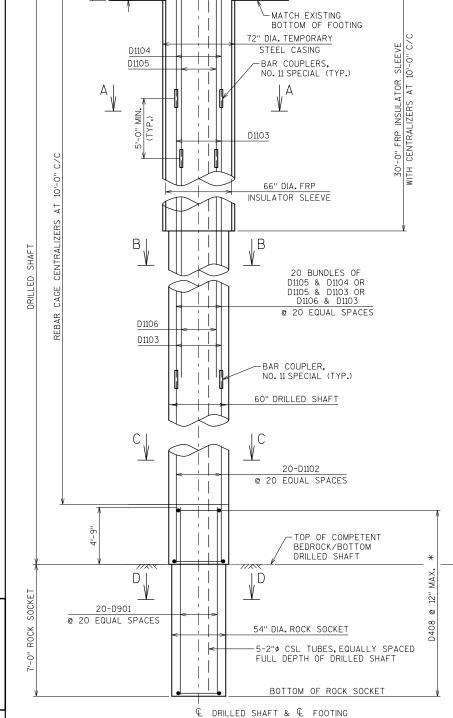
NO. DATE BY STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158 DRAWN CLG PLANS DGM

DRILLED SHAFT DETAILS

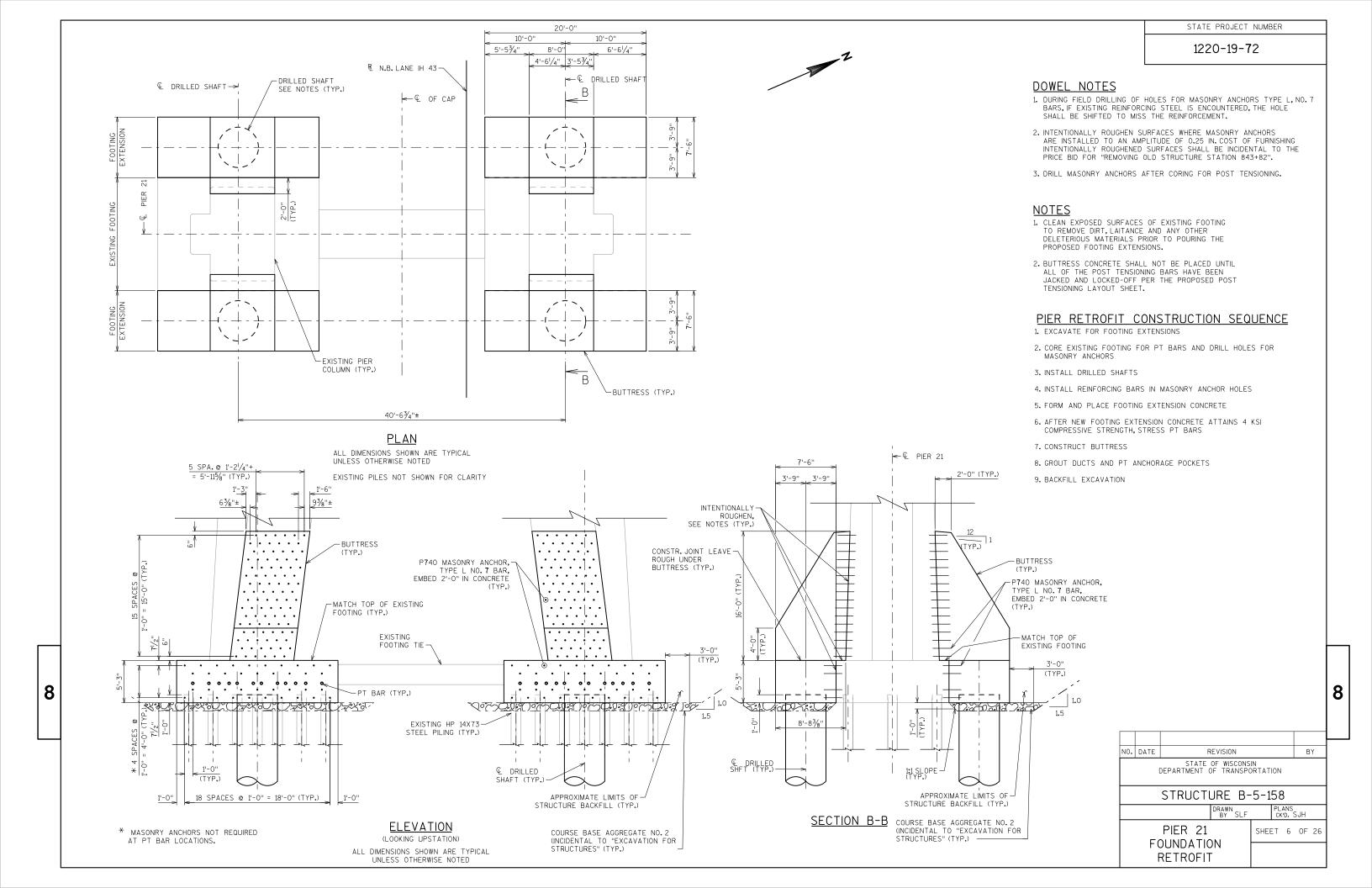
SHEET 5 OF 26

8



DRILLED SHAFT ELEVATION

SECTION B-B



PIER 21 - POST TENSIONING

			ı	1	
FOOTING	TENDON	BAR STRESSING LENGTH	AREA OF BAR	JACKING FORCE	LOCK-OFF FORCE
		L	А	JF	LOF
		(FT)	(IN2)	(KIPS)	(KIPS)
PIER 21	ALL	28.00	5.16	554.0	540

NOTE: THE LOCK-OFF FORCE HAS BEEN CALCULATED AS FOLLOWS: LOF = JF - $((E \times A)/(12 \times L)) \times AS < 0.7 \times fpu \times As$

> WHERE. LOCK-OFF FORCE (KIPS) LOF JACKING FORCE (KIPS) MODULUS OF ELASTICITY (29,700 KSI) AREA OF BAR (IN2)
> BAR STRESSING LENGTH (FT) ANCHOR SET (1/32 IN)

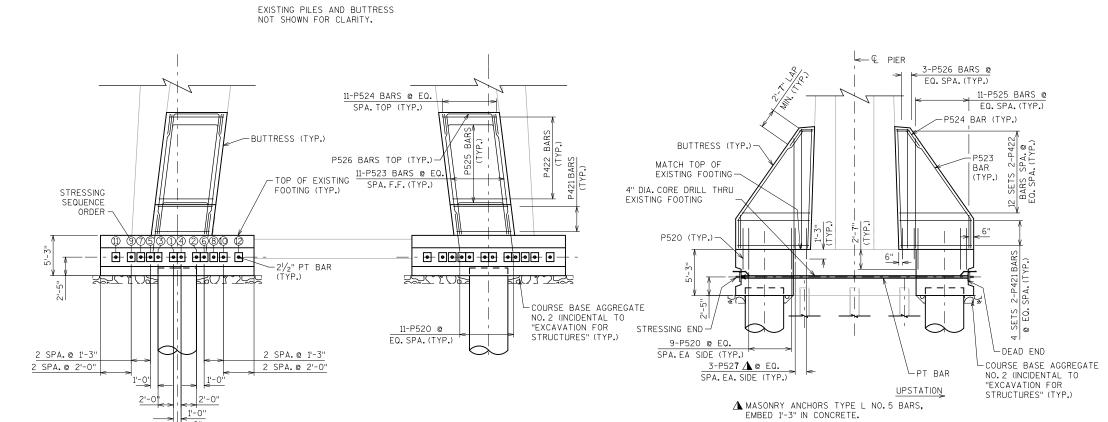
NOTES

- 1. CORE DRILL HOLES FOR PT BARS IN EXISTING FOOTING FOLLOWING EXCAVATION AND PRIOR TO DRILLED SHAFT PLACEMENT AND PRIOR TO POURING FOOTING EXTENSIONS.
- 2. PROVIDE DUCTS IN FOOTING EXTENSIONS FOR PT BARS.
- 3. ALL PT BARS ARE $2^{1}/2^{11}$ DIAMETER, ASTM A 722, GR 150, EPOXY COATED.
- 4. SPIRAL AND OTHER LOCAL ANCHORAGE ZONE REINFORCEMENT IS TO BE DESIGNED BY THE POST-TENSIONING SUPPLIER AND TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 5. IF FIELD CONDITIONS DIFFER FROM ASSUMPTIONS FOR PT. ADJUSTMENT IN JACK FORCE AND ELONGATION SHALL BE MADE ACCORDINGLY AND BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 6. CAST ANCHOR FACES PERPENDICULAR TO PT DUCTS WITH NO OUT-OF-PLANENESS. A THIN LAYER OF MORTAR MAY BE APPLIED WHEN SETTING THE ANCHOR PLATE, SUBJECT TO APPROVAL OF
- 7. STRENGTH OF CONCRETE AT TIME OF POST TENSIONING, f'c = 4 KSI.
- 8. ALL POST TENSIONING MUST BE COMPLETED AND LOCKED OFF PRIOR TO PLACING BUTTRESS CONCRETE.

4" DIA. (ID) PLASTIC DUCT 21/2" PT BAR-4" DIA. CORE DRILL THRU EXISTING FOOTING FOOTING EXTENSION: - FXISTING FOOTING SEAL ALL AROUND

TYPICAL DETAIL OF CONNECTION BETWEEN DUCT AND EXISTING CONCRETE

BEFORE CONCRETING



PROVIDE 4" DIA. (ID) DUCTS IN

FOOTING EXTENSIONS (TYP.)

-R N.B. LANE IH 43

-DRILLED SHAFT (TYP.)

20'-0"

12 PT BARS

© DRILLED
SHAFT (TYP.)

10'-0"

- € 2½" PT BAR (TYP.)

€ OF CAP→

CONTINUOUS POCKET WITH PT

BAR AND 23/4" X 12" X 1'-2"

(CONTRACTOR TO DETERMINE

DIMENSIONS PER MANUFACTURER'S

<u>PLAN</u>

TYPICAL UNLESS NOTED OTHERWISE.

ELEVATION

ALL DIMENSIONS SHOWN ARE

EXISTING PILES NOT SHOWN FOR CLARITY.

TYPICAL UNLESS NOTED OTHERWISE.

ALL DIMENSIONS SHOWN ARE

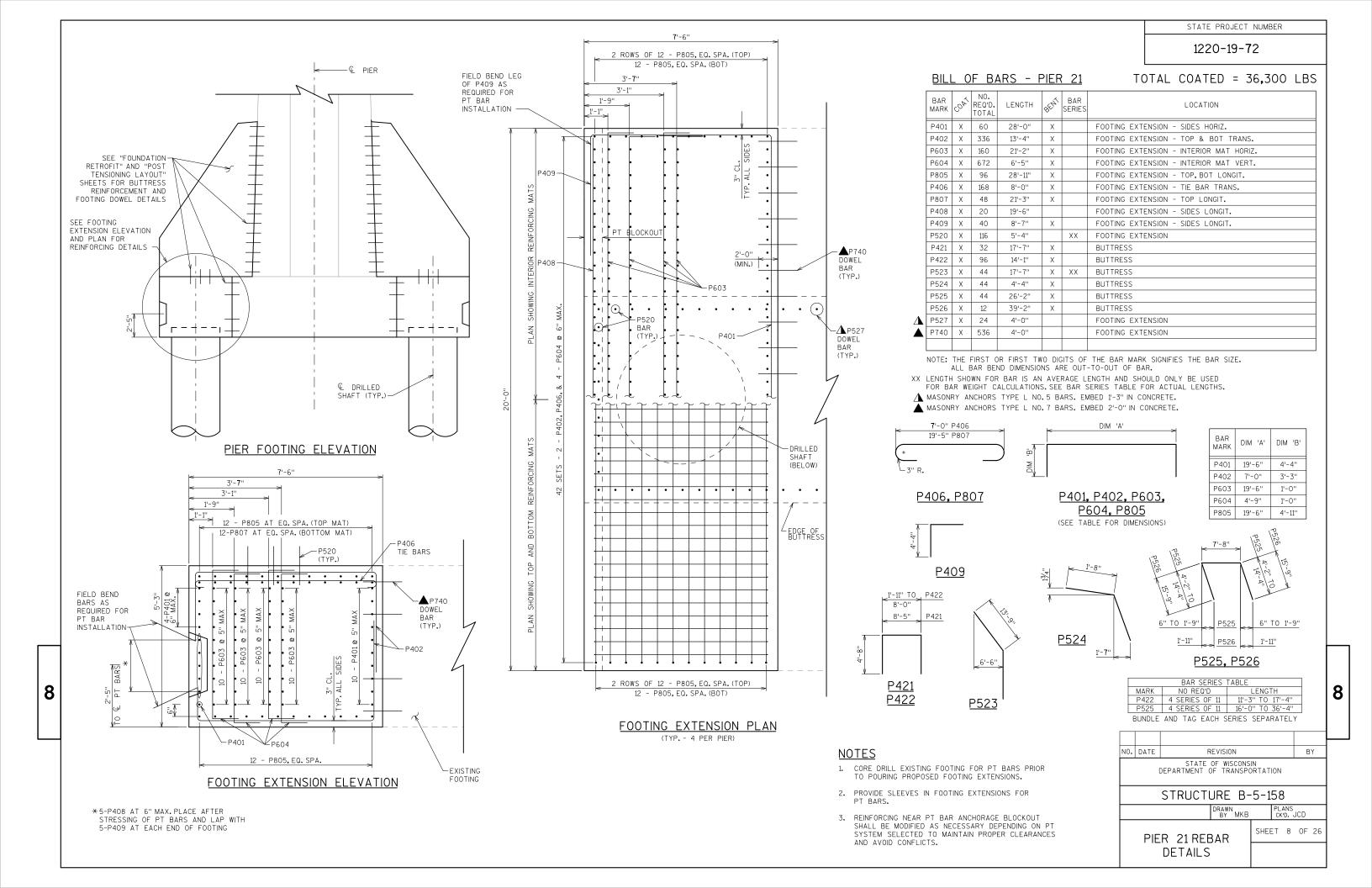
ANCHOR PLATE (TYP.)

RECOMMENDATION)

SECTION C-C

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE. STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION STRUCTURE B-5-158 PLANS CK'D. JWB SHEET 7 OF 26

PIER 21 POST TENSIONING LAYOUT



DOWEL NOTES

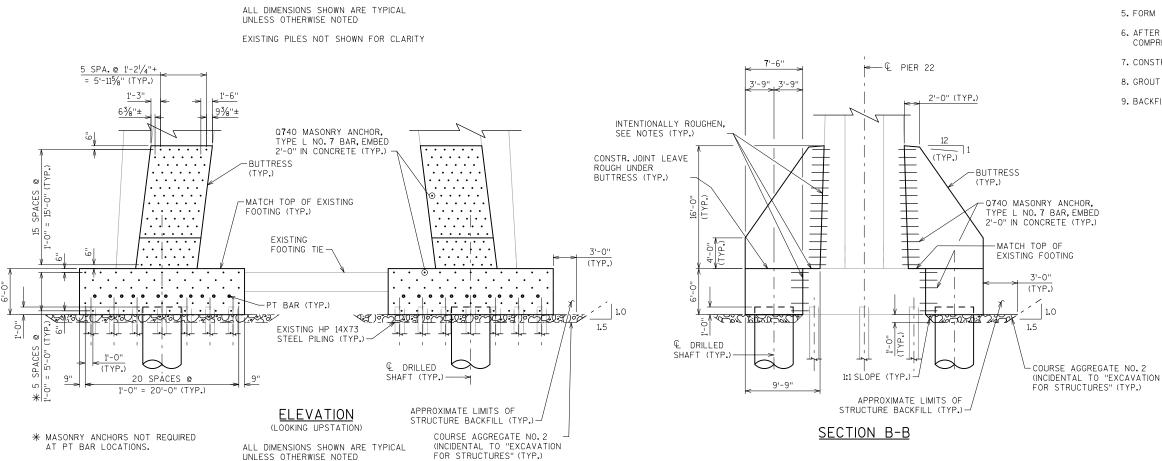
- 1. DURING FIELD DRILLING OF HOLES FOR MASONRY ANCHORS, TYPE L, NO. 7 BARS, IF EXISTING REINFORCING STEEL IS ENCOUNTERED, THE HOLE SHALL BE SHIFTED TO MISS THE REINFORCEMENT.
- 2. INTENTIONALLY ROUGHEN SURFACES WHERE MASONRY ANCHORS ARE INSTALLED TO AN AMPLITUDE OF 0.25 IN. COST OF FURNISHING INTENTIONALLY ROUGHENED SURFACES SHALL BE INCIDENTAL TO THE PRICE BID FOR "REMOVING OLD STRUCTURE STA 843+82".
- 3. DRILL MASONRY ANCHORS AFTER CORING FOR POST TENSIONING.

NOTES

- CLEAN EXPOSED SURFACES OF EXISTING FOOTING TO REMOVE DIRT, LAITANCE AND ANY OTHER DELETERIOUS MATERIALS PRIOR TO POURING THE PROPOSED FOOTING EXTENSIONS.
- 2. BUTTRESS CONCRETE SHALL NOT BE PLACED UNTIL ALL OF THE POST TENSIONING BARS HAVE BEEN JACKED AND LOCKED-OFF PER THE TENSIONING LAYOUT SHEET.

PIER RETROFIT CONSTRUCTION SEQUENCE

- 1. EXCAVATE FOR FOOTING EXTENSIONS
- 2. CORE EXISTING FOOTING FOR PT BARS AND DRILL HOLES FOR MASONRY ANCHORS
- 3. INSTALL DRILLED SHAFTS
- 4. INSTALL REINFORCING BARS IN MASONRY ANCHOR HOLES
- 5. FORM AND PLACE FOOTING EXTENSION CONCRETE
- 6. AFTER NEW FOOTING EXTENSION CONCRETE ATTAINS 4 KSI COMPRESSIVE STRENGTH, STRESS PT BARS
- 7. CONSTRUCT BUTTRESS
- 8. GROUT DUCTS AND PT ANCHORAGE POCKETS
- 9. BACKFILL EXCAVATION



ALL DIMENSIONS SHOWN ARE TYPICAL

UNLESS OTHERWISE NOTED

-BUTTRESS (TYP.)

21'-6"

10'-9"

7'-33/4"

DRILLED SHAFT

3'-5¹/₄"

10'-9'

6'-2¹/₄" 4'-6³/₄"

R N.B. LANE IH 43-

PIER 22

 $40'-2\frac{1}{2}"\pm$

PLAN

EXISTING PILES NOT SHOWN FOR CLARITY

€ OF CAP→

LEXISTING PIER

COLUMN (TYP.)

← € DRILLED SHAFT

DRILLED SHAFT

SEE NOTES (TYP.)-

8

NO. DATE REVISION BY

STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158

PIER 22 FOUNDATION RETROFIT PLANS CKD. SJH SHEET 9 OF 26

PIER 22 - POST TENSIONING

FOOTING	TENDON	BAR STRESSING LENGTH	AREA OF BAR	JACKING FORCE	LOCK-OFF FORCE
		L	А	JF	LOF
		(FT)	(IN2)	(KIPS)	(KIPS)
PIER 22	ALL	30.00	5.16	554.0	540.7

NOTE: THE LOCK-OFF FORCE HAS BEEN CALCULATED AS FOLLOWS: LOF = JF - $((E \times A)/(12 \times L)) \times AS$

LOCK-OFF FORCE (KIPS) LOF JACKING FORCE (KIPS) MODULUS OF ELASTICITY (29,700 KSI) AREA OF BAR (IN2) BAR STRESSING LENGTH (FT)

ANCHOR SET (1/32 IN)

NOTES

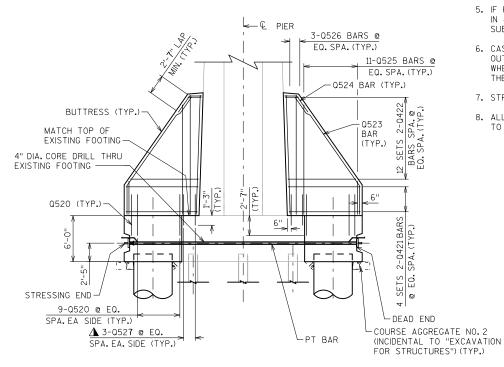
- 1. CORE DRILL HOLES FOR PT BARS IN EXISTING FOOTING FOLLOWING EXCAVATION AND PRIOR TO DRILLED SHAFT PLACEMENT AND PRIOR TO POURING FOOTING EXTENSIONS.
- 2. PROVIDE DUCTS IN FOOTING EXTENSIONS FOR PT BARS.
- 3. ALL PT BARS ARE $2^{1}/2^{11}$ DIAMETER, ASTM A 722, GR 150, EPOXY COATED.
- 4. SPIRAL AND OTHER LOCAL ANCHORAGE ZONE REINFORCEMENT IS TO BE DESIGNED BY THE POST-TENSIONING SUPPLIER AND TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 5. IF FIELD CONDITIONS DIFFER FROM ASSUMPTIONS FOR PT. ADJUSTMENT IN JACK FORCE AND ELONGATION SHALL BE MADE ACCORDINGLY AND BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 6. CAST ANCHOR FACES PERPENDICULAR TO PT DUCTS WITH NO OUT-OF-PLANENESS. A THIN LAYER OF MORTAR MAY BE APPLIED WHEN SETTING THE ANCHOR PLATE, SUBJECT TO APPROVAL OF
- 7. STRENGTH OF CONCRETE AT TIME OF POST TENSIONING, f'c = 4 KSI.
- 8. ALL POST TENSIONING MUST BE COMPLETED AND LOCKED OFF PRIOR TO PLACING BUTTRESS CONCRETE.

4" DIA. (ID) PLASTIC DUCT 21/2" PT BAR--4" DIA, CORE DRILL THRU EXISTING FOOTING FOOTING EXTENSION FOOTING

-21/2" PT BAR

TYPICAL DETAIL OF CONNECTION BETWEEN DUCT AND EXISTING CONCRETE

SEAL ALL AROUND BEFORE CONCRETING



⚠ MASONRY ANCHORS TYPE L NO.5 BARS, EMBED 1'-3" IN CONCRETE.

UPSTATION .

SECTION C-C

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.

10'-9'

10'-9'

12 PT BARS

\$\psi\$ \phi\$

1'-0"

CONTINUOUS POCKET WITH PT

BAR AND 23/4" X 12" X 1'-2" ANCHOR PLATE (TYP.)

RECOMMENDATION)

(CONTRACTOR TO DETERMINE

DIMENSIONS PER MANUFACTURER'S

STRESSING

SEQUENCE

2'-0"

2 SPA.@ 1'-6"

ORDER

- € 2½" PT BAR (TYP.)

€ OF CAP →

R N.B. LANE IH 43-

<u>PLAN</u>

ALL DIMENSIONS SHOWN ARE

SHOWN FOR CLARITY.

-BUTTRESS (TYP.)

TOP OF EXISTING

EXISTING

-21/2" PT BAR (TYP.)

2'-0"

← © DRILLED SHAFT

2 SPA.@ 1'-6"

FOOTING TIE

FOOTING (TYP.)

TYPICAL UNLESS NOTED OTHERWISE.

EXISTING PILES AND BUTTRESS NOT

11-Q524 BARS @ EQ.

SPA. TOP (TYP.)

Q526 BARS TOP (TYP.)-

11-0523 BARS @ EQ.

SPA. F.F. (TYP.)

€ DRILLED

11-Q520 @

EQ. SPA. (TYP.)

ELEVATION

(LOOKING UPSTATION)

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.

EXISTING PILES NOT SHOWN

FOR CLARITY.

- DRILLED SHAFT (TYP.)

PROVIDE 4" DIA. (ID) DUCTS IN

FOOTING EXTENSIONS (TYP.)

COURSE AGGREGATE

STRUCTURES") (TYP.)

"EXCAVATION FOR

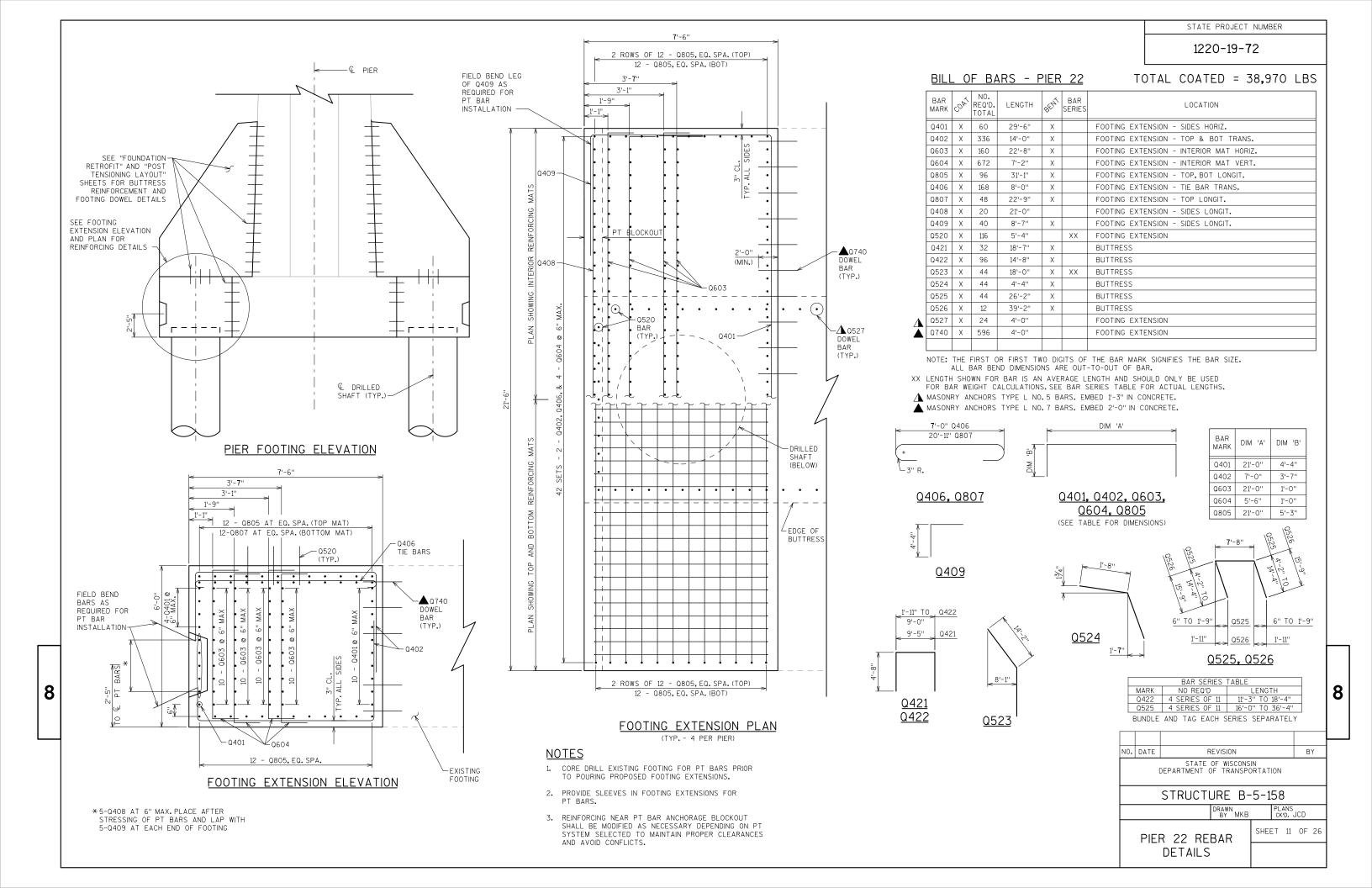
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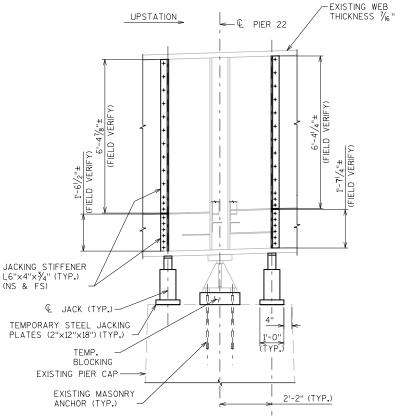
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DEPARTMENT OF TRANSPORTATION STRUCTURE B-5-158 PLANS CK'D. JWB PIER 22 SHEET 10 OF 26 POST TENSIONING

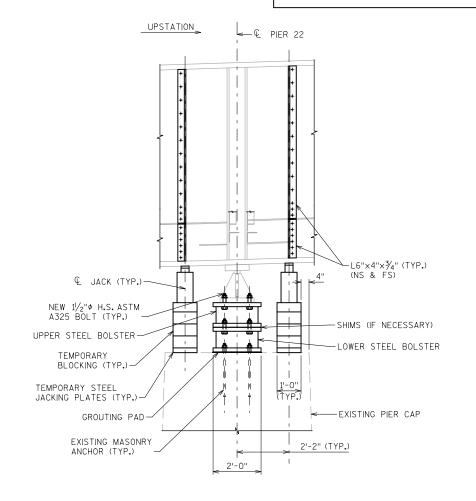
LAYOUT

8



MTB - DENOTES MILL TO BEAR WEATHERING STEEL SHIMS ALLOWED





JACKING - FINAL CONDITION

JACKING - INITIAL CONDITION

JACKING - INTERMEDIATE CONDITION

JACKING PROCEDURE

1. ATTACH EXISTING BEARING TO GIRDER FOR JACKING.

NOTE 1. EXISTING BEARING TO BE CLAMPED TO

BOTTOM FLANGE DURING JACKING.

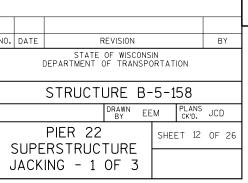
- 2. PERFORM LOWER LATERAL BRACING MODIFICATION PROCEDURE. JACKING STIFFENERS ARE INSTALLED AS PART OF THE LOWER LATERAL BRACING MODIFICATION PROCESS. SEE SHEET 14.
- 3. LEAVE THE EXISTING LATERAL STABILITY BRACKETS IN PLACE. AS THE BRIDGE IS BEING JACKED, ADD SHIMS TO THE PLATES AGAINST THE PIER CAP TO KEEP CONSTANT CONTACT.
- 4. REMOVE NUTS FROM ANCHOR BOLTS.
- 5. SET STEEL JACKING PLATES AND JACKS.
- 6. JACK ALL 5 GIRDERS ON NORTH BOUND SIDE OF THE BRIDGE CONCURRENTLY TO THE MOVEMENT CAPACITY OF THE JACKS, UNLESS BOTH BOUNDS ARE JACKED CONCURRENTLY.
- 7. PLACE TEMPORARY BLOCKS UNDER EXISTING BEARINGS.
- 8. LOWER GIRDERS AND BEARINGS ONTO TEMPORARY BLOCKING.
- 9. RESET JACKS WITH TEMPORARY BLOCKS.
- 10. JACK AGAIN TO MOVEMENT CAPACITY OF JACKS.
- 11. ADD ADDITIONAL BLOCKING UNDER BEARING.
- 12. LOWER GIRDERS AND BEARINGS ONTO TEMPORARY BLOCKING.
- 13. WHEN THE BRIDGE IS JACKED HIGH ENOUGH, REMOVE TEMPORARY BLOCKING FROM UNDER THE BEARING AND INSTALL LOWER SECTION OF BOLSTER, INSTALL LEVEL WITH USE OF LEVELING NUTS UNDER BOTTOM PLATE OF BOLSTER AND GROUT.

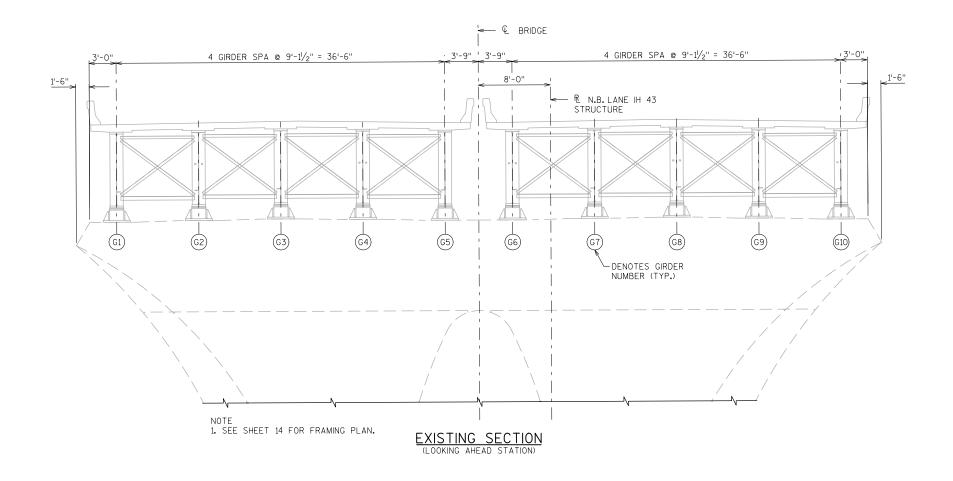
JACKING PROCEDURE (CONTINUED)

- 14. REPEAT THESE STEPS UNTIL GIRDERS ARE ELEVATED TO FINAL LOCATION PLUS 1/4".
- 15. WHEN THE BRIDGE IS JACKED TO ITS HIGHEST POSITION, REMOVE TEMPORARY BLOCKING BENEATH BEARING.
- 16. INSTALL UPPER SECTION OF STEEL BOLSTERS BY BOLTING TO ORIGINAL MASONRY PLATES OF BEARING AND TO LOWER SECTION OF STEEL BOLSTER THAT HAS BEEN PREVIOUSLY INSTALLED. USE SHIM PLATES AS NECESSARY BETWEEN UPPER AND LOWER BOLSTER SECTIONS TO MAINTAIN PROPER ELEVATIONS.
- 17. REMOVE JACKS AND TEMPORARY BLOCKING.
- 18. REMOVE TEMPORARY ATTACHMENT OF BEARINGS TO GIRDERS.
- 19. REMOVE LATERAL STABILITY BRACKETS. JACKING STIFFENERS SHALL BE LEFT IN PLACE.
- 20. APPLY PRIME COAT AND INTERMEDIATE EPOXY COAT OF PAINT TO STEEL BOLSTER.
- 21. PAINT THE NEW JACKING STIFFENERS, HIGH STRENGTH BOLTS, NUTS AND WASHERS CONNECTING THE BEARING TO THE BOLSTER TO MATCH THE EXISTING PAINT ON THE BRIDGE.
- 22. FOLLOW THE SAME PROCEDURE FOR THE OTHER SIDE OF THE BRIDGE (UNLESS ALL 10 GIRDERS WERE JACKED SIMULTANEOUSLY).
- 23. INSTALL CONCRETE PEDESTAL AROUND BOLSTERS.

<u>NOTES</u>

- JACKING PROCEDURE SHALL NOT BEGIN UNTIL PIER 22 FOUNDATION RETROFIT IS COMPLETED. JACKING SHALL NOT START UNTIL APPROVAL IS GIVEN BY THE ENGINEER.
- 2. ALL BOLTS TO BE H.S. 7/8"\$\phi\$ ASTM A325.
- 3. ALL STEEL ANGLES ARE TO BE ASTM A709 GRADE 50 WEATHERING STEEL.
- 4. THE JACKING LOAD IS ESTIMATED TO BE 183 KIPS AT EACH JACK LOCATION.
- 5. PROVIDE JACKS WITH A MINIMUM CAPACITY OF 150% OF REQUIRED JACKING LOAD.
- 6. APPROXIMATE JACKING LOAD IS UNFACTORED AND INCLUDES ONLY THE APPROXIMATE DEAD LOAD OF EXISTING STRUCTURE.
- 7. SEE SHEET 3 FOR DETAILS OF LATERAL STABILITY BRACKETS.





PROCEDURE TO DETERMINE AMOUNT TO JACK SUPERSTRUCTURE AT & OF PIER 22

- 1. SURVEY TOP OF DECK ELEVATION AT EACH GIRDER AT $\mathbb Q$ PIER 21 (STA. 839 + 81.00).
- 2. SURVEY TOP OF DECK ELEVATION AT EACH GIRDER AT $\ensuremath{\mathbb{Q}}$ PIER 23 (STA. 843 + 82.00).
- 3. ADD 5.43 TO THE SURVERY ELEVATION AT PIER 21.
- 4. SUBTRACT 6.17 FROM THE SURVEY ELEVATION AT PIER 23.
- 5. AVERAGE THESE TWO NUMBERS.
- 6. JACK GIRDERS SO THAT THE ELEVATION AT EACH GIRDER AT $\mathbb Q$ PIER 22 (STA. 841 + 62.00) EQUALS THIS ELEVATION.

NO. DATE STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158

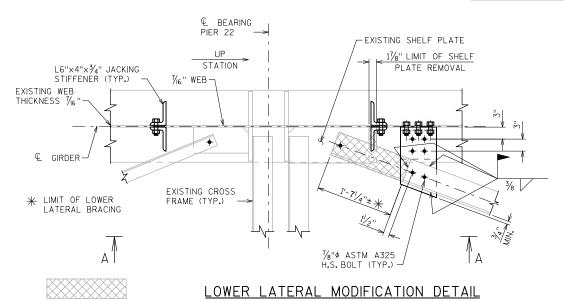
DRAWN EEM PLANS JCD

8

PIER 22 SUPERSTRUCTURE

SHEET 13 OF 26 JACKING - 2 OF 3





DENOTES PORTION OF LOWER LATERAL BRACING AND SHELF PLATE TO BE REMOVED

- PIER 22 STA. 841+62.00 24'-93/8"

← € CATWALK

4'-105/8''

PARTIAL FRAMING PLAN

44'-0"

24'-93/8"

€ FIELD

SPLICE →

842+00 NB

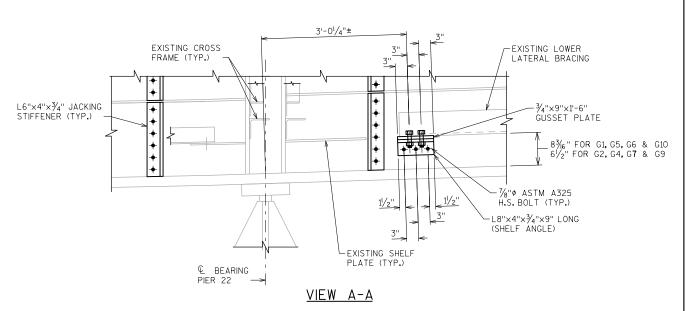
€ EXISTING CROSS

FRAME (TYP.)

STATION

-R N.B. LANE IH 43

STRUCTURE



LATERAL BRACING REMOVAL AND INSTALLATION PROCEDURE

- 1. UNBOLT THE EXISTING LATERAL BRACING ANGLE FROM THE EXISTING SHELF PLATE
- 2. REMOVE THE EXISTING WELD AROUND THE EXISTING LATERAL BRACING ANGLE.
- 3. REMOVE THE PORTION OF THE EXISTING LATERAL BRACING ANGLE AND SHELF PLATE AS INDICATED.
- 4. GRIND SMOOTH THE AREA ON THE GIRDER WEB WHERE THE EXISTING SHELF PLATE WAS REMOVED.
- 5. INSTALL THE NEW JACKING STIFFENERS AND BOLTS.
- 6. INSTALL THE NEW SHELF ANGLE AND BOLTS.
- 7. INSTALL THE NEW GUSSET PLATE AND BOLTS.
- 8. CONNECT THE EXISTING LATERAL BRACING ANGLE TO THE GUSSET PLATE USING NEW BOLTS.
- 9. ONCE THE EXISTING LATERAL BRACING ANGLE IS BOLTED IN PLACE, WELD AROUND THE ANGLE AS INDICATED.

NOTES

1. ALL STEEL TO BE ASTM A709 GRADE 50 WEATHERING STEEL.

8

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STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158

DRAWN EEM PLANS JCD CK.D. SHEET 14 OF 26

SUPERSTRUCTURE
JACKING - 3 OF 3

8

DENOTES GIRDER

G3) + - -

(8) | - -

(9) + - -

LLM - DENOTES LOCATION OF LOWER

LATERAL BRACING MODIFICATION

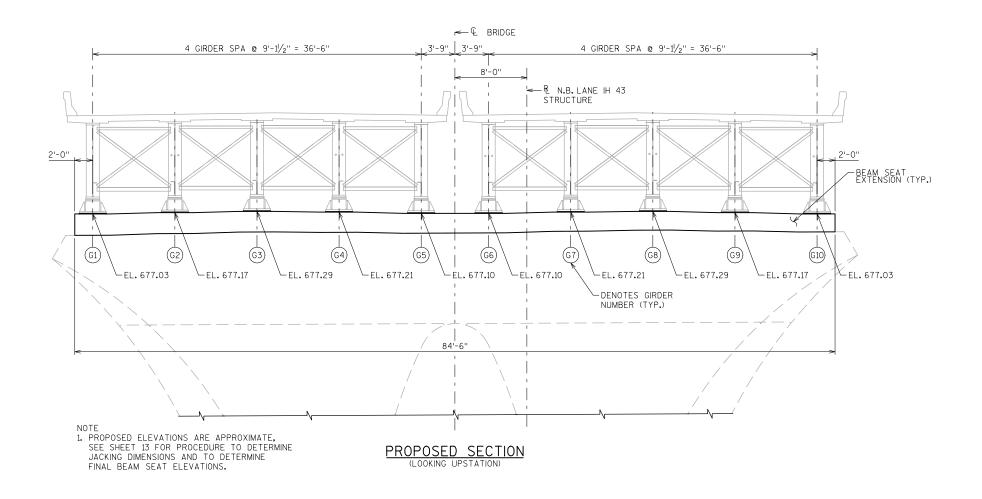
NUMBER (TYP.)

FIELD SPLICE

SOUTHBOUND

NORTHBOUND.

36'-0"



EXISTING BEAM SEAT ELEVATIONS (SEE NOTE 3)

BEAM	ELEVATION
G1	674.79
G2	674.96
G3	675.13
G4	675.11
G5	675.00
G6	675.04
G7	675.23
G8	675.35
G9	675.26
G10	675.13

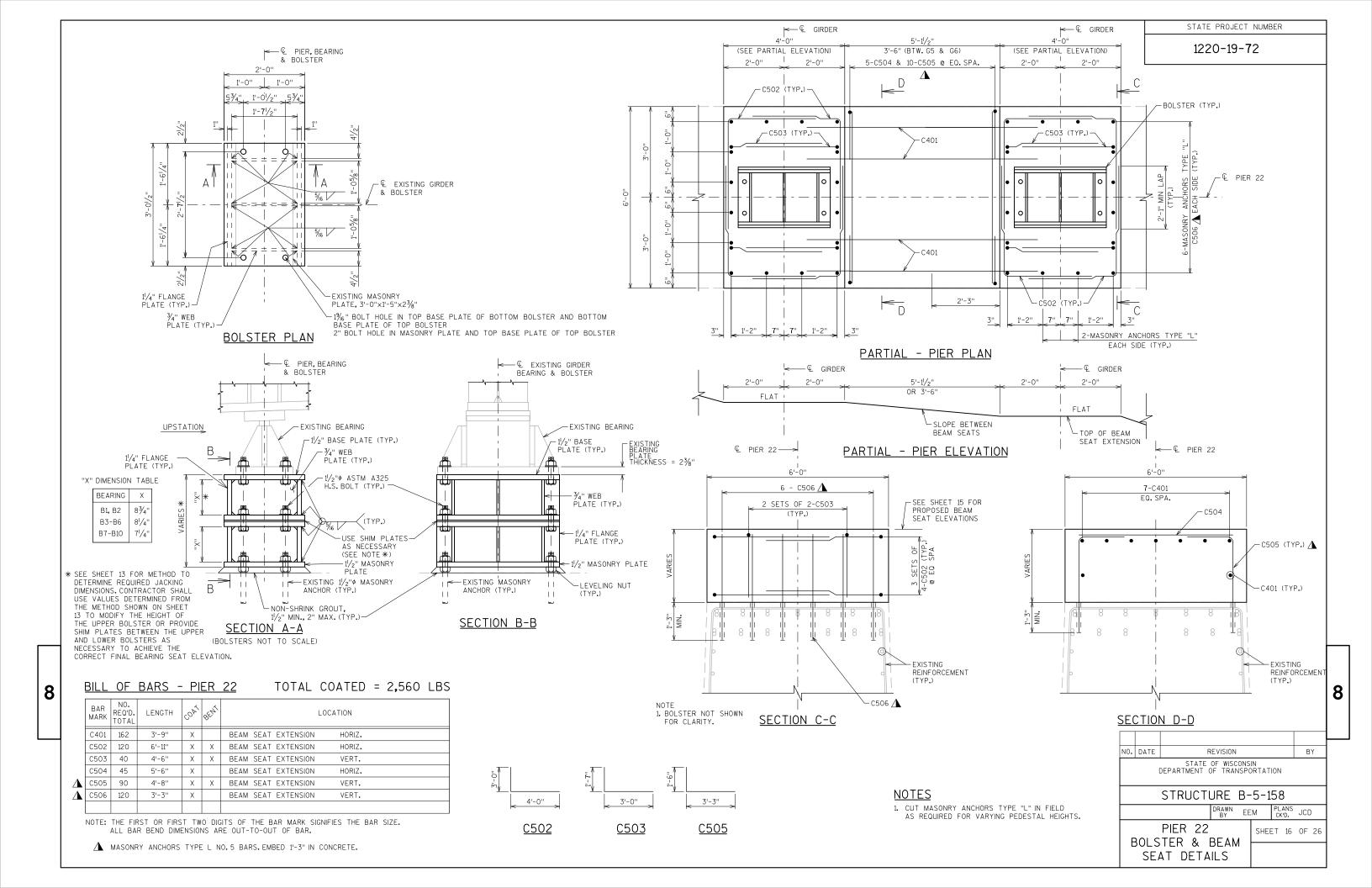
<u>NOTES</u>

- 1. SEE SHEET 16 FOR BOLSTER AND BEAM SEAT DETAILS.
- 2. SEE SHEET 14 FOR FRAMING PLAN.
- 3. EXISTING BEAM SEAT ELEVATIONS ARE BASED ON SURVEY TAKEN 9/25/13. CONTRACTOR IS RESPONSIBLE FOR VERIFYING EXISTING BEAM SEAT ELEVATIONS AT TIME OF JACKING PROCEDURE.

NO.	DATE	F	REVISION					
	STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION							
	STRUCTURE B-5-158							
	DRAWN EEM PLANS							
PII	ER 2	22 BEAM	SEAT	SHEE	ET 15	OF 26		
	_	— — –						

EXTENSION

8





DOWEL NOTES

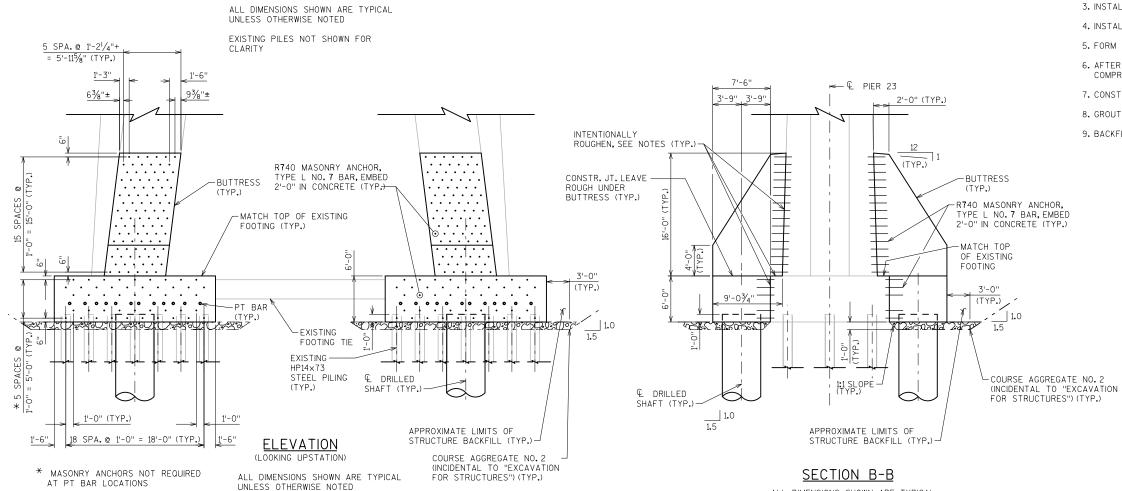
- 1. DURING FIELD DRILLING OF HOLES FOR MASONRY ANCHORS, TYPE L NO.7 BARS, IF EXISTING REINFORCING STEEL IS ENCOUNTERED, THE HOLE SHALL BE SHIFTED TO MISS THE REINFORCEMENT.
- 2. INTENTIONALLY ROUGHEN SURFACES WHERE MASONRY
 ANCHORS ARE INSTALLED TO AN AMPLITUDE OF 0.25 IN.
 COST OF FURNISHING INTENTIONALLY ROUGHEN SURFACES SHALL BE INCIDENTAL TO THE PRICE BID FOR "REMOVING OLD STRUCTURE STATION
- 3. DRILL MASONRY ANCHORS AFTER CORING FOR POST TENSIONING.

<u>NOTES</u>

- 1. CLEAN EXPOSED SURFACES OF EXISTING FOOTING TO REMOVE DIRT, LAITANCE AND ANY OTHER DELETERIOUS MATERIALS PRIOR TO POURING THE PROPOSED FOOTING EXTENSIONS.
- 2. BUTTRESS CONCRETE SHALL NOT BE PLACED UNTIL ALL OF THE POST TENSIONING BARS HAVE BEEN JACKED AND LOCKED-OFF PER THE PROPOSED POST TENSIONING LAYOUT SHEETS.

PIER RETROFIT CONSTRUCTION SEQUENCE

- 1. EXCAVATE FOR FOOTING EXTENSIONS
- 2. CORE EXISTING FOOTING FOR PT BARS AND DRILL HOLES FOR MASONRY ANCHORS
- 3. INSTALL DRILLED SHAFTS
- 4. INSTALL REINFORCING BARS IN MASONRY ANCHOR HOLES
- 5. FORM AND PLACE FOOTING EXTENSION CONCRETE
- 6. AFTER NEW FOOTING EXTENSION CONCRETE ATTAINS 4 KSI COMPRESSIVE STRENGTH, STRESS PT BARS
- 7. CONSTRUCT BUTTRESS
- 8. GROUT DUCTS AND PT ANCHORAGE POCKETS
- 9. BACKFILL EXCAVATION



ALL DIMENSIONS SHOWN ARE TYPICAL

UNLESS OTHERWISE NOTED

DRILLED SHAFT SEE NOTES (TYP.)

EXISTING PIER

COLUMN (TYP.)

_ € PER 23

43'-1[|]/₄''±

<u>PLAN</u>

├── © DRILLED SHAFT

R N.B. LANE IH 43-

OF CAP

BUTTRESS-

(TYP.)

21'-0"

8'-0"

4'-0" | 4'-0"

6'-6"

10'-6"

6'-6"

— € DRILLED SHAFT

В

NO. DATE STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158

PIER 23 FOUNDATION RETROFIT

PLANS CK'D. SJH SHEET 17 OF 26

PIER 23 - POST TENSIONING

FOOTING	TENDON	BAR STRESSING LENGTH	AREA OF BAR	JACKING FORCE	LOCK-OFF FORCE
		L	А	JF	LOF
		(FT)	(IN2)	(KIPS)	(KIPS)
PIER 23	ALL	29.5	5.16	554.0	540

NOTE: THE LOCK-OFF FORCE HAS BEEN CALCULATED AS FOLLOWS:

LOF = JF - ((E \times A)/(12 \times L)) \times AS

LOF LOCK-OFF FORCE (KIPS)
JF JACKING FORCE (KIPS)
E MODULUS OF ELASTICITY (29,700 KSI)
A AREA OF BAR (IN2)
L BAR STRESSING LENGTH (FT)
AS ANCHOR SET (1/32 IN)

NOTES

- CLEAN EXPOSED SURFACES OF EXISTING FOOTING TO REMOVE DIRT, LAITANCE AND ANY OTHER DELETERIOUS MATERIALS PRIOR TO POURING THE FOOTING EXTENSIONS.
- 2. CORE DRILL HOLES FOR PT BARS IN EXISTING FOOTING FOLLOWING EXCAVATION AND PRIOR TO DRILLED SHAFT PLACEMENT AND PRIOR TO POURING FOOTING EXTENSIONS.
- 3. PROVIDE DUCTS IN FOOTING EXTENSIONS FOR PT BARS.
- 4. ALL PT BARS ARE $2\frac{1}{2}$ " DIAMETER, ASTM A 722, GR 150, EPOXY COATED.
- 5. SPIRAL AND OTHER LOCAL ANCHORAGE ZONE REINFORCEMENT IS TO BE DESIGNED BY THE POST-TENSIONING SUPPLIER AND TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 6. IF FIELD CONDITIONS DIFFER FROM ASSUMPTIONS FOR PT, ADJUSTMENT IN JACK FORCE AND ELONGATION SHALL BE MADE ACCORDINGLY AND BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 7. CAST ANCHOR FACES PERPENDICULAR TO PT DUCTS WITH NO OUT-OF-PLANENESS. A THIN LAYER OF MORTAR MAY BE APPLIED WHEN SETTING THE ANCHOR PLATE, SUBJECT TO APPROVAL OF THE FNGINFER.
- 8. STRENGTH OF CONCRETE AT TIME OF POST TENSIONING, f'c = 4 KSI.
- 9. ALL POST TENSIONING MUST BE COMPLETED AND LOCKED OFF PRIOR TO PLACING BUTTRESS CONCRETE.

4" DIA. (ID)
PLASTIC DUCT

21/2" PT BAR

4" DIA. CORE DRILL THRU
EXISTING FOOTING

EXTENSION

SEAL ALL AROUND

21/2" PT BAR

TYPICAL DETAIL OF CONNECTION BETWEEN DUCT AND EXISTING CONCRETE

BEFORE CONCRETING

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.

<u>PLAN</u>

-€ 2½"PT BAR (TYP.)

12 PT BARS

-CONTINUOUS POCKET WITH PT

BAR AND 23/4" X 12" X 1'-2" ANCHOR PLATE (TYP.)

RECOMMENDATION)

(CONTRACTOR TO DETERMINE

DIMENSIONS PER MANUFACTURER'S

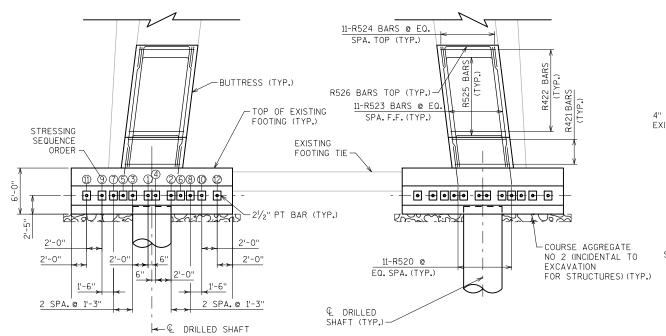
-DRILLED SHAFT (TYP.)

PROVIDE 4" DIA. (ID) DUCTS IN

FOOTING EXTENSIONS (TYP.)

R N.B. LANE IH 43

EXISTING PILES AND BUTTRESS NOT SHOWN FOR CLARITY.



ELEVATION

TYPICAL UNLESS NOTED OTHERWISE.

ALL DIMENSIONS SHOWN ARE

EXISTING PILES NOT SHOWN FOR CLARITY.

NOTES

← € PIER 3-R526 BARS @ EQ. SPA. (TYP.) 11-R525 BARS @ EQ. SPA. (TYP.) R524 BAR (TYP.) BUTTRESS (TYP.)-R523 MATCH TOP OF EXISTING FOOTING (TYP.) 4" DIA. CORE DRILL THRU EXISTING FOOTING R520 (TYP.)-2-R421 BARS SPA. (TYP.) STRESSING END-9-R520 @ EQ. SPA. EA SIDE (TYP.) -COURSE AGGREGATE NO 2 ▲ 3-R527 @ EQ. (INCIDENTAL TO EXCAVATION SPA. EA. SIDE (TYP.)

⚠ MASONRY ANCHORS TYPE L NO.5 BARS, EMBED 1'-3" IN CONCRETE.

SECTION C-C

NOTE ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE. UPSTATION

NO. DATE REVISION BY

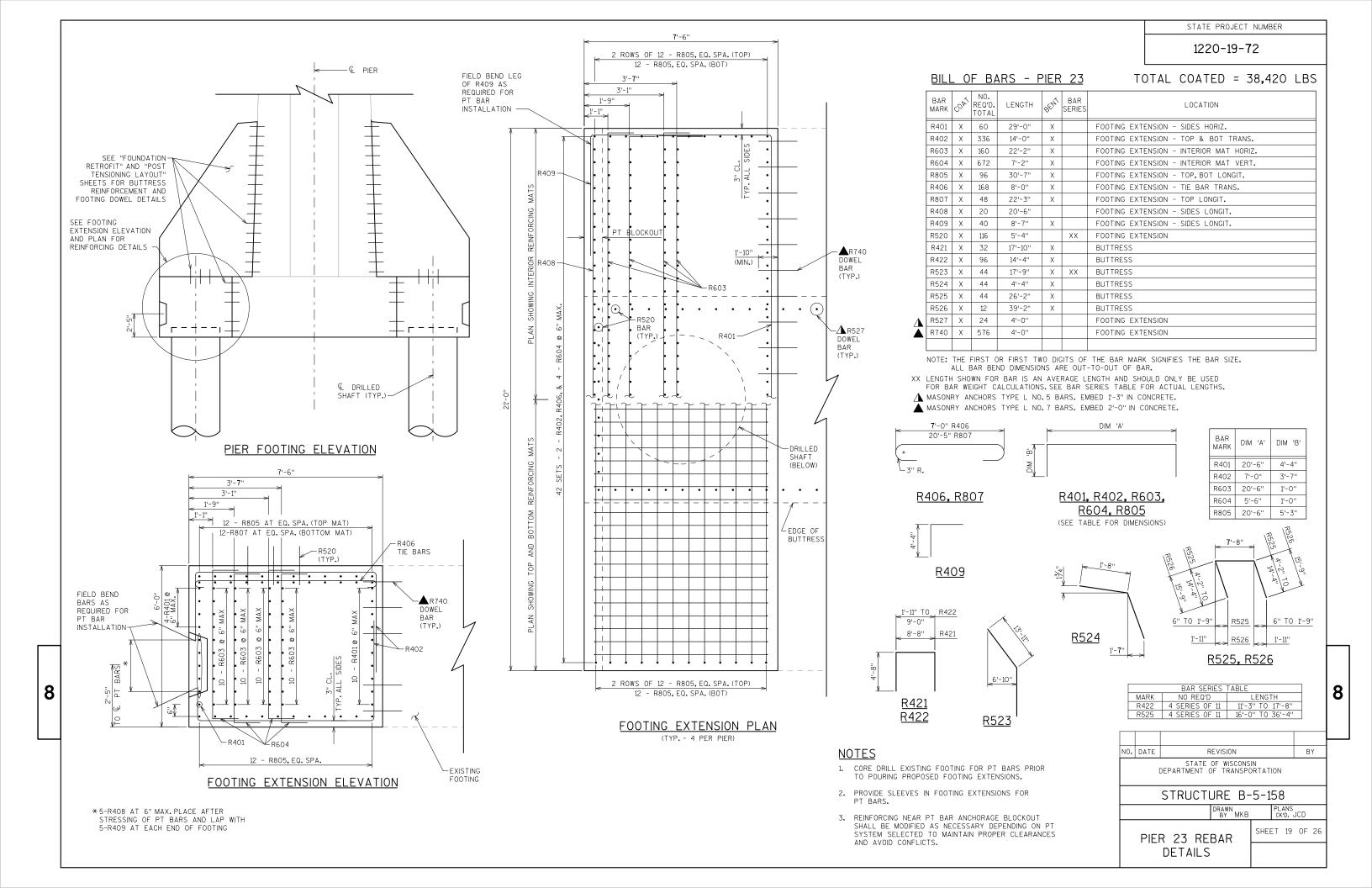
STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION

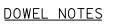
STRUCTURE B-5-158

PIER 23
POST TENSIONING
LAYOUT

SHEET 18 OF 26

8





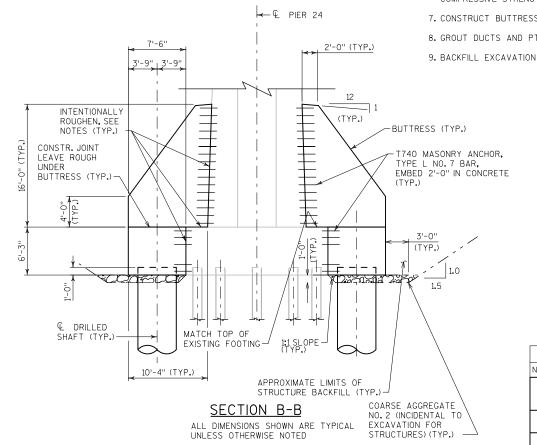
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- 2. INTENTIONALLY ROUGHEN SURFACES WHERE MASONRY ANCHORS ARE INSTALLED TO AN AMPLITUDE OF 0.25 IN. COST OF FURNISHING INTENTIONALLY ROUGHEN SURFACES SHALL BE INCIDENTAL TO THE PRICE BID "FOR REMOVING OLD STRUCTURE STATION 843+82".
- 3. DRILL MASONRY ANCHORS AFTER CORING FOR POST TENSIONING.

NOTES

- 1. CLEAN EXPOSED SURFACES OF EXISTING FOOTING TO REMOVE DIRT, LAITANCE AND ANY OTHER DELETERIOUS MATERIALS PRIOR TO POURING THE PROPOSED FOOTING EXTENSIONS.
- 2. BUTTRESS CONCRETE SHALL NOT BE PLACED UNTIL ALL OF THE POST TENSIONING BARS HAVE BEEN JACKED AND LOCKED-OFF PER THE PROPOSED POST TENSIONING LAYOUT SHEET.

PIER RETROFIT CONSTRUCTION SEQUENCE

- 1. EXCAVATE FOR FOOTING EXTENSIONS
- 2. CORE EXISTING FOOTING FOR PT BARS AND DRILL HOLES FOR MASONRY ANCHORS
- 3. INSTALL DRILLED SHAFTS
- 4. INSTALL REINFORCING BARS IN MASONRY ANCHOR HOLES
- 5. FORM AND PLACE FOOTING EXTENSION CONCRETE
- 6. AFTER NEW FOOTING EXTENSION CONCRETE ATTAINS 4 KSI COMPRESSIVE STRENGTH, STRESS PT BARS
- 7. CONSTRUCT BUTTRESS
- 8. GROUT DUCTS AND PT ANCHORAGE POCKETS



8

* MASONRY ANCHORS NOT REQUIRED AT PT BAR LOCATIONS.

1'-0" (TYP.)

18 SPACES @

1'-0" = 18'-0" (TYP.)

ELEVATION (LOOKING UPSTATION: ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS OTHERWISE NOTED

APPROXIMATE LIMITS OF STRUCTURE BACKFILL (TYP.) COARSE AGGREGATE NO. 2 - (INCIDENTAL TO EXCAVATION FOR STRUCTURES) (TYP.)

← € DRILLED SHAFT (TYP.)

10'-6" 6'-6"

R N.B. LANE IH 43-

44'-1[|]/2"±

<u>PLAN</u>

-BUTTRESS (TYP.)

-MATCH TOP OF EXISTING FOOTING (TYP.)

-PT BAR (TYP.) 7.480

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS OTHERWISE NOTED

EXISTING PILES NOT SHOWN FOR CLARITY

T740 MASONRY ANCHOR,

TYPE L NO. 7 BAR, EMBED

2'-0" IN CONCRETE (TYP.)

EXISTING HP14X73 STEEL PILING (TYP.)-

EXISTING PIER COLUMN (TYP.) € OF CAP→

← € DRILLED SHAFT

24 PIER

DRILLED SHAFT

SEE NOTES (TYP.)

5 SPA.@ 1'-21/4"+

= 5'-11⁵/₈" (TYP.)

1'-3"

63/8"± _

1'-6"

93/8"±

8'-0"

4'-0" 4'-0"

В

B

-BUTTRESS (TYP.)

6'-6"

← € DRILLED SHAFT

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> STRUCTURE B-5-158 PLANS CK'D. SJH

PIER 24 FOUNDATION RETROFIT

SHEET 20 OF 26

PIER 24 - POST TENSIONING

FOOTING	TENDON	BAR STRESSING LENGTH	AREA OF BAR	JACKING FORCE	LOCK-OFF FORCE
		L	А	JF	LOF
		(FT)	(IN2)	(KIPS)	(KIPS)
PIER 24	ALL	32,50	5.16	554.0	541.7

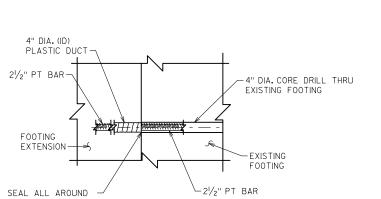
NOTE: THE LOCK-OFF FORCE HAS BEEN CALCULATED AS FOLLOWS:

 $LOF = JF - ((E \times A)/(12 \times L)) \times AS$

WHERE,
LOF LOCK-OFF FORCE (KIPS)
JF JACKING FORCE (KIPS)
E MODULUS OF ELASTICITY (29,700 KSI)
A AREA OF BAR (IN2)
L BAR STRESSING LENGTH (FT)
AS ANCHOR SET (1/32 IN)

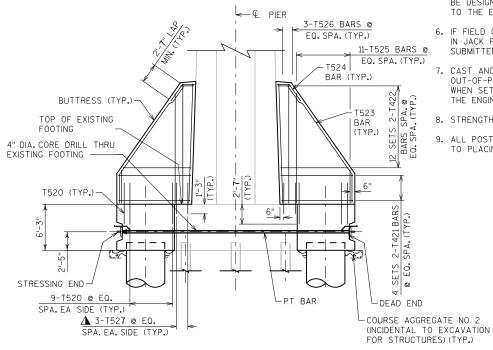
NOTES

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- CORE DRILL HOLES FOR PT BARS IN EXISTING FOOTING FOLLOWING EXCAVATION AND PRIOR TO DRILLED SHAFT PLACEMENT AND PRIOR TO POURING FOOTING EXTENSIONS.
- 3. PROVIDE DUCTS IN FOOTING EXTENSIONS FOR PT BARS.
- 4. ALL PT BARS ARE $2\frac{1}{2}$ " DIAMETER, ASTM A 722, GR 150, EPOXY COATED.
- 5. SPIRAL AND OTHER LOCAL ANCHORAGE ZONE REINFORCEMENT IS TO BE DESIGNED BY THE POST-TENSIONING SUPPLIER AND TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 6. IF FIELD CONDITIONS DIFFER FROM ASSUMPTIONS FOR PT, ADJUSTMENT IN JACK FORCE AND ELONGATION SHALL BE MADE ACCORDINGLY AND BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 7. CAST ANCHOR FACES PERPENDICULAR TO PT DUCTS WITH NO OUT-OF-PLANENESS. A THIN LAYER OF MORTAR MAY BE APPLIED WHEN SETTING THE ANCHOR PLATE, SUBJECT TO APPROVAL OF THE FNGINEER.
- 8. STRENGTH OF CONCRETE AT TIME OF POST TENSIONING, f'c = 4 KSI.
- 9. ALL POST TENSIONING MUST BE COMPLETED AND LOCKED OFF PRIOR TO PLACING BUTTRESS CONCRETE.



TYPICAL DETAIL OF CONNECTION BETWEEN DUCT AND EXISTING CONCRETE

BEFORE CONCRETING



⚠ MASONRY ANCHORS TYPE L NO.5 BARS, EMBED 1'-3" IN CONCRETE. UPSTATION

SECTION C-C

NOTE ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.



STRESSING

SEQUENCE

2 SPA.@ 1'-3"

ORDER -

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STRUCTURE B-5-158

DRAWN
BY
CLG PLANS
CKD. JWB

PIER 24
POST TENSIONING

LAYOUT

8

ELEVATION (LOOKING UPSTATION)

10'-6"

12 PT BARS

-CONTINUOUS POCKET WITH PT

BAR AND 23/4" X 12" X 1'-2"

(CONTRACTOR TO DETERMINE

DIMENSIONS PER MANUFACTURER'S

ANCHOR PLATE (TYP.)

RECOMMENDATION)

0 9753 4 2680 Q

€ 21/2" PT BAR

€ OF CAP ---

<u>PLAN</u>

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.

SHOWN FOR CLARITY.

TOP OF EXISTING FOOTING (TYP.)

-2¹/₂" PT BAR (TYP.)

2 SPA.@ 1'-3"

└── € DRILLED SHAFT

EXISTING PILES AND BUTTRESS NOT

11-T524 BARS @ EQ. SPA. TOP (TYP.)

11-T523 BARS @ EQ.

SPA. FF (TYP.)

11-T520 @

EQ. SPA. (TYP.)

←DRILLED SHAFT (TYP.)

PROVIDE 4" DIA.(ID) DUCTS IN

FOOTING EXTENSIONS (TYP.)

₩ Q DRILLED SHAFT

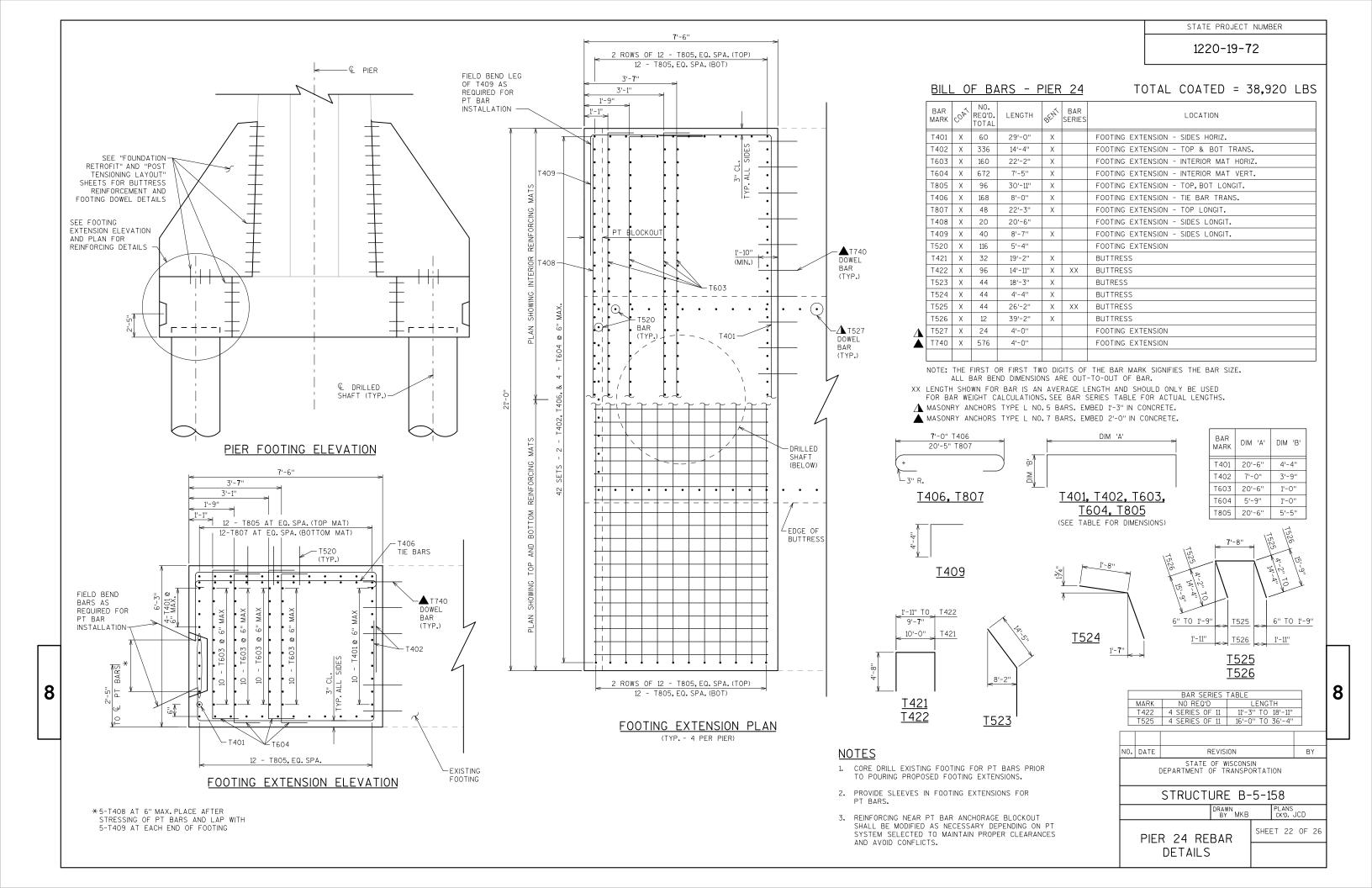
COURSE AGGREGATE NO 2

FOR STRUCTURES) (TYP.)

(INCIDENTAL TO EXCAVATION

NOTE ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.

EXISTING PILES NOT SHOWN FOR CLARITY.



DOWEL NOTES

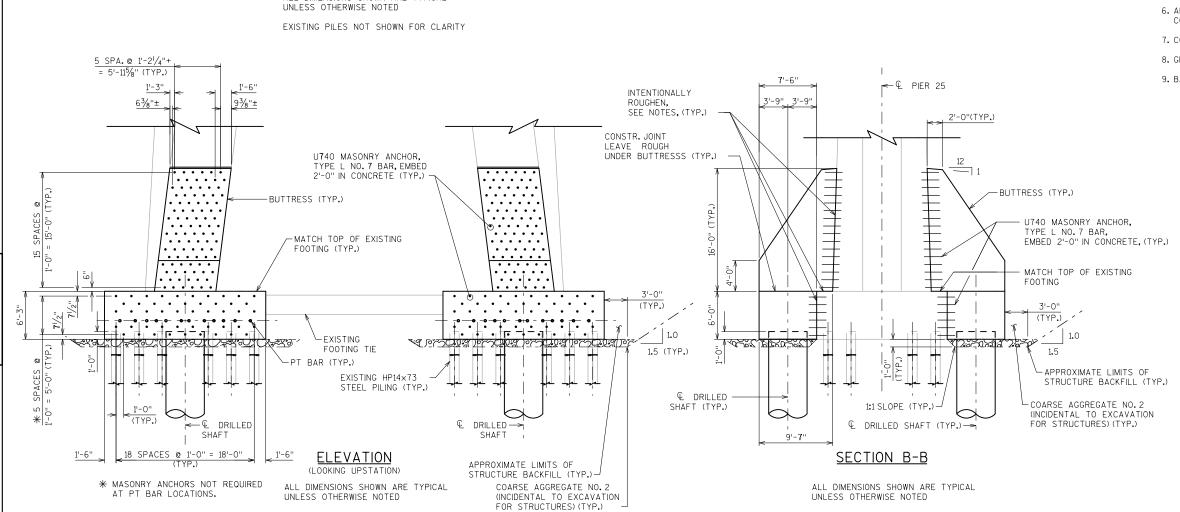
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- 2. INTENTIONALLY ROUGHEN SURFACES WHERE MASONRY ANCHORS ARE INSTALLED TO AN AMPLITUDE OF 0.25 IN. COST OF FURNISHING INTENTIONALLY ROUGHEN SURFACES SHALL BE INCIDENTAL TO THE PRICE BID FOR "REMOVING OLD STRUCTURE STATION 843+82".
- 3. DRILL MASONRY ANCHORS AFTER CORING FOR POST TENSIONING.

NOTES

- 1. CLEAN EXPOSED SURFACES OF EXISTING FOOTING TO REMOVE DIRT, LAITANCE AND ANY OTHER DELETERIOUS MATERIALS PRIOR TO POURING THE PROPOSED FOOTING EXTENSIONS.
- 2. BUTTRESS CONCRETE SHALL NOT BE PLACED UNTIL ALL OF THE POST TENSIONING BARS HAVE BEEN JACKED AND LOCKED-OFF PER THE PROPOSED POST TENSIONING LAYOUT SHEET.

PIER RETROFIT CONSTRUCTION SEQUENCE

- 1. EXCAVATE FOR EXISTING FOOTING EXTENSIONS
- 2. CORE EXISTING FOOTING FOR PT BARS AND DRILL HOLES FOR MASONRY ANCHORS
- 3. INSTALL DRILLED SHAFTS
- 4. INSTALL REINFORCING BARS IN MASONRY ANCHOR HOLES
- 5. FORM AND PLACE FOOTING EXTENSION CONCRETE
- 6. AFTER NEW FOOTING EXTENSION CONCRETE ATTAINS 4 KSI COMPRESSIVE STRENGTH, STRESS PT BARS
- 7. CONSTRUCT BUTTRESS
- 8. GROUT DUCTS AND PT ANCHORAGE POCKETS
- 9. BACKFILL EXCAVATION



21'-0"

8'-0'

4'-0" | 4'-0"

В

10'-6"

6'-6"

DRILLED SHAFT

-BUTTRESS (TYP.)

10'-6"

6'-6"

R N.B. LANE IH 43-

€ OF CAP→

PIER COLUMN

44'-1"±

<u>PLAN</u>

ALL DIMENSIONS SHOWN ARE TYPICAL

(TYP.)

-€ DRILLED SHAFT

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DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158

PIER 25 FOUNDATION RETROFIT

SHEET 23 OF 26

PLANS CK'D. SJH

8

DRILLED SHAFT

SEE NOTES (TYP.)

PIER 25 - POST TENSIONING

FOOTING	TENDON	BAR STRESSING LENGTH	AREA OF BAR	JACKING FORCE	LOCK-OFF FORCE
		L	А	JF	LOF
		(FT)	(IN2)	(KIPS)	(KIPS)
PIER 25	ALL	31.00	5.16	554.0	541.1

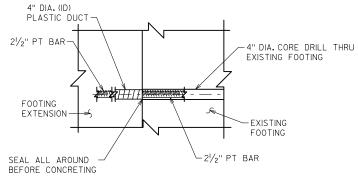
NOTE: THE LOCK-OFF FORCE HAS BEEN CALCULATED AS FOLLOWS:

 $LOF = JF - ((E \times A)/(12 \times L)) \times AS$

LOF LOCK-OFF FORCE (KIPS) JACKING FORCE (KIPS) MODULUS OF ELASTICITY (29,700 KSI) AREA OF BAR (IN2)
BAR STRESSING LENGTH (FT) ANCHOR SET (1/32 IN)

NOTES

- 1. CORE DRILL HOLES FOR PT BARS IN EXISTING FOOTING FOLLOWING EXCAVATION AND PRIOR TO DRILLED SHAFT PLACEMENT AND PRIOR TO POURING FOOTING EXTENSIONS.
- 2. PROVIDE DUCTS IN FOOTING EXTENSIONS FOR PT BARS.
- 3. ALL PT BARS ARE $2\frac{1}{2}$ " DIAMETER, ASTM A 722, GR 150, EPOXY COATED.
- 4. SPIRAL AND OTHER LOCAL ANCHORAGE ZONE REINFORCEMENT IS TO BE DESIGNED BY THE POST-TENSIONING SUPPLIER AND TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 5. IF FIELD CONDITIONS DIFFER FROM ASSUMPTIONS FOR PT, ADJUSTMENT IN JACK FORCE AND ELONGATION SHALL BE MADE ACCORDINGLY AND BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 6. CAST ANCHOR FACES PERPENDICULAR TO PT DUCTS WITH NO OUT-OF-PLANENESS. A THIN LAYER OF MORTAR MAY BE APPLIED WHEN SETTING THE ANCHOR PLATE, SUBJECT TO APPROVAL OF
- 7. STRENGTH OF CONCRETE AT TIME OF POST TENSIONING, f'c = 4 KSI.
- 8. ALL POST TENSIONING MUST BE COMPLETED AND LOCKED OFF PRIOR TO PLACING BUTTRESS CONCRETE.



TYPICAL DETAIL OF CONNECTION BETWEEN DUCT AND EXISTING CONCRETE

11-U524 BARS @ EQ. SPA. TOP (TYP.) -BUTTRESS (TYP.) U526 BARS TOP (TYP.)-11-U523 BARS @ EQ. TOP OF EXISTING SPA. F.F. (TYP.) FOOTING (TYP.) STRESSING -SEQUENCE -EXISTING ORDER FOOTING TIE (1) 9 753 <u>04 268 (0 12</u> -21/2" PT BAR (TYP.) € DRILLED COURSE AGGREGATE SHAFT (TYP.) (INCIDENTAL TO EXCAVATION 11-U520 @ FOR STRUCTURES) EQ. SPA. (TYP.) (TYP.) 1'-6" _ 2 SPA.@ 1'-3" 2 SPA.@ 1'-3" **ELEVATION** ← Q DRILLED (LOOKING AHEAD STATIONS) SHAFT (TYP.)

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.

EXISTING PILES NOT SHOWN FOR CLARITY.

· € 21/2" PT BAR

€ OF CAP ->

R N.B. LANE IH 43-

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE.

<u>PLAN</u>

EXISTING PILES AND BUTTRESS NOT SHOWN FOR CLARITY.

(TYP.)

-DRILLED SHAFT (TYP.)

PROVIDE 4" DIA. (ID) DUCTS IN

FOOTING EXTENSIONS (TYP.)

PF FT

10'-6"

10'-6"

12 PT BARS

-CONTINUOUS POCKET WITH PT

BAR AND 23/4" X 12" X 1'-2"

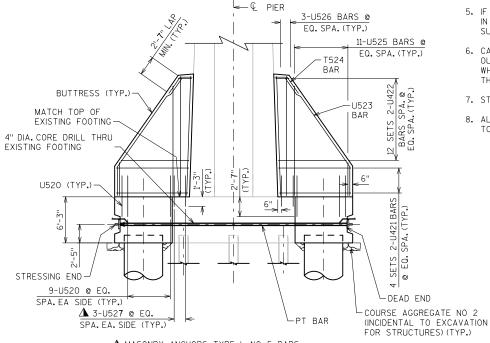
(CONTRACTOR TO DETERMINE

DIMENSIONS PER MANUFACTURER'S

ANCHOR PLATE (TYP.)

RECOMMENDATION)

8



⚠ MASONRY ANCHORS TYPE L NO. 5 BARS. EMBED 1'-3" IN CONCRETE.

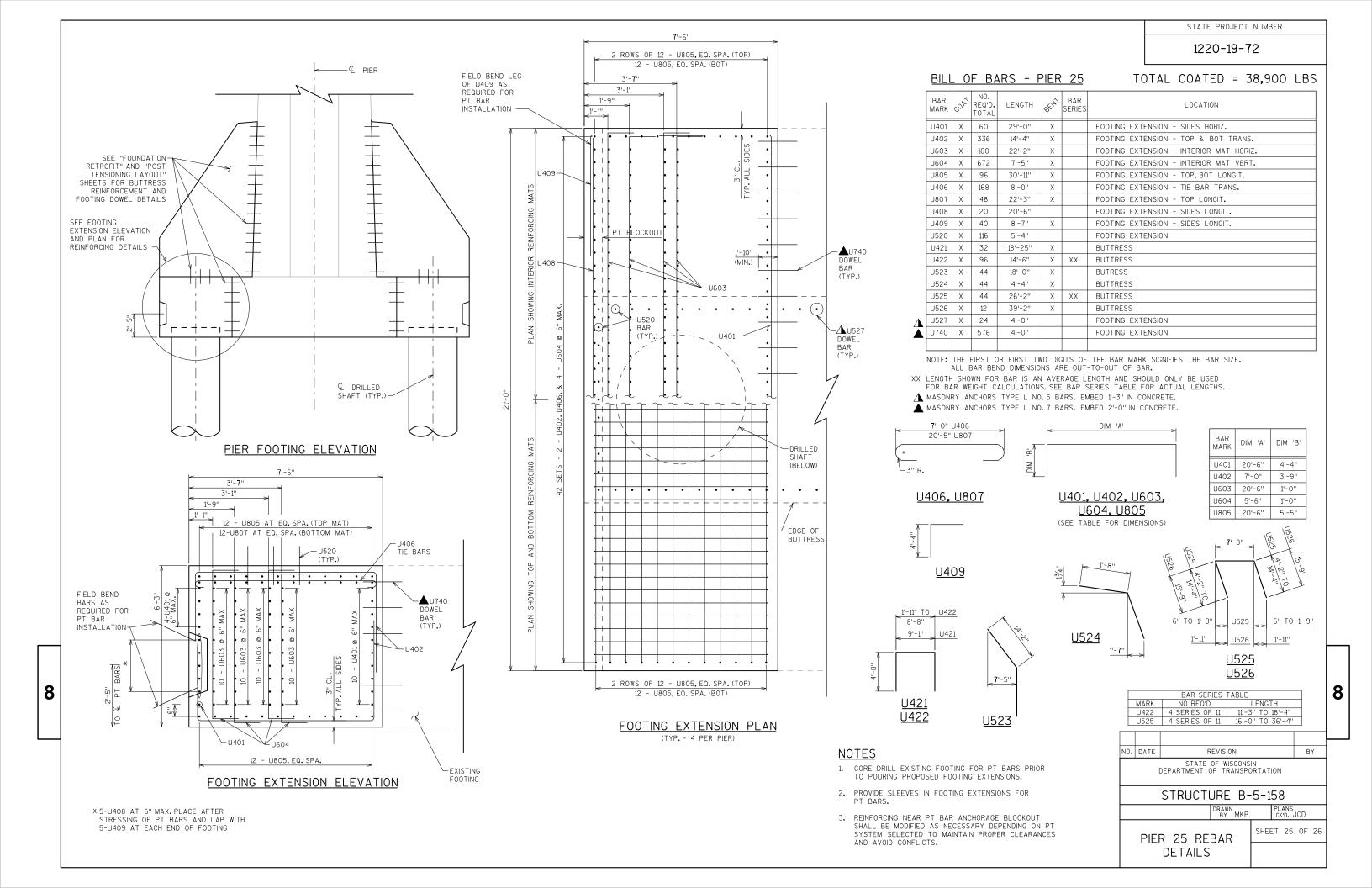
UPSTATION ->

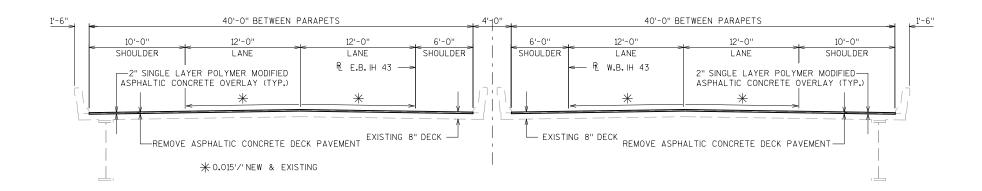
SECTION C-C

ALL DIMENSIONS SHOWN ARE TYPICAL UNLESS NOTED OTHERWISE. NO. DATE STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION STRUCTURE B-5-158 PLANS CK'D. JWB PIER 25 SHEET 24 OF 26

8

POST TENSIONING LAYOUT

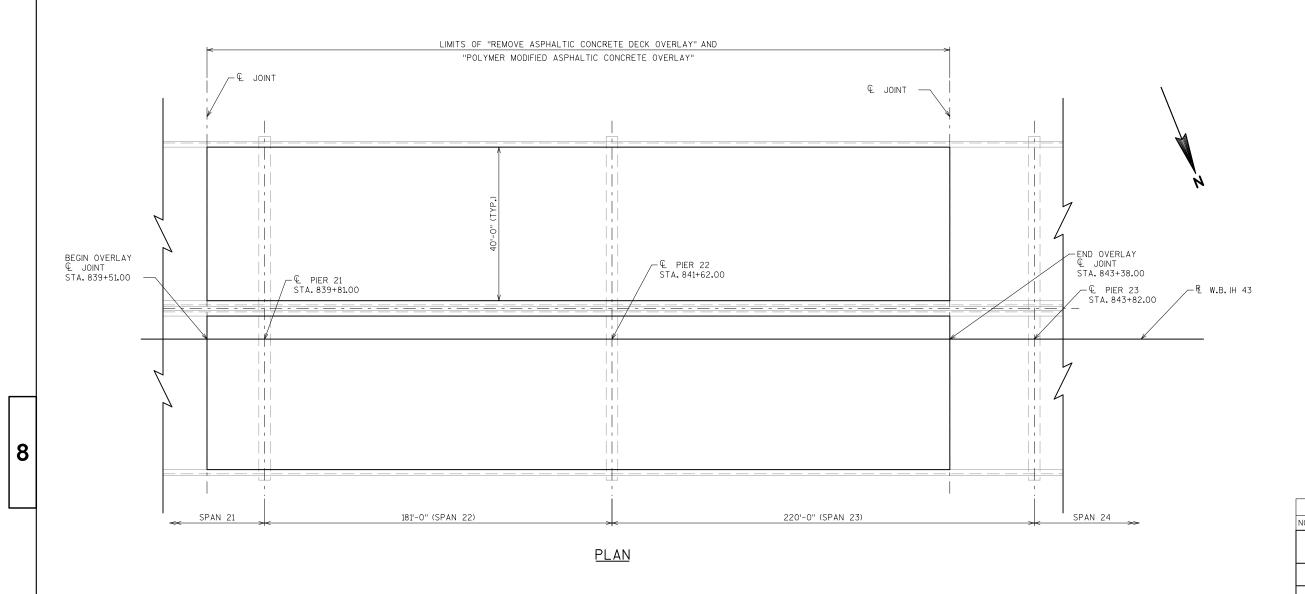




CROSS SECTION THRU ROADWAY LOOKING WEST

NOTE:

 MILLING AND ASPHALT OVERLAY WORK SHALL NOT PROCEED UNTIL WRITTEN AUTHORIZATION IS GIVEN BY THE DEPARTMENT.



NO. DATE REVISION BY

STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION

STRUCTURE B-5-158

DRAWN EEM PLANS MSC CKD. MSC CKD.

POLYMER MODIFIED SHEET 26 OF 26 ASPHALTIC
CONCRETE PAVEMENT

Notes



Wisconsin Department of Transportation

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