

June 15, 2020

Mr. Dan Weinkauf Integrated Grading & Excavating (IGE), Inc. 605 Grossman Drive Schofield, WI 54476

#### Re: Raymond Road Storm Sewer Backfill Evaluation and Identified Soil Improvement Areas

Dear Mr. Weinkauf:

After construction on Raymond Road in 2018, subsidence features occurred at some locations in the roadway. The Wisconsin Department of Transportation (WisDOT) requested that the general contractor, Integrity Grading and Excavating (IGE), address the subsidence features by excavating and recompacting the soils along the entire storm sewer alignment. IGE retained Barr Engineering Co. (Barr) to evaluate the soil conditions in the backfill generally above the pipe and identify locations that may warrant soil improvement to help ensure future consequential soil subsidence within the roadway does not occur. The site location is shown on Figure 1.

This letter summarizes our geotechnical investigation analyses and testing completed to date and identifies locations and depths of storm sewer pipe backfill improvement for soils located between the pipe spring line and the ground surface.

## 1.0 Introduction

#### 1.1 Purpose

In order to address WisDOT concerns regarding the pipe backfill and future subsidence features from developing in the backfill above the pipe, IGE requested Barr expand the investigation of the backfill to the entire 3,100-foot-long project alignment. The goal of the investigation is to provide a basis for identification of areas requiring backfill improvement to limit the potential for additional future consequential subsidence under the assumption that the joints in the pipe will be soil tight. Additionally, as a result of our investigation, we have identified areas along the pipe where soil improvement does not appear to be necessary.

#### 1.2 Scope of Work

This investigation included soil borings and select laboratory testing, geophysical methods including surface and seismic Cone Penetrometer Testing (CPT) methods, test pits, and CPT soundings. Barr's investigation included reviewing construction information, observing soil borings, test pits, and CPT, documenting nuclear and laboratory density testing conducted by others, and recording pore water pressures from vibrating wire piezometers.

Our analysis and subsequent recommendations are based on review of the following documents and reports:

- The report: Presentation of Site Investigation Results Raymond Road, prepared by ConeTec dated April 28, 2020, which is included as Attachment A.
- Soil borings observed by Barr in June and July 2019 and May 2020, which are included as boring logs in Attachment B.
- Test pits observed by Barr conducted in July 2019, and May 20, 2020 which are included as test pit logs in Attachment C.
- Laboratory testing performed by Cooper Laboratories and Soils & Engineering Services, Inc., which was conducted in May 2020 for which the test results are provided in Attachment D.
- Collier Geophysics Multi-Spectral Analysis of Shear Waves (MASW) Report, May 2020, report provided as Attachment E.
- Information gathered by or provided to Barr for the prior subsidence investigation including conversations with CNA Consulting Engineers, Brent Anderson & Associates, IGE, and others.

# 2.0 Discussion of Investigation Findings and Analyses

The primary means of evaluating the backfill was through analysis of CPT data. The CPT method consists of applying force to advance a probe rod that can be fitted with a variety of instruments that are designed to allow for near-continuous subsurface data collection and measurement of soil properties. The specific cone penetrometer for this project included a piezometer (also known as a piezocone) that measures pore water pressure to estimate water levels at the time of the soundings. The CPT equipment is housed within a large truck that uses hydraulic rams to push the soil probe into the subsurface. Other data collection methods using hollow stem auger drilling methods, geophysics, or collection of soil samples involve measurement of soil properties that are averaged over a vertical portion of the subsurface that is generally greater than 1.5 feet. The CPT method allows for greater precision and in-situ measurement of subsurface soil properties allowing for much finer detail than other methods.

The locations of the investigation soil borings, CPT soundings, and test pits are shown on Figures 2 through 4. The CPT probe advancement (referred to as "soundings") was pushed along the side and directly over the centerline of the pipe at intervals of approximately every 100 feet along the pipe alignment. Additional soundings were advanced in specific areas of interest such as previously observed subsidence features and additional soundings were advanced a few feet from where shallow refusal was encountered. The soundings that were advanced to depths below the pipe crown were positioned approximately 2 to 3 feet laterally beyond the edge of the pipe or were terminated approximately 1 to 2 feet above the pipe to prevent damage to the CPT equipment at storm sewer pipe. Due to local topography limiting safe rig access, the presence of the curb, or other constraints, the CPT soundings along the side of the pipe were generally advanced along the westerly/northwesterly side of the storm sewer.

## 2.1 CPT Tip Resistance (qt)

One type of measurement collected by the CPT method is cone resistance,  $q_c$ , and as cone tip resistance,  $q_t$ , which is cone resistance corrected for cone area ratio and pore pressure. The attached Table 1 provides

a visual summary of CPT  $q_t$  versus depth along the storm sewer alignment. Cone tip resistance is commonly used by geotechnical engineers to correlate soil strength and density. The recorded tip resistances were averaged for each 1-foot interval and were shaded from green to red to indicate relatively high (in green) to low cone resistances (in red). Table 1 was generated with the following notes:

- 1. CPT tip resistance is provided in values of tons per square-foot (tsf). Color shading is based on:
  - a. Values less than 30 tsf are shaded red.
  - b. Values between 30 and 60 tsf are shaded orange to yellow.
  - c. Values between 60 tsf and 100 tsf shade from yellow to green.
  - d. Values greater than 100 tsf are shaded dark green.
- 2. The tip resistance, q<sub>t</sub>, is averaged over 1-foot intervals. Actual tip resistance values were measured at an interval of 0.05 meters and were directly provided by ConeTec (Attachment A).
- 3. Depth is measured from the existing ground surface at time of CPT exploration; the ground surface consisted of graded base aggregate in the roadway and grass-covered soil outside the roadway.
- 4. The roadway station locations were provided based on survey performed by Burse Surveying and Engineering.
- 5. The CPT locations were primarily surveyed by Burse Engineering and Surveying with some locations being recorded by Barr.
- 6. CPT locations CPT57 and CPT57B are shown separately since these were performed outside the anticipated trench and backfill zone of the storm sewer excavation. These soundings were advanced to evaluate the ground condition in areas where the pipe installation was not expected to have disturbed the native soil and were conducted for comparative purposes.
- 7. CPT locations CPT58, CPT59, and CPT60 are shown separately since these soundings were performed in an area where the existing trench backfill was excavated and recompacted under Barr's April 20, 2020 observation. The purpose of these soundings was to correlate the observed backfill process and condition with density testing and subsequent CPT tip resistance.
- 8. Heavy black borders of table cells indicate the estimated/approximate top and bottom of the pipe based on information provided in the Design Drawings. As-built drawings were not available and a current-condition pipe invert survey has not been performed. The location of the heavy black line is anticipated to generally be within 1 foot of the design elevation of the pipe. The actual pipe elevations may vary.
- 9. Heavy blue border of table cells indicates the estimated groundwater level based on CPT piezocone data. The water levels of the previously and subsequently performed soil borings or vibrating wire piezometer data is not represented in this data set. Groundwater levels are expected to fluctuate seasonally and with precipitation.

## 2.2 Soil Behavior Type (SBT)

ConeTec provided the Normalized Soil Behavior Type based on  $Q_{tn}$  ( $Q_{tn}$  SBT) (Robertson, 2009) to classify the soil. SBT is a calculated parameter based on CPT data that is intended to predict soil type based on

mechanical properties recorded during the CPT soundings. The calculated parameter Q<sub>tn</sub> is the cone resistance that is normalized to account for the vertical stresses. Table 2 is color-coded visual summary of the SBT data collected along the storm sewer alignment and reported by ConeTec compared to depth.

The CPT parameter calculations are based on values of tip resistance  $(q_t)$  sleeve friction  $(f_s)$  and pore pressure  $(u_2)$ . Effective stresses are calculated based on unit weights that have been assigned to the individual soil behavior type zones and the assumed equilibrium pore pressure profile. Soils were classified as either drained or undrained based on the  $Q_{tn}$  SBT. Calculations for both drained and undrained parameters were included for materials that classified as silt mixtures (zone 4). Table 2 was generated with the following notes, and Table 1 notes 3, 4, 5, 8, and 9 also apply to Table 2 (not restated for brevity):

- 1. The SBT is averaged over 1-foot intervals. Actual SBT values were provided by ConeTec at 0.05meter intervals over the depth of the CPT sounding. (Attachment A).
- 2. The SBTs at CPT57 and CPT57B (considered the roadway section construction) are shown as primarily 5 to 7 with the underlying native soil being SBT 3 and some type 4. At greater depths below about 27 to 28 feet, SBTs of 5 to 7 are found.
- 3. The SBT at CPT58, CPT59, and CPT60, correlated with the SBT of the backfill soils in the other CPT soundings performed in known backfill.

### 2.3 Relative Density, Dr, from CPT Correlation

The attached Table 3 provides a visual summary along the storm sewer alignment of CPT-correlated relative density,  $D_r$ , with respect to depth with the relative density based on the CPT data. Relative density is commonly used in the geotechnical industry as an index to correlate and estimate soil strength and relative compressibility as it identifies the minimum and maximum void ratio for a given soil.

Based on this information and the observed means and methods to place and compact the backfill at the test trench at station 526+50 at a depth of 4 feet, we estimate that the maximum dry densities (maximum unit weight) as determined by the Standard Proctor test (ASTM D698) and Maximum Index Unit Weight test are within about 5%. A relative density value of 50% was selected as the threshold for improvement as it correlates to soil that is compacted to about 90% of the maximum dry density as determined by the Standard Proctor test. A minimum specified density of 90% of the maximum dry density based on the Standard Proctor test is commonly specified for backfill placed deep below roadways. For our correlation, we assigned a 0% relative density to the determined unit weight based on the Minimum Index Density and Unit Weight of Soils Test (ASTM 4254) and 100% relative density to the Maximum Index Density and Unit Weight of Soils Test (ASTM 4253). Cooper Laboratory determined minimum unit weights of 107.1 and 88.9 pounds per cubic foot (pcf) at test pit locations excavated at stations 526+50 and 518+46, respectively. Similarly, Cooper Laboratory determined maximum unit weights of 128.9 and 122.2 pcf at test pit locations excavated at stations 526+50 and 518+46, respectively. Relative density, Dr, correlates to minimum, maximum, and in-place unit weight based on the formula:

$$D_r = \frac{\gamma_{dmax}}{\gamma_d} \left[ \frac{\gamma_d - \gamma_{dmin}}{\gamma_{dmax} - \gamma_{dmin}} \right]$$

where  $\gamma_d$  = dry unit weight from nuclear density testing or proctor density testing  $\gamma_{dmin}$  = minimum dry unit weight from ASTM 4254

#### $\gamma_{dmax}$ = maximum dry unit weight from ASTM 4253

Therefore, we computed unit weights of 118.0 and 105.6 pcf at test location at stations 526+50 and 518+46, respectively, that correspond to 50% relative density. We then divided the 50% relative density unit weights by the maximum unit weight determined by the appropriate Standard Proctor test, which resulted in an average of 91.5% (or about 90%) of the maximum dry density determined by the Standard Proctor test. The table below provides a summary of the relationship between laboratory and insitu densities for sampling at a depth of 4 feet at the location of the observed test pits.

	Min. Unit	Maximum Unit	Unit	Maximum Unit	
	Weight	Weight	Weight at	Weight	
	(ASTM 4254)	(ASTM 4253)	50%	(ASTM D698)	
Test Location	0% Relative	100% Relative	Relative	Standard	% of Maximum dry
Approximate	Density	Density	Density	Proctor	density
Station	(pcf)	(pcf)	(pcf)	(pcf)	(Standard Proctor)
526+50	107.1	128.9	118.0	123.6	95.5
518+46	88.9	122.2	105.6	120.5	87.6
				Average:	91.5

Table 3 is color-coded with only primarily granular soil behavior types (6 and greater) on a green to yellow to red-shaded scale to indicate relatively high to low correlated relative density. The table approximately provides where loose subgrade (red color range) exist that may require mitigation. Typically, red colors represent relative densities less than 40%, orange 40% to 50%, yellow from 50% to 60%, and green becoming darker from 60% and greater. Table 3 was generated with the following notes, and Table 1 notes 3, 4, 5, 8, and 9 also apply to Table 3 (again not restated for brevity):

- 1. The provided calculation of relative density from the CPT data is based on Baldi et al (1986) applied to SBTs of 6 or greater. Limitation of the correlation is to granular soils, specifically cleaner granular soils with small percentages of soil that pass the #40 and #200 sieve (soil types 6 and 7).
- 2. Relative density estimates may be influenced by artificially "high" cone resistances due to the presence of gravel, cobbles, and/or boulders and therefore not be locally representative of the relative density of the subgrade. Any value calculated as greater than 90% should be considered quite dense. Instead of reporting the correlated relative density value for any value greater than 90%, the cell is provided as a dark green shading.

As previously indicated, the CPT tip resistance can correlate to relative density using empirical formulas. To better define the correlation at this site, and aid in estimating relative density using other test methods, e.g., sand cone and Standard Proctor tests, Cooper Laboratories performed Minimum and Maximum Index Unit weight test on select bulk samples from the test trenches.

### 2.4 SPT N<sub>60</sub> Value Correlation from CPT Data

The attached Table 4 provides a visual summary along the storm sewer alignment (based on the CPT locations) of CPT-correlated Standard Penetration Test (SPT)  $(N_1)_{60}$ -value, with respect to depth and corrected for overburden pressure. The correlated N-values were provided by ConeTec and SPT N-values are commonly used data for estimating soil strength and density. Table 4 is color-coded on a green to yellow to red-shaded scale to indicate low correlated N-values of 4 or less which are shaded red color

range, N values of 5 to 7 are orange, 8 through 10 are yellow, and 11 and greater are light green. Values greater than 11 range from light green to dark green.

The N-values from SPT sampling generally aligned well with the correlated  $(N_1)_{60}$ -values from the CPT soundings for  $(N_1)_{60}$ -values for values below approximately 30. Above an  $(N_1)_{60}$ -value of 30, the soils are considered dense and the correlation appears to be heavily influenced by the presence of cobbles or gravel in the backfill soil. Note that the N-values reported on the logs are field recorded values and not corrected for overburden or sampling methods. The N-value correlations are provided for reference and were not utilized for identification of soil improvement areas.

Table 4 was generated based on Table 1 notes 3, 4, 5, 8, and 9 (again not restated for brevity) and includes the following notes:

- 1. The (N<sub>1</sub>)<sub>60</sub>-value provided is corrected for overburden stress and is based on the correlation by Lunne, Robertson, and Powell (1997).
- 2. Relative density estimates may be influenced by artificially "high" cone resistances due to the presence of gravel, cobbles, and/or boulders and therefore not be locally representative of the relative density of the subgrade. Any N-value calculated as greater than 30 should be considered dense. Instead of reporting the correlated N- value for any value greater than 30, the cell is provided as a dark green shading.
- 3. CPT locations CPT57 and CPT57B are shown separately since these were performed outside the anticipated trench and backfill zone of the storm sewer excavation. These soundings were advanced to evaluate the ground condition in areas where the pipe installation was not expected to have disturbed the native soil and were conducted for comparative purposes. The N-values in this area is similar to N-values observed below the expected pipe excavation zone from approximately station 524+00 to 525+75.

## 2.5 Comparison of MASW Results to CPT Soil Data

Geophysical methods used in the investigation included a MASW survey conducted by Collier Geophysics (Attachment E). This type of seismic geophysical method allowed collection of continuous subsurface data along three transects oriented parallel to the pipe alignment. Details on the method and the data collection are included in the Collier report found in Attachment E.

Shear wave velocities collected with a shear wave generation equipment attached to the CPT rig were compared to the MASW shear wave velocities. Generally, good agreement was found between the two data sources given the difference in scale between the CPT point measurements and geophysics which is a form of bulk subsurface measurement. Limitations for precise agreement between the different measurement methods include the subsurface resolution, different lateral locations, and the heterogeneity of soils observed within the backfill. Each consideration is discussed briefly below.

The MASW subsurface resolution is between 1-4 feet at the near surface, and increases with depth, in contrast to the CPT soil measurements which were made every 0.05 meters. CPT seismic readings were made approximately every 1-2 meters. As such, CPT soil observations that showed differences in small intervals were not observed in the MASW survey. Similarly, but to a lesser degree, the CPT seismic soundings interval remained at approximately 1-2 meters throughout the soil column, while the MASW

intervals increased with depth. However, significantly low shear wave velocities, representative of weak soils, are observed in both sets of data in similar locations.

The MASW survey transects were located immediately over the storm sewer alignment, and one each located approximately 15-feet on either side of the pipe centerline. The CPT soundings were generally 8-9 feet from the storm sewer centerline, which is between the two MASW survey transects. This requires interpolation of two MASW transect data sets for a single comparison to the CPT data set.

Soil heterogeneity that naturally occurs during backfilling of construction projects is observed at a single lateral location in CPT soundings, but is averaged along the MASW transect data interpretation. Similar to the depth resolution, this difference in lateral spatial observation limits direct comparison of CPT data to MASW data.

Notwithstanding the above limitations, our review supports the generalized characterization that relatively low (red), medium (yellow-orange), and high (green) velocity shear waves measured by MASW sufficiently correlate to respective low, medium, and high velocity shear waves as measured by CPT.

### 2.6 Comparison of SPT Observations to CPT Soil Data

The comparison of SPT observations, recovered soil samples, and N-values to CPT soil behavior is limited by the soil heterogeneity and the differences in soils observed in undisturbed locations off the roadway and the disturbed soils within the roadway. SPT N-values and observed recovered soil samples, generally correlate well to the observed CPT soil behavior types, as noted in Section 2.4. Generally, soil observations, and select laboratory test results, from SPT borings confirm the soil backfill exhibits significant heterogeneity in some areas, which agrees with the heterogeneity observed in CPT soundings.

## 2.7 Comparison of Test Trench Observations to CPT Soil Data

Test trenches were performed to observe a soil profile as well as to measure in situ soil densities and collect samples for our evaluation, which were explicitly considered in conjunction with the CPT data for development of the identified soil improvement areas further discussed.

### 2.8 Combined Comparison of Data Sources

Overall, the observations and results from CPT soundings correlate relatively well with the observations and results from the MASW survey, SPT borings, lab testing, and test trench observations.

Geologic cross sections developed with consideration of the CPT soundings, SPT borings, and test pits are included as Figures 5 through 11.

### 2.9 Groundwater Elevations

Vibrating wire (VW) piezometers were previously installed in three nested locations, as summarized in the August 1, 2019 letter. Field activities performed in May 2020 included installation of four additional VW piezometers at the following SPT boring locations and corresponding depths: SB20-03 (30'), SB20-05 (30'), and SB20-07 (38'). The VW piezometer SB20-07 was installed immediately adjacent to the previously installed B3C VW piezometer nest. The locations of the VW piezometers are illustrated on Figures 2 through 4.

The measured potentiometric surface from May 18 – June 10, 2020 for each of the three nested vibrating wire piezometers are shown on Figures 12, 13, and 14. As observed in these hydrographs, the VW piezometers at higher elevations have higher pore water pressures than the lower elevation VW piezometers, which indicated a downward vertical gradient over the measured time period.

Comparison of VW piezometer potentiometric surface elevations to pore water pressure observations in CPT soundings indicate general agreement. Specifically, the B2B and B1B elevations compare favorably to nearby CPT soundings CPT20-11, CPT20-48, CPT20-13; and B3C elevations compare favorably to nearby CPT sounding CPT20-20 and CPT20-17.

# 3.0 Identification of Pipe Backfill Improvement Areas

Based on the review of the information presented in this letter, consideration of information from a variety of subgrade parameters from several investigation sources was used to aid in the delineation of the identified improvement areas.

For our analysis, we relied on the provided subgrade strength  $(q_t)$ , soil behavior type (SBT), and relative density  $(D_t)$ , with consideration of the relativeness thickness and proximity of subgrade conditions to the pavement layers and storm sewer pipe.

In our opinion, using a single parameter likely results in an overly conservative or insufficient limit to the areas which need to be addressed. As a result, the identified areas for the soil backfill improvement are based on soil resistance as derived from CPT tip resistance, soil behavior type and relative density as correlated from CPT testing, laboratory testing, observed backfill processes in the test excavations, nuclear density testing of the backfill, and the thickness and location of the loose soils.

The attached Table 5 provides a visual summary of the areas identified for soil improvement by the criteria along the storm sewer alignment with the identified depths described in text format in Table 6.

## 3.1 Overview of Findings

Table 6 summarizes the individual sections along the alignment where conditions meet the aboveidentified criteria for soil improvement. Consistent with Table 5, we identified areas where soil improvements are not warranted. Also consistent with Table 5, we identified areas that could benefit from soil improvement.

## 3.2 Backfill Improvement in Paved Areas

Our identification of areas for potential soil improvement to mitigate risk of consequential future soil subsidence are based on a tiered criterion. The evaluation is performed for the soil backfill located from the pipe spring line to the ground surface and do not include the soils beneath the pipe. The identified areas are provided with the understanding that pipe joint remediation is occurring that will inhibit soil migration into the pipe and that soil immediately below the pipe is improved in the process of the joint improvement.

Zone	Station Start	Station Stop	Approximate Length	Soil Improvement Area and Depth
1	Start	502+40	225	Area over pipe not within roadway. Excavate and recompact upper 10 feet of backfill under observation to WisDOT requirements.
2	502+40	504+50	110	Soil improvement from spring line to approximately 3 to 9 feet over the top of the pipe.
3	504+50	506+50	100	No improvement need identified.
4	506+50	507+50	100	Soil improvement from top of pipe to approximately 9 feet over pipe
5	507+50	519+75	1225	No improvement need identified.
6	519+75	520+75	100	Soil improvement from spring line to up to 7 feet over the top of the pipe.
7	520+75	523+25	250	No improvement need identified.
8	523+25	524+00	75	Soil improvement from the spring line to approximately 3 feet over the top of the pipe.
9	524+00	525+75	175	Unanticipated geologic feature. Soil imporovement should be completed to address subgrade conditions within this specific zone.
10	525+75	527+00	125	Soil improvement from the spring line to approximately 5 feet over the top of the pipe.
11	527+00	528+00	100	No improvement need identified.
12	528+00	528+75	75	Soil improvement from spring line to up 3 to 9 feet over the top of the pipe.
13	528+75	530+00	125	No improvement need identified.
14	530+00	531+25	125	Soil improvement from spring line to approximately feet over the top of the pipe.
15	531+25	End	250	Area over pipe not within roadway. Excavate and recompact upper 10 feet of backfill under observation to Wisconsin DOT requirements.

#### Table 6 Identified Soil Backfill Improvement Areas

Soils exhibiting response indicative of improvement must exhibit low tip resistance and low relative density and be present in a layer or collective layers of at least 3 feet or more. Areas targeted for improvement below the roadway are to meet the criteria of:

- 1. Exhibit CPT tip resistance, qt, of less than 40 tsf and consist of soil type 5 or less or,
- 2. Exhibit CPT tip resistance, q<sub>t</sub>, of less than 40 tsf and consist of SBT type 6 or greater and exhibit correlated relative densities less than 50% which indicate the soil may be qualitatively classified as loose or the lower range of medium dense. These threshold values are based on density testing of a test area of the backfill as well as advancing cone penetration test soundings through these areas, and the relevant laboratory and field testing described in Section 2.3. A relative density of

50% was selected as this generally corresponds to a standard proctor density of approximately 90%,

- 3. and be at least 3 contiguous feet or more than 4 collective feet in the evaluation depth if multiple layers of less than 3 feet are present in the vertical profile,
- 4. and be present at elevations at or above the pipe spring line.

If the criteria identified in items 1 or 2 and 3 and 4 above were not met, the area is not currently identified for ground improvement to mitigate future subsidence potential.

### 3.3 Backfill Improvement in Unpaved Areas

Given the location of the pipe alignment outside of the roadway occurring in green space, no soil improvement is immediately required as subsidence would not be consequential to the pavement and any subsidence would likely appear as a shallow depression. As shown in Table 5 and recommended in Table 6, unpaved areas (greenspace), such as southeast of approximate station 503+20 and northeast of station 530+50, have areas where loose soils were found to exist above the pipe. Due to the shallow nature of the pipe, these soils could be addresses by traditional excavation and backfill.

For these areas of greenspace outside of the roadway, a potential method to address these soils may include excavation and observed recompaction of the soils within 10 feet of the surface. The potential for subsidence in the areas where the pipe is in green space is limited by the fact that loads are not anticipated over the pipe, the pipe backfill is relatively shallow, and the joints will be sealed. The excavated soils should be excavated, the exposed subgrade evacuated, and the soils replaced and recompacted to Wisconsin DOT specification.

We assume that when soil improvement occurs, an experienced engineer familiar with the pipe constraints and selected soil improvement technique should be present while work is occurring to observe the improvement process and determine if the soil improvement may be suspended. The extent of the soil improvement should occur based on effectiveness and response of the soil within the recommended station interval but may be extended or shortened based on the observed soil improvement response. Upon starting the work, conditions may be encountered that would indicate a need for modifications to the identified soil improvement areas.

Table 6 provides a summary of the areas identified for soil improvement, as well as areas for which soil improvement needs were not observed. Table 5 provides a graphical display of these areas in the similar format as Table 2 through Table 4.

#### 3.4 Station 524+00 to 525+75 Anomaly

Through the course of the most recent investigation, a previously unidentified subsurface condition was encountered from approximately station 524+00 to 525+75. Generally, this station range correlates to the location of previous subsidence features. Considerably weak soils were identified around and at elevations below the pipe to appreciable depth. One design soil boring was conducted by CGC (Attachment B) that identified a single soil sample of clay with low strength.

Soil improvement within the backfill in this area is necessary. However, this letter does not provide soil improvement recommendations between stations 524+00 to 525+75. Geotechnical conditions encountered indicate that soil improvement below the pipe spring line will likely be necessary in this area.

It is our belief is that soil improvement of the remainder of the pipe alignment outside of stations 524+00 to 525+75 should occur while soil improvement requirements for this specific area are determined.

# 4.0 Limitations

The initial scope of our review, analysis, and provided areas of improvement included only the soils present from the pipe spring line to the surface. The investigation encountered geotechnical conditions that indicate loose soil may be present beneath the pipe spring line and to depths well below the pipe invert in some locations. The identified soil improvement areas assume that the joint improvement addresses the pipe conditions found below the pipe spring line and effectively develops soil-tight joint conditions.

The physical investigation utilizing SPT borings and CPT soundings cannot evaluate soils immediately adjacent to the pipe without potential for contacting and damaging the pipe. Therefore, the conditions encountered were observed as practically close to the pipe as reasonable; within a few feet. This approach is considered as an appropriate and acceptable practice within the industry to gather necessary subsurface information. Although the conditions immediately surrounding and beneath the pipe may differ from what is observed in these investigation techniques, as the soil in these locations could not be directly assessed, the information gathered is considered to be sufficient for the purposes of identifying areas needing soil improvement.

The soil improvement is recommended to reasonably limit the potential for development of consequential subsidence features above the pipe alignment and the recommendations are based on observed investigation data. Methods of soil improvement are based on the encountered soil type, location of the improvement area relative to the pipe, pipe loading limitations, depth and thickness of treatment zone, access and space limitations, and schedule. This letter does not include recommendations regarding pipe joint repairs, improvement of soil below the pipe spring line, or pipe subgrade remediation.

Barr Engineering Company's services for this project were performed in a manner consistent with that level of care and skill ordinarily exercised by members of the profession currently practicing in this area. No warranty, expressed or implied, is made.

Please contact us regarding any questions you may have. We appreciate the opportunity to assist you with this project.

Sincerely,

Travis A. Davidsavor, PE Wisconsin Professional Engineer #E-39112-6 Vice President, Sr. Geotechnical Engineer



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Enclosures Tables Figures Attachments

#### References

Robertson, P.K., 2009, "Interpretation of cone penetration tests – a unified approach", Canadian Geotechnical Journal, Volume 46: 1337-1355.

Lunne, T., Robertson, P.K. and Powell, J.J.M., 1997, "Cone Penetration Testing in Geotechnical Practice," Blackie Academic and Professional.

Baldi, G., Bellotti, VN., Ghionna, N., Jamiolkowski, M., and Pasqualini, E., 1986, "Interpretation of CPT's and CPTU's," Proceedings of the 4th International Geotechnical Seminar Field Instrumentation and in-situ Measurements, Nanyang Technological Institute, Singapore, 25–27 November 1986.

Tables

Table 1 Summary of CPT Q<sub>t</sub> Values

Zone	1		2			2	4							F								6		7				٥	1		11		12	12		14	15	Off	nt Backfill CPT
Zone	1	2	3 4	6 7	8	9	10	11	12	13	14 15	16	17	18	19	20 21	22	23	24 2	25 26	27 #	29 30	# 22	34	37	20	40 41	# # #	40 50			62 66	# 69	13	72	73	74 75	Alignmer	
Name/ Average Depth (ft)	CPT20-45	CPT20-44	SCPT20-46 4	CPT20-438 9 CPT20-42 ^	SCPT20-41 ×	CPT20-40	CPT20-39	CPT20-38	SCPT20-37	CPT20-36	CPT20-34 ht	CPT20-33	CPT20-32	SCPT20-31	CPT20-29	CPT20-30	CPT20-27	26	SCPT20-53	CP120-54 07 CPT20-25 07	CPT20-55 7 CPT20-23 #	CPT20-24 6	CPT20-22 #	CPT20-01B	SCPT20-02C	CPT20-03B	CPT20-20 &	CPT20-19 # SCPT20-05 # EP120-06 #	CPT20-16 &	CPT20-08 4	CPT20-14C &	СРТ20-09С 8 СРТ20-51С 8	CPT20-61 # SCPT20-50 & CPT20-62 &	CPT20-10	SCPT20-11	CPT20-48	CPT20-13 1	CPT20-57 SCPT20-57B	CPT20-5 SCPT20-
Station 0.5	500+88	503+01	503+51	00++00 00++00 00++01 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00 00++00	505+01	66+505	506+99	10+805 263	508+97	510+00	510+98	511+99	<sup>512+90</sup>	513+99	514+96	514+97	516+81	516+81	518+00	518+48	519+52	520+51	520+91 520+93	521+98	522+93	523+78	524+16	524+74 524+99 <u>525+1</u> 4 525+22	525+58	526+82	527+42	527+82	528+46 528+50 528+55	529+08	529+77	244	531+39	222 55+022 525+022	226+55 8526+55 8526+59
1.5 2.5 3.5	59 58 19	111 217 282 127	11         125           263         330           326         245           224         255	173 368	280 241 180	409 349 227	254 366 330 304	302 314 312	359 429 482	425 494 482	235 139 331 288 493 309 632 557	386 383 313	269 341 270	406 398 447	341 435 358	104         157           386         430           326         364           333         240	306 464 526	572 592 444	289         1           379         2           431         3           383         4	198         348           198         348           153         371           142         454	380 ## 260 ##	184         296           #         356         408           #         382         411           #         503         502	207 205 293 435 387 379 540 495	256 428 543	289 367 521	126 96 324	256 265 # 301 313 #	###         145         215           ###         288         314           ###         300         343           ###         339         319	172         228           310         297           271         141           336         278	200 251 211 440 265 312	252 339 254 555 241 260	150         221           322         335           645         220           490         177	195         #         175           269         #         306         1           243         #         376         3           300         #         359         3	87         191           193         293           311         288           222         350	207 265 408 253	267	56         28           92         103           63         100           22         58	358 36 207 22	7         197         166         135           4         191         159         132
4.5 5.5 6.5	82 30 22	89 106 125	216 115 136 109 81 154	83         144           129         292           186         281	145 172 158	152 225 213	208 244 301	256 375 347	298 246 272	383 387 313	508         427           241         334           247         283	321 293 276	109 92 126	251 161 158	198 233 187	164         227           80         190           85         149	374 231 177	292 171 135	227 2 149 1 86	134         258           157         190           82         161	347 ## 256 ## 219 ##	353         362           #         358         281           #         250         227           #         114         194	376 317 207 224 126 150	292 208 167	265 209 166	294 262 77	130         143         #           58         74         #           39         33         #	###         217         225           87         122         176           78         75         112	210 151 104 88 48 60	132 143 61 146 43 118	191         112           166         107           218         171	395         118           286         52           193         28	259         #         259         #         284         1           172         #         236         1           128         #         143	149         155           122         87           89         75		182 121 81	5 54 7 53 17 53	15 17	297         267         281           297         267         281           278         244         211           148         141         126
7.5 8.5 9.5	13 11 10	95 123 105	62 133 137 93 214 115		143 205 89	179 143 116	287 287 201	283 279 221	183 229 227	347 321 259	212 339 209 229 178 207	266 310 261	109 91 75	144 124 126	148 151 156	238 152 226 124 180 62	172 176 202	137 101 141	77 83 65 7	61 153 81 110 73 98	202 61 157 54 124 ##	64 155 37 128 29 171	83 122 177 133 104 130	143 107 91	114 87 59	40 35 27	32         39         3           36         65         3           39         60         4	77         79         92           54         76         99           49         60         108	61 33 63 33 51 33	42 142 38 107 30 63	163 219 55 255 84 154	244 47 151 33 187 40	102 # 67 81 # 79 90 # 106	84 65 81 50 72 40	71	78 63 43	25 45 27 51 30 88	14 12 7.7 9	188         167         130           119         131         113
10.5 11.5 12.5 13.5	27 15 7	36 21 27	189         169           121         157           127         116	182 196 168 111		168 161 175	215 339 285	136 135 73	187 211 193	266 253 180	163         235           138         256           136         212	174 109 121	94 92 99	163 107 108	165 126 108	151         118           122         135           83         113	196 184 187	145 162 158	65 1	86 62 104 21 92 29	86 96 69 77 70 67	56 190 56 137 91 94	41 78 69 55 59 39	125 106 125	61 68 95	29 68	74         24           71         10	28     50     -     91       8     80     -     60       3     62     -     35	18 72	46 56 48 75 59 35	65         85           54         66           60         114	88         47           57         83           77         62	54         #         128           80         #         50         1           47         #         32         2	83         34           120         46           226         109	51 50 48	28 15 12	26         105           21         93           24         57	5.1 7	67         90         78           91         43         64           330         97         181           -         -         -
13.5 14.5 15.5 16.5	7 8 8	51 60 41	115 130 104 117 90 95		284	161 184 250	240 134 233	121 166 187	150 128 110	199 143 133	154 168 66 120 37 219	103 79 63	83 93 59	70 58 48	99 94 113	111 87 134 67 69	194	87 110 60	66 1 74 77	102         21           94         42           76         74	70 44 83 - 82 -	88 86 73 43 40 34	57 31 86 - 91 -	125 159 68	92 53 39	128 168 410	44 4 8 5 10 7	3         62         -         35           3         44         -         39           6         48         -         41           5         27         -         15	10 55	35 27 40 57 104 95	85 - 113 - 282 -	- 34 - 35 - 50	28         #         81         1           34         #         53         1           53         #         42	127 110 134 88 98 111	62 57 37	24	26 134 35 139 35 112	5.1 6 7.7 9 7.7 1	·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·         ·
16.5 17.5 18.5 19.5	7 14 115 223	29 33 33	72 76 51 63 36 57	- 63 - 36 - 25	162 142 80 142	287 261 217 225	196 181 182 138	163 190 169	91 110 107	116 103 82	73 - 137 - 122 -	47 40 31	53 41 53	51 45 66	96 104 108	- 54 - 46 - 54		67 59 41	87 94 115	49 58 40 41 57 65	86 - 103 - 149 -	24 20 21 26 26 20	101 - 74 - 59 -	74 87 84	85 65 55	541 - -	11 8 9 8 8 8	7 10 - 7 8 9 - 8 8 8 - 8	11         67           102         89           242         195		505 - 605 - 729 -	- 35 - 84 - 8		128 77 183 70 808 123	49 83 110	20	47 69 166 79 286 79 248 71	8 10 8.3 9 8.7 5	
20.5 21.5 22.5	142 62 123	44 40 62 57	24 87 30 67 29 46	- 21 - 21 - 33 - 39	129 88	190	236 153 105	156 179 181	52 51 81	167 160 119	47 - 91 -	31 59 62	83 110 84 139	44 37 42 58	71 62 82	- 80 - 84 - 94		105 179 165 171	144 151 1 165 1 157 0	28 111 24 119 94 226	216 - 306 - 234 -	106 27 168 39 196 39 202 179	112 - 107 - 94 -	93 140 320 474	24 20 21	-	9 7 8 8 10 8 16 9		67         134           92         151           368         165           355         284		· · ·	- 84 - 57 - 137	45 # 32 3 91 1 62 5 58 1 41	193           303         299           525         336           -         184	88 124 59	415 671 585	248         71           330         49           272         80           243         121	7.1 8 7.3 8	
23.5 24.5 25.5	52 92 126	56 63 118	29 34 29 25 33 22	- 65 - 66 - 56	59 80 71	163 141 172	67 37 28	171 148 151	57 61 62	422 578	121 - 154 - 150 -	22 25 41	157 161 101	115 81 169	128 150 193	- 44 - 44 - 90		194 188 196	148 1 134 1 180 1	26 320 28 342 38 235	211 - 248 - 374 -	197 188 194 134 161 130	161 - 167 - 124 -	-	22 23 31	-	14 9 17 43 57 120 #	9 8 - 18 15 8 - 121	294 528 327 571 165 -	490 118 - 126 - 318	 	- 259 - 334 - 195	127 # 39 143 # 22 122 # 55	- 147 - 267 - 367	50 47 42	300 273 195	499         108           486         80           225         271	8.3 9 8.7 9 18 1	
26.5 27.5 28.5	197 183 182	142 144 129	55         22           91         29           109         47	- 54 - 82 - 102	69 55	163 128 103	22 34 58	158 134 100	58 53 75	-	106 - 137 - 339 -	57 602	92 145 154	107 103 162	234 268 122	- 151 - 100 - 60	· ·	217 227 211	186 1 178 1 223 9	168 223 153 216 92 191	278 - 357 - 381 -	218         137           225         128           216         140	128 - 128 - 130 -	-	40 44 72	-	57         120         #           35         200         #           6         174         #           10         223         #	8         - 332           ###         8         - 339           ###         58         - 364	204 - 172 - 45 -	- 191 - 108 - 111	 	- 226 - 168 - 285	222 # 90 248 # 154 169 # 206	- 490 - 568 - 301	61 360 440	526 855 640	371         179           322         94           350         246	13         10           96         11           117         12           80         15	2 2
29.5 30.5 31.5	209 235 227	159 165 166	100         68           102         102           99         172	- 108 - 115 - 130	104	149 168 156	36 26 34	101 144 287	96 133 124			-	168 84 89	200 69 250	45 33 41	- 55 - 107 - 81	· · ·	215 242 231	227         3           222         3           217         3	117         159           138         146           103         135	310 - 254 - 329 -	292 137 389 151 - 177	121 - 124 - 138 -	-	72 88 90		92         166           122         131           115         19	87 82 417 56 254 516 ### - 492	43 - 40 - 121 -	- 109 - 115 - 166	 	- 338 - 350 - 434	161 # 254 168 # 230 - # 229	- 307 - 412 	481 675 801	822 949	305         470           293         336           274         313	80 15 137 34 250 64 274 11	-         -         -           4         -         -         -           4         -         -         -
32.5 33.5 34.5 35.5	-	156	135 157 212 134 	- 149 - 173 - 146	328 424 479	108 84	149 235	473	129 262	-	 	-	-	362 - -	35 84 164	- 52 - 40 		-	-	- 142 - 166 - 144		 	142 - 175 - 174 -	-	99 112 113 110	-	- 5 - 4 - 14	79 604 19 532 32 580	177 - 370 - 256 -	- 192 - 180 - 230	 	 	- # - - # - 	 	-	-	255 329 270 504 327 476	364 27	8 5 2
36.5 37.5	-	-		 					-	-		-	-	-	-			-	-			· ·	171 - 149 - 203 - 379 -	-	110 179 221 193	-	- 459	19         -         -         675           11         -         -         -           52         -         -         -	136 - 171 - 195 -	- 214 - 247 - 581	· ·	 	· · ·	 	-	-	366 393 321 511 260 695 274 -	- 33 - 41 - 36 - 49 - 66 - 62	6
38.5 39.5 40.5 41.5	-								-	-		-	-		-			-	-				432 - 189 - 160 -	-	258 205	-			366 - 583 -	· ·	 			 	-	-	201 - 161 -	- 62	3 6 9
42.5 43.5 44.5 45.5	-				-			-	-			-	-		-			-	-				147 - 149 - 135 -	- -	-	-		**** ****	 	 	 	 	· · ·	 	-	-	 	- 47 - 51 - 72 - 73 - 73	5         .         .         .           3         .         .         .           4         .         .         .           2         .         .         .           9         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .           .         .         .         .
46.5 47.5	- - -	-	· · ·	 	-		-	-	-	-		-	-	-	-			-	-	 		 	134 - 130 - 122 -	-	-	-		**** - • • • •	· ·	· ·	 	 		 	-	-	· ·		
48.5 49.5 50.5	-	-		 	-		-	-	-		 	-	-	-	-	 		-	-			 	193 - 176 - 175 -		-	-	 	 	 	· ·	 	 		 	-	-	 		

Table 2 Soil Behavior Type (SBT) Visual Data Summary

Zone	1		2			3		4								5									6			7		8	9	)		10	11		12		13	14		15	Off Alignment
Reference	1	2	3	4	7 8		9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24 2	5 26	5 27 #	# 29 3	30 #	32	34	37	39 4	0 41 #	# #	49 5	0 54	55 58 60	63 66	# #	69 70	71 72	73	74	75	44 46
Name/ Average Depth (ft)	8 CPT20-45	LCPT20-44	LSCPT20-46	/ CPT20-47 5 CPT20-43	LCPT20-42 LCPT20-42		) CPT20-40	) CPT20-39	LCPT20-38	SCPT20-37	)CPT20-36	3 CPT20-34	8 CPT20-35	CPT20-33	CPT20-32	SCPT20-31	5 CPT20-29	/ CPT20-30	CPT20-28	LCPT20-27	LSCPT20-26	SCPT20-53	CPT20-25	CPT20-55	CPT20-23	L CPT20-56	1 CPT20-21	8 CPT20-01B	SCPT20-02C	CPT20-03B	SCPT20-04	SCPT20-05 EP120-15 CPT20-06	3 CPT20-16	CPT20-08	CPT20-52 CPT20-14C CPT20-15B	CPT20-09C	5 5 5 5 5 5 0 5 0 5 0 5 0 5 0 5 0 5 0 5	CPT20-62 8 CPT20-10	CPT20-49	5 CPT20-48	CPT20-13	5 CPT20-12	Control         Contro <thcontrol< th=""> <thcontrol< th=""> <thco< td=""></thco<></thcontrol<></thcontrol<>
Station Burse	500+85	503+01	503+51	503+77 503+95	504+01 505+01		505+95	506+96	508+01	508+97	510+00	510+98	510+98	511+99	512+90	513+99	514+96	514+97	516+00	5	516+81	518+00	518+95	519+52	519+86	520+51	520+93	521+98	522+93	523+78	524+74 524+74	524+99 525+22 525+22	525+58	526+82	527+09 527+42 527+30	527+82 528+33	528+46 528+50	528+55 529+08	529+35 529+77	530+46	531+39	531+96	524+81 525+02
0.5 1.5	4	6	6	6 6	6 6	_	7	6	6	6	6	6	7	6	6	6	6	4	6	7	7	7	7 7	7 7	76	7 7	7	7	7	5	7 7 7	7 7	7 7	7 7	7 7 7	7 7	7 7	6 6	7 7	7	5	5	7 7
2.5	5	6		6 6	6 6		7	6	7	7	7	7	7	7	7	6	7	5	7	7	7	7	7 7	6 6	6 7	7 6	6	7	7	6	5 7 7	7 7	7 6	6	6 6 7	7 7	6 7	7 7	7 7	6	6	6	6 6
3.5	4	6	6		6 6		6	7	6	7	7	6	7	6	6	6	6	6	6	7	6	7	7 7	7 7	7 7	7 6	7	7	7	6	5 6 6	7 7	6 7	7 7	6 7 6	7 6	7 6	6 6	8 6	6	5	6	6 6
4.5	6	6	6		6 6		6	6	6	6	7	7	6	6	7	6	6	6	6	6	6	6	5 6	7 (	6 6	6 6	6	6	6	6	566	6 6	6 6	5 6	6 6 6	7 6	6 7	6 6	7 6	6	3	6	5 5
5.5 6.5	5	6	6		6 6		7	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6 6	6 6	7 6	6	6	6	6	6 6	6 6	6 6	6	6 6 6	6 5	6 6	6 6	6 6	6	4	6	5 5
7.5	4	6	6		6 6		6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6 6	6 6	6 6	6	7	6	6	6 6 6 6	6 6	6 6	5 5	6 6 6	6 6	6 6	6 6	6 6	6	5	6	5 5 4 4
8.5	3	6	6		6 6		6	6	6	6	6	6	6	7	6	6	6	6	6	6	6	6	5 6	6 6	6 5	6 6	6	6	6	5	6 6	6 6	6 5	6	6 5 6	6 5	6 6	6 6	6 6	6	5	6	3 3
9.5	4	6	6	6 -	6 6		6	6	6	6	6	6	6	6	5	6	6	6	5	7	6	6	6 6	6 6	6 5	6 6	6	6	5	4	5 6 6	6 6	6 5	5	5 5 6	7 5	6 6	6 6	6 6	5	5	6	3 3
10.5 11.5	5	5	6		6 6		6	6	6	6	6	6	7	6	5	6	6	6	6	6	6	5	6	6 6	6 6	6 5	6	6	6	6	6 5	6 6	6 5	5	5 6 6	5 5	6 6	6 6	5 6	5	5	6	3 3
11.5	3	4	6		6 6 6 6		6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	5	5 6	5 6	6 6	5	6	6	5		6 5	4 6	6	5 6 -	5 5	5 6	5 6	6 6	4	5	6	3 3
13.5	3	5	6		6 6		6	6	6	6	6	8	6	6	5	5	6	6	6	6	6	6	5 5	6	- 6	6 6	-	6	5	6		5 5		5 5	5 5 -	- 5	5 6	6 6	6 6	4	4	6	3 3
14.5	3	5	6		6 6		6	6	6	6	6	5	5	5	6	5	6	-	5	•	6	6	6 6	6	- 5	5 6	-	6	5	6			5 5	5 4	5 6 -	- 5	5 5	5 6	6 6	5	5	6	3 3
15.5	3	5	6		6 6		6	6	6	6	6	5	-	5	5	5	6	-	5	-	5	6	6 6	6	- 5	5 6	-	5	5	6	3 3 3	4 4		6		- 5	5 5	5 6	6 5	5	5	6	3 3
16.5 17.5	3	5	6		6 6		6	6	6	6	6	5	-	5	4	4	6	•	5	•	5	6	5 5	6	- 5	4 6	-	5	6	7	3 3 3	3 3		6		- 4	5 5	4 5	5 5	5	5	5	3 3
18.5	6	5	5		5 5		6	6	6	6	6	5	-	4	5	4	6	-	5		5	6	5 6	6	- 5	5 5		6	6	-	3 3 3	3 3	6 6	6 6 6	6 6 -	- 3	6 5	1 6	6 6	6	6	5	3 3
19.5	6	5	5		5 6		6	6	6	6	6	6	-	4	6	4	6		5	· •	6	6	6 6	6	- 6	4 6	-	6	5	-	3 3 3	3 - 3	5 6		6	- 5	5 5	3 6	6 5	6	6	5	3 3
20.5	6	4	5		5 6		6	6	6	5	6	4	-	4	6	4	6	-	6	•	6	6	5 5	6	- 6	5 6	-	6	5	-	3 3 3	3 3	5 6	5 6	6	- 5	5 6	5 6	6 5	6	6	4	3 3
21.5 22.5	5	5	5		5 6		6	6	6	5	6	5	-	5	5	5	5	•	5	·	6	6	5 5	6	- 6	5 6		6	5	· .	3 3 3	3 - 3	6 6		6	- 5	5 0	6 6	6 5	7	6	5	3 3
22.5	5	5	5	_	5 5		6	6	6	6	6	5		4	6	5	6		4	· ·	6	6	5 6	6	- 6	6 6	-	-	5	-	3 3 3	3 3	6 6 6 7	2 7	6	- 6	5 0	5 -	6 5		6	5	3 3
24.5	5	5	5		5 6		6	5	6	6	-	5		4	6	6	6		4		6	6	5 6	6	- 6	6 6	-		5		3 4 3		6 -		6	- 6	6 6	5 -	6 4	6	7	5	3 3
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Soil Behavior	
Type (SBT)	Soil Description
1	Sensitive, Fine-Grained
2	Organic Soils
3	Clays
4	Silt Mixtures
5	Sand Mixtures
6	Sands
7	Gravelly Sand to Sand
8	Stiff Sand to Clayey Sand
9	Very Stiff Fine-Grained

	51	52	53
	526+55 CPT20-58	SCPT20-59	526+59 CPT20-60
	526+55	✓ 526+57 S	526+59
	6	6	6
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Table 3 CPT Results Correlated Relative Density

Zone	1		2			3	4							5							6		7		8	9	I	10	11	1	12	13	14	15	Off Alignment	Backfill CPT
Reference	1	2	3 4	5 7	8	9	10	11	12	13	14 15	16	17	18	19 20	21	22	23 24	25 2	6 27	/ # 29	30 #	32 34	37	39	40 41 #	# # 4	9 50 54	55 58 60	0 63 66 #	# 69 7	70 71 72	73	74 75	44 46	51 52 53
Name/ Average Depth (ft)	CPT20-45	СРТ20-44	SCPT20-46 CPT20-47	СРТ20-43 СРТ20-42	SCPT20-41	СРТ20-40	СРТ20-39	CPT20-38	SCPT20-37	CPT20-36	CPT20-34 CPT20-35	CPT20-33	CPT20-32	SCPT20-31	CPT20-29 CPT20-30	CPT20-28	CPT20-27	SCPT20-26 SCPT20-53	CPT20-54		CPT20-23 CPT20-24	CPT20-56 CPT20-22	CPT20-21 CPT20-01B	SCPT20-02C	СРТ20-03В	CPT20-20 SCPT20-04 CPT20-19	SCPT20-05 EP120-05 CPT20-06 CPT20-16	СРТ20-07 СРТ20-08	CPT20-52 CPT20-14C CPT20-15B	CPT20-09C CPT20-09C CPT20-51C CPT20-61	SCPT20-50 CPT20-62	CPT20-10 CPT20-49 SCPT20-11	СРТ20-48	СРТ20-13 СРТ20-12	524+81 CPT20-57 525+02 SCPT20-57B	CPT20-58 SCPT20-59 CPT20-60
Station Burse 0.5 1.5	500+88	503+01	503+51		505+01	505+99	506+99	508+01	508+97	510+00	510+98	511+99	512+90	513+99	514+96 514+97	516+00	516+81	516+81 518+00	518+48	519+52	519+88 519+88	520+51	520+93	522+93	82+523	524+16 524+68 524+74	524+99 525+22 525+22 525+22	525+86	527+09 527+42 527+30	527+82 527+82 528+33 528+36	528+50	529+08 529+39 529+77	530+46	531+39 8 531+96	122 525+02 123 123	254 254 255 255 255 255 255 255 255 255
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6.5 7.5 8.5 9.5 10.5		82 70 76 71	74 59 82 80 66 71 88 84	- 84	86 65 73	82 72 84		80	00		88 83	95	83 74 65 64 61	81 77 75	88 86 84 83 84	8 89 87 76 68 71		86         68           83         61           68         62           79         51           78         44	66 52 8 61 7 54 6 58 4	7 2 85 5 74	66         54           59         38           85         -           76         42	83 88 64 78 70 87 66	81 71 62	75 63 41	70 37 36 58 55	38         35         65           28         38         61           29         52         45           32         48         42           28         20         21	63         78         44           62         68         53           58         68         53           48         69         44           39         60         33		81         89           71         -           60         64           40         54         77		88         87         7           88         49         6           89         61         6           64         65         5           39         74         5	54 32 47	61	- 50 - 40 - 47 - 61 - 66		89         87           70         76         70           68         60         60
11.5 12.5 13.5 14.5	-		68         64           74         79           69         66           64         69           59         63	- 88 - 70 - 62	72 72 75	80 82 77 80	68	73 48 65 77	85 74 67	83 85 71	74 71 - 55 81	72 68 61	61 54 54 54	69 63 54	71         83           63         70           58         76           55         -	i 73 i 65 i 57		No         44           80         45           81         35           55         42           61         45	63 63 57 59 55 3	- 50 - 48 - 45	70         42           66         40           66         57           -         55           -         53	36         36           74         -           59         40           55         37           36         51	54         65           -         68           -         66           -         75	49 58 58 -		30         35         21           51         -         -           47         -         -           39         -         -           -         -         -	53     -     45     -       44     -     36     -       27     -     34     -	49         48           39         53           -         46           -         -	49         51         67           -         40         -           -         60         -	7         55         55         61           -         -         -         40           -         -         -         40           -         -         -         40           -         -         -         -	31         35         6           32         -         8           29         62         6	57         -         35           88         65         32           57         62         40	-	- 62 - 55 - 70 - 70		80         87           70         76         70           68         -         60           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -           -         -         -
15.5 16.5 17.5 18.5 19.5	58	-	52         54           42         44           -         38           -         34	- 47 - 38  	81 73 66 54	90 82	87 80 85 76	80 73 77 72	59 52 57 54	67 60 54 44	  67 - 61 -	-	50 - - -	-	60 - 53 - 55 - 55 -			53         46           -         49           -         51           -         57	46 4 - 3 - 36 3	7 48	· · ·	- 53 - 55 44	- 46  - 56 - 45	- 50 - 30	-			38         73           40         85           9         50           5         80         88	54 - 69 - 81 -	<mark>36 39</mark> 56 -  57	38         -         5           33         -         7           -         -         7           -         -         7	55         59         25           74         52         -           76         -         52           67         56         -		- 61 		·         ·         ·           ·         ·         ·         ·           ·         ·         ·         ·           ·         ·         ·         ·
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24.5 25.5 26.5 27.5	51 61 76 72	35 51 58 59	  41 -	  36	38 35 -	59 66 63 53	- - -	61 62 63 56	28 29 - -	-	65 - 61 - 48 - -	-	67 57 - 66	49 69 71 55	64 - 73 - 80 - 85 -	- 57 65 -		72         58           73         69           77         70           78         67	57 59 66 7 68	82 9 7 86 5	- 74 - 66 - 78 - 79	56 54	 			-         -         69           -         73         71           -         70         64	64 66 7/ 90 8: 7/	  L	56         -         -           81         -         -           67         -         -           50         -         -	-         -         61           -         70         55           -         76         77           -         63         81	46         -         -           66         31         -           53         42         -           64         62         -	85         -           -         -         -           -         -         -           -         -         -	87 76 -	80 88 84 - 47	· · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · · ·	-         -         -           -         -         -           -         -         -           -         -         -
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Table 4 Visual Summary of N-Value

Zone	1		2			3	4							5								6			7	8		9		10	1	11	12		13	14	15	;	Off Alignment
Reference	1	2	3 4	7	8	9	10	11	12	13	14 15	16	17	18	19	20 2	1 22	23	24	25	26 27	# 29	30 #	32 3	4 37	39	40 4	41 # #	# 49	50 54	55 58	60 63 6	6 # # 6	9 70 7	1 72	73	74	75	44 46
Name/ Average Depth (ft)	CPT20-45	CPT20-44	SCPT20-46 CPT20-47	CPT20-43 CPT20-42	SCPT20-41	CPT20-40	CPT20-39	CPT20-38	SCPT20-37	CPT20-36	CPT20-34 CPT20-35	CPT20-33	CPT20-32	SCPT20-31	CPT20-29	CPT20-30	CF120-20	SCPT20-26	SCPT20-53	CPT20-54	CPT20-25 CPT20-55	CPT20-23 CPT20-24	CPT20-56 CPT20-22	CPT20-21	CPT20-02C	CPT20-03B	CPT20-20	SCPT20-04 CPT20-19 SCPT20-05	CPT20-06 CPT20-06 CPT20-16	CPT20-07 CPT20-08	CPT20-52 CPT20-14C	CPT20-15B CPT20-09C	CPT20-61 SCPT20-50		CPT20-49	CPT20-48	CPT20-13	CPT20-12	524+81 CPT20-57 <b>†</b>
Station Burse 0.5 1.5	s 500+88	10+E05 29		#	505+01	505+99	506+99	508+01	508+97	510+00	510+98 510+98	511+99	06+215 21	513+99	514+96	514+97	516+81	516+81	518+00	518+48	518+99	519+88 519+88	520+51	520+93	522+93 522+93	ه 523+78	524+16	524+68 524+74 524+99	525+22 525+58 525+58	525+86	527+09	527+30 527+82 527+82	528+46 528+50 528+50	80+675 24	529+39	530+46	66+1E5 18	11 531+96	524+81
2.5 3.5 4.5 5.5	21 22 9 26 12	29											30			26						Image: Constraint of the sector of				30	19	23 27		27 20			9		27		20 8 4 5	19 18 17	26 22 6 7
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11.5 12.5 13.5 14.5 15.5	7 3 3 3 3	7 8 13 14 10	28           27         25           24         27           21         24           18         19	- 22 - 21	29		28	29 16 25	29 25 21	28	30 15 25 9	26 27 23 18 14	22 23 20 20 14	26 24 17 14 12	29 24 21 20 22	27 19 2 24 2 - 11	6 0 5 - 5 -	19 23 14	15 13 14 16	23 20 21 20 15	6 17 8 17 5 16 9 18	18 13 16 20 - 20 - 16	29         18           21         14           19         12           10         18           8         19	13 2 10 2 - 2 -	17         20           27         21           13         10	8 17 27	18 16 11 3	7     3     18       3     2     14       2     1     10	- 15 6 - 9 5 - 10 6 - 10 6	17         12           11         14           7         9           8         10           12         21	19 14 9 14 8 20 13 24	17 14 2 - 15 1 9 9	0     19     11     1       7     12     11     8       0     7     10     1       0     8     10     1       1     12     15     1		12 12 25 11 23 13 18 12 22 8	5 4 4 4 6	6 6 7 9	21 14 27 27 23	3         3           2         3           2         2           3         3           3         3
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Table 5 Soil Improvement Criteria

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Figures









- 2020 IGE Boring
   2020 CPT Boring
   2020 SPT Boring
   Test Trench
   Storm Sewer Manhole
- Storm Sewer
- 10 ft Elevation Contour
- 2 ft Elevation Contour

CPT nomenclature is shortened for illustration purposes; for example, CPT20-05 is labeled as CPT-05.

Imagery: Nearmap 2020-04-16



INVESTIGATION LOCATIONS CENTRAL Raymond Road Project Verona, Wisconsin





0	2011 and 2016 CGC Bori
ullet	2019 SPT Boring
۲	2020 IGE Boring
$\heartsuit$	2020 CPT Boring
0	2020 SPT Boring
	Piezometer
	Test Trench
0	Storm Sewer Manhole
	Storm Sewer
	Badger Mill Creek
	10 ft Elevation Contour



- Elevations based on Soils & Engineering Services, Inc. (SES) boring logs (Boring 18 - Boring 16) collected on 10/4/19.

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- Geologic contacts are interpretations based on multiple subsurface project investigation methods.

- SPT N-values are shown at the bottom depth of the collected sample.

SP-SM- Sand with Silt and Gravel

- Approximate water table based on boring logs as either water levels at the time of drilling or the top depth described as "wet" on split spoon sample descriptions.

- Water elevation interpretations are from CPT pore water pressure observations in May 2020.

- See applicable boring logs for additional geotechnical information and details.

- Surficial fill inferred along roadway, as interpreted from in-roadway investigation locations. IGE 2019 Borings are projected from immediately south/southeast of roadway, with surficial fill superimposed.

Standard Penetration Test (SPT) N-value

Cone Penetration Test (CPT) Location

0

GE B18

OPT45



50 Approximate Horizontal Scale in Feet 5X Vertical Exaggeration



EAST

— 1030

— 1010

— 1000

— 990

GEOLOGIC CROSS SECTION Stations 500+50 to 506+00 Raymond Road Project Verona, Wisconsin



- Elevations based on Soils & Engineering Services, Inc. (SES) boring logs (Boring 16 - Boring 14) collected on 10/3/19-10/4/19.

- Geologic contacts are interpretations based on multiple subsurface project investigation methods.

- SPT N-values are shown at the bottom depth of the collected sample.

- Approximate water table based on boring logs as either water levels at the time of drilling or the top depth described as "wet" on split spoon sample descriptions.

- Water elevation interpretations are from CPT pore water pressure observations in May 2020.

- See applicable boring logs for additional geotechnical information and details.

- Surficial fill inferred along roadway, as interpreted from in-roadway investigation locations. IGE 2019 Borings are projected from immediately south/southeast of roadway, with surficial fill superimposed.



GEOLOGIC CROSS SECTION Stations 506+00 to 511+00 Raymond Road Project Verona, Wisconsin

Approximate Horizontal Scale in Feet

50

5X Vertical Exaggeration



- Elevations based on Soils & Engineering Services, Inc. (SES) boring logs (Boring 14 - Boring 11) collected on 10/3/19.

- Geologic contacts are interpretations based on multiple subsurface project investigation methods.

- SPT N-values are shown at the bottom depth of the collected sample.

- Approximate water table based on boring logs as either water levels at the time of drilling or the top depth described as "wet" on split spoon sample descriptions.

- Water elevation interpretations are from CPT pore water pressure observations in May 2020.

- See applicable boring logs for additional geotechnical information and details.

- Surficial fill inferred along roadway, as interpreted from in-roadway investigation locations. IGE 2019 Borings are projected from immediately south/southeast of roadway, with surficial fill superimposed.



GEOLOGIC CROSS SECTION Stations 511+00 to 517+00 **Raymond Road Project** Verona, Wisconsin

Approximate Horizontal Scale in Feet

50

5X Vertical Exaggeration



- Elevations based on Soils & Engineering Services, Inc. (SES) boring logs (Boring 11 - Boring 8) collected on 10/2/20-10/3/19.

- Geologic contacts are interpretations based on multiple subsurface project investigation methods.

- SPT N-values are shown at the bottom depth of the collected sample.

- Approximate water table based on boring logs as either water levels at the time of drilling or the top depth described as "wet" on split spoon sample descriptions.

- Water elevation interpretations are from CPT pore water pressure observations in May 2020.

- See applicable boring logs for additional geotechnical information and details.

- Surficial fill inferred along roadway, as interpreted from in-roadway investigation locations. IGE 2019 Borings are projected from immediately south/southeast of roadway, with surficial fill superimposed.



GEOLOGIC CROSS SECTION Stations 517+00 to 522+00 Raymond Road Project Verona, Wisconsin

Approximate Horizontal Scale in Feet

50

5X Vertical Exaggeration



- Water elevation interpretations are from CPT pore water pressure observations in May 2020 and VWP elevations collected on 6/1/20. - See applicable boring logs for additional geotechnical information and details.

- Maximum density per "Standard Proctor Test (ASTM D698)."

- Surficial fill inferred along roadway, as interpreted from in-roadway investigation locations. IGE 2019 Borings are projected from immediately south/southeast of roadway, with surficial fill superimposed.

GEOLOGIC CROSS SECTION Stations 522+00 to 528+00 Raymond Road Project Verona, Wisconsín



- Maximum density per "Standard Proctor Test (ASTM D698)."

- Surficial fill inferred along roadway, as interpreted from in-roadway investigation locations. IGE 2019 Borings are projected from immediately south/southeast of roadway, with surficial fill superimposed.



- Elevations based on Barr Engineering Co. (BARR) boring logs (SPT-20-03, SPT-20-06, and SPT-20-04) collected on 5/11/20-5/13/20 and the City of Madison's 2016 LiDAR acquisition.

- Geologic contacts are interpretations based on multiple subsurface project investigation methods.

- Construction features on the ground surface (e.g., curbs) are not shown.

- SPT N-values are shown at the bottom depth of the collected sample.

- Approximate water table based on boring logs as water levels at the time of drilling and VWP elevations collected on June 1, 2020.

- See applicable boring logs for additional geotechnical information and details.

- Surficial fill inferred along roadway, as interpreted from in-roadway investigation locations.



Scale is consistent with Figures 1-6

R



GEOLOGIC CROSS SECTION 524+97 to 525+10 Fisher Bren Privileged and Confidential Verona, Wisconsin



#### Figure 12 Vibrating Wire Piezometer Nest 1B Raymond Road Storm Sewer Verona, Wisconsin



Figure 13 Vibrating Wire Piezometer Nest 2B Raymond Road Storm Sewer Verona, Wisconsin



#### Figure 14 Vibrating Wire Piezometer Nest 3C Raymond Road Storm Sewer Verona, Wisconsin

Attachments

Attachment A ConeTec April 2020 Report

#### PRESENTATION OF SITE INVESTIGATION RESULTS

## Raymond Road Verona, Wisconsin

Prepared for:

**Barr Engineering** 

ConeTec Job No: 20-61-20766

Project Start Date: 16-Apr-2020 Project End Date: 20-Apr-2020 Report Date: 28-Apr-2020



Prepared by:

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## Introduction

The enclosed report presents the results of a piezocone penetration testing (CPTu or CPT) and seismic piezocone penetration testing (SCPTu or SCPT) program carried out along Raymond Road in Verona, Wisconsin. The site investigation program was conducted by ConeTec Inc. (ConeTec), under contract to Barr Engineering of Minneapolis, Minnesota.

A total of 62 cone penetration tests and 13 seismic cone penetration tests were completed at 62 locations (multiple locations were offset and attempted several times due to shallow refusal). The CPT and SCPT program was performed to evaluate the subsurface soil conditions. CPT and SCPT sounding locations were selected and numbered under supervision of Barr Engineering personnel (Brian Moynihan).

**Project Information** 

Project	
Client	Barr Engineering
Project	Raymond Road, Verona, WI
ConeTec project number	20-61-20766

A map from CESIUM including the CPT and SCPT test locations is presented below.





<b>Rig Description</b>	Deployment System	Test Type
CPT Truck Rig	25 ton truck mounted (twin cylinders)	CPT and SCPT

Coordinates		
Test Type	Collection Method	EPSG Number
CPT and SCPT	GPS (GlobalSat MR-350)	32616 (WGS 84 / UTM North)

Cone Penetration Test (CPT)							
Depth reference	Ground surface at the time of the investigation.						
Tip and sleeve data offset	0.1 meter. This has been accounted for in the CPT data files.						
Pore pressure dissipation (PPD)	Two pore pressure dissipation tests were completed primarily to						
tests	determine the phreatic surface.						
Additional plots	Advanced, Seismic and Soil Behavior Type (SBT) scatter plots are						
	included in the data release package.						

Cone Penetrometers Used for this Project								
Cone Description	Cone Number	Cross Sectional Area (cm <sup>2</sup> )	Sleeve Area (cm <sup>2</sup> )	Tip Capacity (bar)	Sleeve Capacity (bar)	Pore Pressure Capacity (psi)		
568:T1500F15U500	527	15	225	1500	15	500		
640:T1500F15U500	640	15	225	1500	15	500		
678:T1500F15U500	678	15	225	1500	15	500		
The CPT Summary indicat	es which con	e was used for ea	ach soundi	ng.				



Calculated Geotechnical Pa	arameters Tables
Additional information	The Normalized Soil Behavior Type Chart based on Q <sub>tn</sub> (SBT Qtn) (Robertson, 2009) was used to classify the soil for this project. A detailed set of calculated CPT parameters have been generated and are provided in Excel format files in the release folder. The CPT parameter calculations are based on values of corrected tip resistance (q <sub>t</sub> ) sleeve friction (f <sub>s</sub> ) and pore pressure (u <sub>2</sub> ). Effective stresses are calculated based on unit weights that have been assigned to the individual soil behavior type zones and the assumed equilibrium pore pressure profile. Soils were classified as either drained or undrained based on the Q <sub>tn</sub> Normalized Soil Behavior Type Chart (Robertson, 2009). Calculations for both drained and undrained parameters were included for materials that classified as silt mixtures (zone 4).

### Limitations

This report has been prepared for the exclusive use of Barr Engineering (Client) for the project titled "Raymond Road, Verona, WI". The report's contents may not be relied upon by any other party without the express written permission of ConeTec. ConeTec has provided site investigation services, prepared the factual data reporting and provided geotechnical parameter calculations consistent with current best practices. No other warranty, expressed or implied, is made.

The information presented in the report document and the accompanying data set pertain to the specific project, site conditions and objectives described to ConeTec by the Client. In order to properly understand the factual data, assumptions and calculations, reference must be made to the documents provided and their accompanying data sets, in their entirety.



Cone penetration tests (CPTu) are conducted using an integrated electronic piezocone penetrometer and data acquisition system manufactured by Adara Systems Ltd., a subsidiary of ConeTec.

ConeTec's piezocone penetrometers are compression type designs in which the tip and friction sleeve load cells are independent and have separate load capacities. The piezocones use strain gauged load cells for tip and sleeve friction and a strain gauged diaphragm type transducer for recording pore pressure. The piezocones also have a platinum resistive temperature device (RTD) for monitoring the temperature of the sensors, an accelerometer type dual axis inclinometer and a geophone sensor for recording seismic signals. All signals are amplified down hole within the cone body and the analog signals are sent to the surface through a shielded cable.

ConeTec penetrometers are manufactured with various tip, friction and pore pressure capacities in both 10 cm<sup>2</sup> and 15 cm<sup>2</sup> tip base area configurations in order to maximize signal resolution for various soil conditions. The specific piezocone used for each test is described in the CPT summary table presented in the first appendix. The 15 cm<sup>2</sup> penetrometers do not require friction reducers as they have a diameter larger than the deployment rods. The 10 cm<sup>2</sup> piezocones use a friction reducer consisting of a rod adapter extension behind the main cone body with an enlarged cross sectional area (typically 44 mm diameter over a length of 32 mm with tapered leading and trailing edges) located at a distance of 585 mm above the cone tip.

The penetrometers are designed with equal end area friction sleeves, a net end area ratio of 0.8 and cone tips with a 60 degree apex angle.

All ConeTec piezocones can record pore pressure at various locations. Unless otherwise noted, the pore pressure filter is located directly behind the cone tip in the " $u_2$ " position (ASTM Type 2). The filter is 6 mm thick, made of porous plastic (polyethylene) having an average pore size of 125 microns (90-160 microns). The function of the filter is to allow rapid movements of extremely small volumes of water needed to activate the pressure transducer while preventing soil ingress or blockage.

The piezocone penetrometers are manufactured with dimensions, tolerances and sensor characteristics that are in general accordance with the current ASTM D5778 standard. ConeTec's calibration criteria also meet or exceed those of the current ASTM D5778 standard. An illustration of the piezocone penetrometer is presented in Figure CPTu.





Figure CPTu. Piezocone Penetrometer (15 cm<sup>2</sup>)

The ConeTec data acquisition systems consist of a Windows based computer and a signal conditioner and power supply interface box with a 16 bit (or greater) analog to digital (A/D) converter. The data is recorded at fixed depth increments using a depth wheel attached to the push cylinders or by using a spring loaded rubber depth wheel that is held against the cone rods. The typical recording interval is 2.5 cm; custom recording intervals are possible. The system displays the CPTu data in real time and records the following parameters to a storage media during penetration:

- Depth
- Uncorrected tip resistance (q<sub>c</sub>)
- Sleeve friction (f<sub>s</sub>)
- Dynamic pore pressure (u)
- Additional sensors such as resistivity, passive gamma, ultra violet induced fluorescence, if applicable

All testing is performed in accordance to ConeTec's CPT operating procedures which are in general accordance with the current ASTM D5778 standard.



Prior to the start of a CPTu sounding a suitable cone is selected, the cone and data acquisition system are powered on, the pore pressure system is saturated with either glycerin or silicone oil and the baseline readings are recorded with the cone hanging freely in a vertical position.

The CPTu is conducted at a steady rate of 2 cm/s, within acceptable tolerances. Typically one meter length rods with an outer diameter of 1.5 inches are added to advance the cone to the sounding termination depth. After cone retraction final baselines are recorded.

Additional information pertaining to ConeTec's cone penetration testing procedures:

- Each filter is saturated in silicone oil under vacuum pressure prior to use
- Recorded baselines are checked with an independent multi-meter
- Baseline readings are compared to previous readings
- Soundings are terminated at the client's target depth or at a depth where an obstruction is encountered, excessive rod flex occurs, excessive inclination occurs, equipment damage is likely to take place, or a dangerous working environment arises
- Differences between initial and final baselines are calculated to ensure zero load offsets have not occurred and to ensure compliance with ASTM standards

The interpretation of piezocone data for this report is based on the corrected tip resistance  $(q_t)$ , sleeve friction  $(f_s)$  and pore water pressure (u). The interpretation of soil type is based on the correlations developed by Robertson et al. (1986) and Robertson (1990, 2009). It should be noted that it is not always possible to accurately identify a soil behavior based on these parameters. In these situations, experience, judgment and an assessment of other parameters may be used to infer soil behavior type.

The recorded tip resistance  $(q_c)$  is the total force acting on the piezocone tip divided by its base area. The tip resistance is corrected for pore pressure effects and termed corrected tip resistance  $(q_t)$  according to the following expression presented in Robertson et al. (1986):

$$q_t = q_c + (1-a) \cdot u_2$$

where:  $q_t$  is the corrected tip resistance

q<sub>c</sub> is the recorded tip resistance

u<sub>2</sub> is the recorded dynamic pore pressure behind the tip (u<sub>2</sub> position)

a is the Net Area Ratio for the piezocone (0.8 for ConeTec probes)

The sleeve friction ( $f_s$ ) is the frictional force on the sleeve divided by its surface area. As all ConeTec piezocones have equal end area friction sleeves, pore pressure corrections to the sleeve data are not required.

The dynamic pore pressure (u) is a measure of the pore pressures generated during cone penetration. To record equilibrium pore pressure, the penetration must be stopped to allow the dynamic pore pressures to stabilize. The rate at which this occurs is predominantly a function of the permeability of the soil and the diameter of the cone.



The friction ratio (Rf) is a calculated parameter. It is defined as the ratio of sleeve friction to the tip resistance expressed as a percentage. Generally, saturated cohesive soils have low tip resistance, high friction ratios and generate large excess pore water pressures. Cohesionless soils have higher tip resistances, lower friction ratios and do not generate significant excess pore water pressure.

A summary of the CPTu soundings along with test details and individual plots are provided in the appendices. A set of files with calculated geotechnical parameters were generated for each sounding based on published correlations and are provided in Excel format in the data release folder. Information regarding the methods used is also included in the data release folder.

For additional information on CPTu interpretations and calculated geotechnical parameters, refer to Robertson et al. (1986), Lunne et al. (1997), Robertson (2009), Mayne (2013, 2014) and Mayne and Peuchen (2012).

## References

ASTM D5778-12, 2012, "Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils", ASTM, West Conshohocken, US.

Lunne, T., Robertson, P.K. and Powell, J. J. M., 1997, "Cone Penetration Testing in Geotechnical Practice", Blackie Academic and Professional.

Mayne, P.W., 2013, "Evaluating yield stress of soils from laboratory consolidation and in-situ cone penetration tests", Sound Geotechnical Research to Practice (Holtz Volume) GSP 230, ASCE, Reston/VA: 406-420.

Mayne, P.W. and Peuchen, J., 2012, "Unit weight trends with cone resistance in soft to firm clays", Geotechnical and Geophysical Site Characterization *4*, Vol. 1 (Proc. ISC-4, Pernambuco), CRC Press, London: 903-910.

Mayne, P.W., 2014, "Interpretation of geotechnical parameters from seismic piezocone tests", CPT'14 Keynote Address, Las Vegas, NV, May 2014.

Robertson, P.K., Campanella, R.G., Gillespie, D. and Greig, J., 1986, "Use of Piezometer Cone Data", Proceedings of InSitu 86, ASCE Specialty Conference, Blacksburg, Virginia.

Robertson, P.K., 1990, "Soil Classification Using the Cone Penetration Test", Canadian Geotechnical Journal, Volume 27: 151-158.

Robertson, P.K., 2009, "Interpretation of cone penetration tests – a unified approach", Canadian Geotechnical Journal, Volume 46: 1337-1355.



Shear wave velocity (Vs) testing is performed in conjunction with the piezocone penetration test (SCPTu) in order to collect interval velocities. For some projects seismic compression wave velocity (Vp) testing is also performed.

ConeTec's piezocone penetrometers are manufactured with a horizontally active geophone (28 hertz) that is rigidly mounted in the body of the cone penetrometer, 0.2 meters behind the cone tip.

Shear waves are typically generated by using an impact hammer horizontally striking a beam that is held in place by a normal load. In some instances an auger source or an imbedded impulsive source maybe used for both shear waves and compression waves. The hammer and beam act as a contact trigger that initiates the recording of the seismic wave traces. For impulsive devices an accelerometer trigger may be used. The traces are recorded using an up-hole integrated digital oscilloscope which is part of the SCPTu data acquisition system. An illustration of the shear wave testing configuration is presented in Figure SCPTu-1.



Figure SCPTu-1. Illustration of the SCPTu system

All testing is performed in accordance to ConeTec's SCPTu operating procedures which are in general accordance with the current ASTM 5778 and ASTM D7400 standards.

Prior to the start of a SCPTu sounding, the procedures described in the Cone Penetration Test section are followed. In addition, the active axis of the geophone is aligned parallel to the beam (or source) and the horizontal offset between the cone and the source is measured and recorded.

Prior to recording seismic waves at each test depth, cone penetration is stopped and the rods are decoupled from the rig to avoid transmission of rig energy down the rods. Typically, five wave traces for each orientation are recorded for quality control purposes and uncertainty analysis. After reviewing wave traces for consistency the cone is pushed to the next test depth (typically one meter intervals or as requested by the client). Figure SCPTu-2 presents an illustration of a SCPTu test.





For additional information on seismic cone penetration testing refer to Robertson et. al. (1986).

Figure SCPTu-2. Illustration of a seismic cone penetration test

Calculation of the interval velocities are performed by visually picking a common feature (e.g. the first characteristic peak, trough, or crossover) on all of the recorded wave sets and taking the difference in ray path divided by the time difference between subsequent features. Ray path is defined as the straight line distance from the seismic source to the geophone, accounting for beam offset, source depth and geophone offset from the cone tip.

For all SCPTu soundings that have achieved a depth of at least 100 feet (30 meters), the average shear wave velocity to a depth of 100 feet ( $\bar{v}_s$ ) has been calculated using the following equation presented in ASCE (2010).

$$\bar{v}_s = \frac{\sum_{i=1}^n d_i}{\sum_{i=1}^n \frac{d_i}{v_{si}}}$$

where:  $\bar{v}_s$  = average shear wave velocity ft/s (m/s)

 $d_i$  = the thickness of any layer between 0 and 100 ft (30 m)

 $v_{si}$  = the shear wave velocity in ft/s (m/s)

 $\sum_{i=1}^{n} d_i$  = 100 ft (30 m)

Average shear wave velocity,  $\bar{v}_s$  is also referenced to V<sub>s100</sub> or V<sub>s30</sub>.

The layer travel times refers to the travel times propagating in the vertical direction, not the measured travel times from an offset source.

Tabular results and SCPTu plots are presented in the relevant appendix.



#### References

American Society of Civil Engineers (ASCE), 2010, "Minimum Design Loads for Buildings and Other Structures", Standard ASCE/SEI 7-10, American Society of Civil Engineers, ISBN 978-0-7844-1085-1, Reston, Virginia.

ASTM D5778-12, 2012, "Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils", ASTM, West Conshohocken, US.

ASTM D7400-14, 2014, "Standard Test Methods for Downhole Seismic Testing", ASTM, West Conshohocken, US.

Robertson, P.K., Campanella, R.G., Gillespie D and Rice, A., 1986, "Seismic CPT to Measure In-Situ Shear Wave Velocity", Journal of Geotechnical Engineering ASCE, Vol. 112, No. 8: 791-803.



The cone penetration test is halted at specific depths to carry out pore pressure dissipation (PPD) tests, shown in Figure PPD-1. For each dissipation test the cone and rods are decoupled from the rig and the data acquisition system measures and records the variation of the pore pressure (u) with time (t).



Figure PPD-1. Pore pressure dissipation test setup

Pore pressure dissipation data can be interpreted to provide estimates of ground water conditions, permeability, consolidation characteristics and soil behavior.

The typical shapes of dissipation curves shown in Figure PPD-2 are very useful in assessing soil type, drainage, in situ pore pressure and soil properties. A flat curve that stabilizes quickly is typical of a freely draining sand. Undrained soils such as clays will typically show positive excess pore pressure and have long dissipation times. Dilative soils will often exhibit dynamic pore pressures below equilibrium that then rise over time. Overconsolidated fine-grained soils will often exhibit an initial dilatory response where there is an initial rise in pore pressure before reaching a peak and dissipating.



Figure PPD-2. Pore pressure dissipation curve examples



In order to interpret the equilibrium pore pressure  $(u_{eq})$  and the apparent phreatic surface, the pore pressure should be monitored until such time as there is no variation in pore pressure with time as shown for each curve in Figure PPD-2.

In fine grained deposits the point at which 100% of the excess pore pressure has dissipated is known as  $t_{100}$ . In some cases this can take an excessive amount of time and it may be impractical to take the dissipation to  $t_{100}$ . A theoretical analysis of pore pressure dissipations by Teh and Houlsby (1991) showed that a single curve relating degree of dissipation versus theoretical time factor (T\*) may be used to calculate the coefficient of consolidation ( $c_h$ ) at various degrees of dissipation resulting in the expression for  $c_h$  shown below.

$$c_h = \frac{T^* \cdot a^2 \cdot \sqrt{I_r}}{t}$$

Where:

- T\* is the dimensionless time factor (Table Time Factor)
- a is the radius of the cone
- I<sub>r</sub> is the rigidity index
- t is the time at the degree of consolidation

Table Time Factor	. T* versus degree of dissipation (Teh and Houlsby (1991))
-------------------	------------------------------------------------------------

Degree of Dissipation (%)	20	30	40	50	60	70	80
T* (u <sub>2</sub> )	0.038	0.078	0.142	0.245	0.439	0.804	1.60

The coefficient of consolidation is typically analyzed using the time ( $t_{50}$ ) corresponding to a degree of dissipation of 50% ( $u_{50}$ ). In order to determine  $t_{50}$ , dissipation tests must be taken to a pressure less than  $u_{50}$ . The  $u_{50}$  value is half way between the initial maximum pore pressure and the equilibrium pore pressure value, known as  $u_{100}$ . To estimate  $u_{50}$ , both the initial maximum pore pressure and  $u_{100}$  must be known or estimated. Other degrees of dissipations may be considered, particularly for extremely long dissipations.

At any specific degree of dissipation the equilibrium pore pressure (u at  $t_{100}$ ) must be estimated at the depth of interest. The equilibrium value may be determined from one or more sources such as measuring the value directly ( $u_{100}$ ), estimating it from other dissipations in the same profile, estimating the phreatic surface and assuming hydrostatic conditions, from nearby soundings, from client provided information, from site observations and/or past experience, or from other site instrumentation.

For calculations of  $c_h$  (Teh and Houlsby (1991)),  $t_{50}$  values are estimated from the corresponding pore pressure dissipation curve and a rigidity index (I<sub>r</sub>) is assumed. For curves having an initial dilatory response in which an initial rise in pore pressure occurs before reaching a peak, the relative time from the peak value is used in determining  $t_{50}$ . In cases where the time to peak is excessive,  $t_{50}$  values are not calculated.

Due to possible inherent uncertainties in estimating  $I_r$ , the equilibrium pore pressure and the effect of an initial dilatory response on calculating  $t_{50}$ , other methods should be applied to confirm the results for  $c_h$ .



Additional published methods for estimating the coefficient of consolidation from a piezocone test are described in Burns and Mayne (1998, 2002), Jones and Van Zyl (1981), Robertson et al. (1992) and Sully et al. (1999).

A summary of the pore pressure dissipation tests and dissipation plots are presented in the relevant appendix.

#### References

Burns, S.E. and Mayne, P.W., 1998, "Monotonic and dilatory pore pressure decay during piezocone tests", Canadian Geotechnical Journal 26 (4): 1063-1073.

Burns, S.E. and Mayne, P.W., 2002, "Analytical cavity expansion-critical state model cone dissipation in fine-grained soils", Soils & Foundations, Vol. 42(2): 131-137.

Jones, G.A. and Van Zyl, D.J.A., 1981, "The piezometer probe: a useful investigation tool", Proceedings, 10<sup>th</sup> International Conference on Soil Mechanics and Foundation Engineering, Vol. 3, Stockholm: 489-495.

Robertson, P.K., Sully, J.P., Woeller, D.J., Lunne, T., Powell, J.J.M. and Gillespie, D.G., 1992, "Estimating coefficient of consolidation from piezocone tests", Canadian Geotechnical Journal, 29(4): 551-557.

Sully, J.P., Robertson, P.K., Campanella, R.G. and Woeller, D.J., 1999, "An approach to evaluation of field CPTU dissipation data in overconsolidated fine-grained soils", Canadian Geotechnical Journal, 36(2): 369-381.

Teh, C.I., and Houlsby, G.T., 1991, "An analytical study of the cone penetration test in clay", Geotechnique, 41(1): 17-34.



The appendices listed below are included in the report:

- Cone Penetration Test Summary and Standard Cone Penetration Test Plots
- Advanced Cone Penetration Test Plots with Ic, Su(Nkt), Phi and N1(60)Ic
- Seismic Cone Penetration Test Plots
- Seismic Cone Penetration Test Shear Wave (Vs) Traces
- Seismic Cone Penetration Test Tabular Results
- Soil Behavior Type (SBT) Scatter Plots
- Pore Pressure Dissipation Summary and Pore Pressure Dissipation Plots



# Cone Penetration Test Summary and Standard Cone Penetration Test Plots





Client:

Project:

Start Date:

End Date:

20-61-20766 Barr Engineering Raymond Road, Verona, WI 16-Apr-2020 20-Apr-2020

		СО	NE PENETRATIO	N TEST SL	IMMARY				
Sounding ID	File Name	Date	Cone	Assumed Phreatic Surface <sup>1</sup> (ft)	Final Depth (ft)	Shear Wave Velocity Tests	Northing <sup>2</sup> (m)	Easting <sup>2</sup> (m)	Refer to Notation Number
CPT20-01	20-61-20766_CP01	16-Apr-2020	678:T1500F15U500		17.55		4766289	294048	4
CPT20-01B	20-61-20766_CP01B	17-Apr-2020	640:T1500F15U500	19.0	22.15		4766290	294047	3
CPT20-02	20-61-20766_CP02	17-Apr-2020	640:T1500F15U500		2.79		4766308	294074	4
CPT20-02B	20-61-20766_CP02B	17-Apr-2020	640:T1500F15U500		6.07		4766308	294075	4
SCPT20-02C	20-61-20766_SP02C	17-Apr-2020	640:T1500F15U500	18.5	40.03		4766308	294072	3
CPT20-03	20-61-20766_CP03	17-Apr-2020	640:T1500F15U500		3.28		4766321	294095	4
CPT20-03B	20-61-20766_CP03B	17-Apr-2020	640:T1500F15U500	15.0	16.57		4766321	294097	3
SCPT20-04	20-61-20766_SP04	17-Apr-2020	640:T1500F15U500	18.5	36.25	9	4766343	294114	3
SCPT20-05	20-61-20766_SP05	17-Apr-2020	640:T1500F15U500	23.5	30.51	7	4766351	294124	3
CPT20-06	20-61-20766_CP06	17-Apr-2020	640:T1500F15U500	11.5	35.43		4766359	294132	3
CPT20-07	20-61-20766_CP07	17-Apr-2020	640:T1500F15U500	15.0	24.11		4766363	294144	3
CPT20-08	20-61-20766_CP08	17-Apr-2020	640:T1500F15U500	21.0	23.46		4766377	294169	3
CPT20-09	20-61-20766_CP09	17-Apr-2020	640:T1500F15U500		2.79		4766392	294199	4
CPT20-09B	20-61-20766_CP09B	17-Apr-2020	640:T1500F15U500		3.12		4766393	294200	4
CPT20-09C	20-61-20766_CP09C	17-Apr-2020	640:T1500F15U500		12.63		4766389	294295	4
CPT20-10	20-61-20766_CP10	17-Apr-2020	640:T1500F15U500	18.5	21.49		4766402	294236	3
SCPT20-11	20-61-20766_SP11	17-Apr-2020	640:T1500F15U500	20.0	31.00	8	4766417	294248	3
CPT20-12	20-61-20766_CP12	18-Apr-2020	640:T1500F15U500	19.0	37.40		4766408	294327	3
CPT20-13	20-61-20766_CP13	18-Apr-2020	640:T1500F15U500	17.0	40.03		4766408	294311	3
CPT20-14	20-61-20766_CP14	18-Apr-2020	640:T1500F15U500		10.01		4766378	294187	4
CPT20-14B	20-61-20766_CP14B	18-Apr-2020	640:T1500F15U500		10.50		4766379	294189	4



Client:

Project:

Start Date:

End Date:

20-61-20766 Barr Engineering Raymond Road, Verona, WI 16-Apr-2020 20-Apr-2020

	CONE PENETRATION TEST SUMMARY										
Sounding ID	File Name	Date	Cone	Assumed Phreatic Surface <sup>1</sup> (ft)	Final Depth (ft)	Shear Wave Velocity Tests	Northing <sup>2</sup> (m)	Easting <sup>2</sup> (m)	Refer to Notation Number		
CPT20-14C	20-61-20766_CP14C	18-Apr-2020	640:T1500F15U500	17.0	19.03		4766381	294186	3		
CPT20-15	20-61-20766_CP15	18-Apr-2020	640:T1500F15U500		4.10		4766378	294189	4		
CPT20-15B	20-61-20766_CP15B	18-Apr-2020	640:T1500F15U500		12.14		4766378	294185	4		
CPT20-16	20-61-20766_CP16	18-Apr-2020	640:T1500F15U500	23.5	40.03		4766358	294136	3		
CPT20-17	20-61-20766_CP17	18-Apr-2020	640:T1500F15U500	17.0	31.00		4766356	294126	3		
CPT20-18	20-61-20766_CP18	18-Apr-2020	640:T1500F15U500		9.51		4766348	294126	4		
CPT20-19	20-61-20766_CP19	18-Apr-2020	640:T1500F15U500	20.0	47.08		4766336	294115	3		
CPT20-20	20-61-20766_CP20	18-Apr-2020	640:T1500F15U500	19.0	62.66		4766334	294103	3		
CPT20-21	20-61-20766_CP21	18-Apr-2020	640:T1500F15U500		13.12		4766261	294029	4		
CPT20-22	20-61-20766_CP22	18-Apr-2020	640:T1500F15U500	24.5	50.03		4766267	294031			
CPT20-23	20-61-20766_CP23	18-Apr-2020	640:T1500F15U500		13.12		4766241	294013	4		
CPT20-24	20-61-20766_CP24	18-Apr-2020	640:T1500F15U500	18.0	30.68		4766242	294010	3		
CPT20-25	20-61-20766_CP25	18-Apr-2020	640:T1500F15U500	23.0	35.11		4766226	293984	3		
SCPT20-26	20-61-20766_SP26	18-Apr-2020	640:T1500F15U500	20.0	32.81	9	4766200	293965	3		
CPT20-27	20-61-20766_CP27	18-Apr-2020	640:T1500F15U500		13.62		4766165	293947	4		
CPT20-28	20-61-20766_CP28	19-Apr-2020	640:T1500F15U500	20.0	33.79		4766152	293928	3		
CPT20-29	20-61-20766_CP29	19-Apr-2020	640:T1500F15U500	19.0	34.28		4766131	293910	3		
CPT20-30	20-61-20766_CP30	18-Apr-2020	640:T1500F15U500		14.27		4766128	293910	4		
SCPT20-31	20-61-20766_SP31	19-Apr-2020	640:T1500F15U500	17.5	32.81	10	4766108	293891	3		
CPT20-32	20-61-20766_CP32	19-Apr-2020	568:T1500F15U500	21.0	31.17		4766088	293866	3		
CPT20-33	20-61-20766_CP33	19-Apr-2020	568:T1500F15U500	18.0	27.89		4766065	293853	3		



Client:

Project:

Start Date:

End Date:

20-61-20766 Barr Engineering Raymond Road, Verona, WI 16-Apr-2020 20-Apr-2020

	CONE PENETRATION TEST SUMMARY										
Sounding ID	File Name	Date	Cone	Assumed Phreatic Surface <sup>1</sup> (ft)	Final Depth (ft)	Shear Wave Velocity Tests	Northing <sup>2</sup> (m)	Easting <sup>2</sup> (m)	Refer to Notation Number		
CPT20-34	20-61-20766_CP34	19-Apr-2020	568:T1500F15U500	23.0	28.71		4766042	293829	3		
CPT20-35	20-61-20766_CP35	19-Apr-2020	568:T1500F15U500		15.09		4766039	293831	4		
CPT20-36	20-61-20766_CP36	19-Apr-2020	568:T1500F15U500	20.0	24.11		4766020	293809	3		
SCPT20-37	20-61-20766_SP37	19-Apr-2020	568:T1500F15U500	28.0	33.30	10	4765991	293783	3		
CPT20-38	20-61-20766_CP38	19-Apr-2020	568:T1500F15U500	24.0	32.81		4765971	293763	3		
CPT20-39	20-61-20766_CP39	19-Apr-2020	568:T1500F15U500	29.0	33.14		4765945	293743	3		
CPT20-40	20-61-20766_CP40	19-Apr-2020	568:T1500F15U500	28.0	33.14		4765929	293727	3		
SCPT20-41	20-61-20766_SP41	19-Apr-2020	568:T1500F15U500	29.0	34.12	11	4765910	293706	3		
CPT20-42	20-61-20766_CP42	19-Apr-2020	568:T1500F15U500	28.0	34.12		4765883	293684	3		
CPT20-43	20-61-20766_CP43	19-Apr-2020	568:T1500F15U500		8.53		4765882	293685	4		
CPT20-43B	20-61-20766_CP43B	19-Apr-2020	568:T1500F15U500		15.09		4765880	293686	4		
CPT20-44	20-61-20766_CP44	19-Apr-2020	568:T1500F15U500	25.0	33.96		4765863	293666	3		
CPT20-45	20-61-20766_CP45	19-Apr-2020	568:T1500F15U500	14.0	32.15		4765796	293638	3		
SCPT20-46	20-61-20766_SP46	20-Apr-2020	568:T1500F15U500	26.0	33.14	10	4765872	293673	3		
CPT20-47	20-61-20766_CP47	20-Apr-2020	568:T1500F15U500	29.0	33.14		4765878	293681	3		
CPT20-48	20-61-20766_CP48	20-Apr-2020	568:T1500F15U500	18.0	30.02		4766425	294270	3		
CPT20-49	20-61-20766_CP49	20-Apr-2020	568:T1500F15U500	18.0	30.02		4766415	294243	3		
SCPT20-50	20-61-20766_SP50	20-Apr-2020	568:T1500F15U500	22.0	33.14	10	4766400	294214	3		
CPT20-51	20-61-20766_CP51	20-Apr-2020	568:T1500F15U500		15.58		4766393	294213	4		
CPT20-51B	20-61-20766_CP51B	20-Apr-2020	568:T1500F15U500		3.45		4766395	294209	4		
CPT20-51C	20-61-20766_CP51C	20-Apr-2020	568:T1500F15U500	28.0	31.66		4766386	294216	3		



Client:

Project:

Start Date:

End Date:

20-61-20766 Barr Engineering Raymond Road, Verona, WI 16-Apr-2020 20-Apr-2020

	CONE PENETRATION TEST SUMMARY											
Sounding ID	File Name	Date	Cone	Assumed Phreatic Surface <sup>1</sup> (ft)	Final Depth (ft)	Shear Wave Velocity Tests	Northing <sup>2</sup> (m)	Easting <sup>2</sup> (m)	Refer to Notation Number			
CPT20-52	20-61-20766_CP52	20-Apr-2020	568:T1500F15U500	20.0	37.24		4766370	294180	3			
SCPT20-53	20-61-20766_SP53	20-Apr-2020	568:T1500F15U500	22.5	32.15	10	4766202	293968	3			
CPT20-54	20-61-20766_CP54	20-Apr-2020	568:T1500F15U500	21.0	31.00		4766218	293977	3			
CPT20-55	20-61-20766_CP55	20-Apr-2020	568:T1500F15U500	21.0	31.00		4766239	294003	3			
CPT20-56	20-61-20766_CP56	20-Apr-2020	568:T1500F15U500	21.0	31.00		4766261	294020	3			
CPT20-57	20-61-20766_CP57	20-Apr-2020	568:T1500F15U500	17.0	33.46		4766353	294116	3			
SCPT20-57B	20-61-20766_SP57B	20-Apr-2020	568:T1500F15U500	17.0	43.80	13	4766355	294122	3			
CPT20-58	20-61-20766_CP58	20-Apr-2020	568:T1500F15U500		11.16		4766374	294165	4			
SCPT20-59	20-61-20766_SP59	20-Apr-2020	568:T1500F15U500		11.16	5	4766375	294166	4			
CPT20-60	20-61-20766_CP60	20-Apr-2020	568:T1500F15U500		11.16		4766376	294167	4			
CPT20-61	20-61-20766_CP61	20-Apr-2020	568:T1500F15U500	20.0	30.02		4766396	294215	3			
CPT20-62	20-61-20766_CP62	20-Apr-2020	568:T1500F15U500	20.9	31.00		4766399	294218				
Totals	75 soundings				1917.96	112						

1. The assumed phreatic surface was based on pore pressure dissipation tests. Hydrostatic data were used for the calculated parameters.

2. Coordinates were acquired using a MR-350 GlobalSat GPS Receiver in datum: WGS84 / UTM Zone 16 North.

3. The assumed phreatic surface was estimated from the dynamic pore pressure data.

4. No phreatic surface detected.









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→ Hydrostatic Line O Ueq O Assumed Ueq O PPD, Ueq achieved O PPD, Ueq not achieved The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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Advanced Cone Penetration Plots with Ic, Su(Nkt), Phi and N1(60)Ic





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Seismic Cone Penetration Test Plots





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Seismic Cone Penetration Test Shear Wave (Vs) Traces









Date: 17-Apr-2020





Date: 17-Apr-2020





Date: 18-Apr-2020





Date: 19-Apr-2020





Date: 19-Apr-2020





Date: 19-Apr-2020





















Project: Raymond Road, Verona, WI

Sounding: SCPT20-59

Filter: 25-200hz

Date: 20-Apr-2020

120

Job No: 20-61-20766

Client: Barr Engineering

Seismic Cone Penetration Test Tabular Results





Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI SCPT20-04 Sounding ID: Date: 17-Apr-2020 Seismic Source: Beam Seismic Offset (ft): 1.97 Source Depth (ft): 0.00

Geophone Offset (ft): 0.66

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs							
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)			
3.28	2.63	3.28						
9.84	9.19	9.40	6.11	12.55	487			
13.12	12.47	12.62	3.23	6.02	536			
16.40	15.75	15.87	3.25	6.57	495			
19.69	19.03	19.13	3.26	6.35	513			
22.97	22.31	22.40	3.27	6.59	496			
26.25	25.59	25.67	3.27	6.55	500			
32.81	32.15	32.21	6.55	6.50	1007			
36.25	35.60	35.65	3.44	2.87	1198			



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI SCPT20-05 Sounding ID: Date: 17-Apr-2020 Seismic Source: Beam Seismic Offset (ft): 1.97 Source Depth (ft): 0.00 Geophone Offset (ft): 0.66

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs							
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)			
3.28	2.63	3.28						
6.56	5.91	6.23	2.94	7.30	403			
16.40	15.75	15.87	9.65	13.80	699			
19.69	19.03	19.13	3.26	4.60	709			
22.97	22.31	22.40	3.27	4.53	721			
26.25	25.59	25.67	3.27	3.53	926			
29.53	28.87	28.94	3.27	3.64	898			



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI SCPT20-11 Sounding ID: Date: 17-Apr-2020 Seismic Source: Beam Seismic Offset (ft): 1.97 Source Depth (ft): 0.00

Geophone Offset (ft): 0.66

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs						
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)		
3.28	2.63	3.28					
9.84	9.19	9.40	6.11	15.60	392		
13.12	12.47	12.62	3.23	6.18	523		
16.40	15.75	15.87	3.25	5.50	591		
19.69	19.03	19.13	3.26	3.57	915		
22.97	22.31	22.40	3.27	2.43	1345		
26.25	25.59	25.67	3.27	2.49	1313		
29.53	28.87	28.94	3.27	2.29	1430		



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-26 Date: 18-Apr-2020 B Seismic Source: Source Offset (ft): Source Depth (ft):

Geophone Offset (ft):

Beam
1.97
0
0.66

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs							
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)			
3.28	2.62	3.28						
6.56	5.91	6.23	2.94	3.87	762			
9.84	9.19	9.40	3.17	4.61	687			
13.12	12.47	12.62	3.23	3.54	912			
16.40	15.75	15.87	3.25	5.16	630			
19.69	19.03	19.13	3.26	3.77	865			
22.97	22.31	22.40	3.27	5.22	625			
26.25	25.59	25.67	3.27	4.17	785			
29.53	28.87	28.94	3.27	3.03	1082			



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI SCPT20-31 Sounding ID: Date: 19-Apr-2020 Seismic Source: Beam Seismic Offset (ft): 1.97 Source Depth (ft): 0.00

Geophone Offset (ft): 0.66

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs							
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)			
3.28	2.63	3.28						
6.56	5.91	6.23	2.94	4.75	620			
9.84	9.19	9.40	3.17	3.97	798			
13.12	12.47	12.62	3.23	4.53	713			
16.40	15.75	15.87	3.25	3.97	818			
19.69	19.03	19.13	3.26	3.86	844			
22.97	22.31	22.40	3.27	4.09	800			
26.25	25.59	25.67	3.27	3.75	871			
29.53	28.87	28.94	3.27	3.97	823			
32.81	32.15	32.21	3.27	2.96	1105			



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-37 Date: 19-Apr-2020 Seismic Source: Beam Source Offset (ft): 1.97 Source Depth (ft): 0

Geophone Offset (ft):

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs							
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)			
3.28	2.62	3.28						
6.56	5.91	6.23	2.94	3.60	817			
9.84	9.19	9.40	3.17	3.37	940			
13.12	12.47	12.62	3.23	4.28	754			
16.40	15.75	15.87	3.25	4.10	792			
19.69	19.03	19.13	3.26	5.54	589			
22.97	22.31	22.40	3.27	5.48	596			
26.25	25.59	25.67	3.27	4.30	761			
29.53	28.87	28.94	3.27	5.27	621			
32.81	32.15	32.21	3.27	4.25	771			



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-41 Date: 19-Apr-2020 Seismic Source: Beam Source Offset (ft): 1.97 Source Depth (ft): 0

Geophone Offset (ft):

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs							
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)			
3.28	2.62	3.28						
6.56	5.91	6.23	2.94	4.35	677			
9.84	9.19	9.40	3.17	4.07	780			
13.12	12.47	12.62	3.23	3.80	849			
16.40	15.75	15.87	3.25	3.97	819			
19.69	19.03	19.13	3.26	3.55	918			
22.97	22.31	22.40	3.27	5.50	593			
26.25	25.59	25.67	3.27	4.80	682			
29.53	28.87	28.94	3.27	4.74	690			
32.81	32.15	32.21	3.27	3.80	862			
34.12	33.46	33.52	1.31	1.19	1100			



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-46 Date: 20-Apr-2020 Seismic Source: Beam Source Offset (ft): 1.97 Source Depth (ft): 0

Geophone Offset (ft):

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs						
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)		
3.28	2.62	3.28					
6.56	5.91	6.23	2.94	3.89	757		
9.84	9.19	9.40	3.17	3.89	815		
13.12	12.47	12.62	3.23	3.68	877		
16.40	15.75	15.87	3.25	4.89	664		
19.52	18.86	18.97	3.10	4.16	744		
22.97	22.31	22.40	3.43	6.84	501		
26.25	25.59	25.67	3.27	6.33	517		
29.53	28.87	28.94	3.27	5.00	655		
32.81	32.15	32.21	3.27	4.77	687		



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-50 Date: 20-Apr-2020 Seismic Source: Beam Source Offset (ft): 1.97 Source Depth (ft): 0

Geophone Offset (ft):

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs						
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)		
3.28	2.62	3.28					
6.56	5.91	6.23	2.94	4.43	665		
9.84	9.19	9.40	3.17	4.56	695		
13.12	12.47	12.62	3.23	6.03	535		
16.40	15.75	15.87	3.25	7.45	436		
19.69	19.03	19.13	3.26	5.85	557		
23.13	22.47	22.56	3.43	4.78	718		
25.10	24.44	24.52	1.96	3.02	649		
29.53	28.87	28.94	4.42	4.46	990		
32.81	32.15	32.21	3.27	3.34	980		



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-53 Date: 20-Apr-2020 Seismic Source: Beam Source Offset (ft): 1.97 Source Depth (ft): 0

Geophone Offset (ft):

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs						
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)		
3.28	2.62	3.28					
6.56	5.91	6.23	2.94	4.37	674		
9.84	9.19	9.40	3.17	5.26	603		
13.12	12.47	12.62	3.23	5.60	576		
16.40	15.75	15.87	3.25	4.78	680		
19.69	19.03	19.13	3.26	4.38	744		
22.97	22.31	22.40	3.27	4.84	674		
26.25	25.59	25.67	3.27	4.05	808		
29.53	28.87	28.94	3.27	3.37	970		
32.15	31.50	31.56	2.62	2.68	978		



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-57B Date: 20-Apr-2020 Seismic Source: Beam Source Offset (ft): 1.97 Source Depth (ft): 0

Geophone Offset (ft):

	SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs							
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)			
3.28	2.62	3.28						
6.56	5.91	6.23	2.94	5.62	524			
9.84	9.19	9.40	3.17	7.75	409			
13.12	12.47	12.62	3.23	7.31	441			
16.40	15.75	15.87	3.25	7.67	424			
19.69	19.03	19.13	3.26	6.73	485			
22.97	22.31	22.40	3.27	6.34	515			
26.25	25.59	25.67	3.27	7.22	453			
29.53	28.87	28.94	3.27	6.45	507			
32.81	32.15	32.21	3.27	4.08	802			
36.09	35.43	35.49	3.28	2.31	1415			
39.37	38.71	38.76	3.28	2.00	1638			
42.65	41.99	42.04	3.28	1.97	1666			



Job No: 20-61-20766 Client: Barr Engineering Project: Raymond Road, Verona, WI Sounding ID: SCPT20-59 Date: 20-Apr-2020 Seismic Source: Beam Source Offset (ft): 1.97 Source Depth (ft): 0 Geophone Offset (ft): 0.66

SCPTu SHEAR WAVE VELOCITY TEST RESULTS - Vs					
Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)
2.95	2.30	3.03			
5.08	4.43	4.85	1.82	2.55	714
7.05	6.40	6.69	1.85	2.79	663
9.02	8.37	8.59	1.90	3.66	519
11.15	10.50	10.68	2.09	1.69	1235

Soil Behavior Type (SBT) Scatter Plots



Job No: 20-61-20766 Date: 2020-04-16 10:19 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-01 Cone: 678:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 09:17 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-01B Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 10:06 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-02 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 10:17 Site: Raymond Road, Verona, WI

### Sounding: CPT20-02B Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 10:34 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-02C Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 11:56 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-03 Cone: 640:T1500F15U500


Job No: 20-61-20766 Date: 2020-04-17 12:22 Site: Raymond Road, Verona, WI

### Sounding: CPT20-03B Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 12:57 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-04 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 13:56 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-05 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 14:46 Site: Raymond Road, Verona, WI

### Sounding: CPT20-06 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 15:23 Site: Raymond Road, Verona, WI

### Sounding: CPT20-07 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 15:56 Site: Raymond Road, Verona, WI Sounding: CPT20-08 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 16:30 Site: Raymond Road, Verona, WI

### Sounding: CPT20-09 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 16:38 Site: Raymond Road, Verona, WI

### Sounding: CPT20-09B Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 16:50 Site: Raymond Road, Verona, WI

### Sounding: CPT20-09C Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 17:23 Site: Raymond Road, Verona, WI

### Sounding: CPT20-10 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-17 18:02 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-11 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 08:33 Site: Raymond Road, Verona, WI Sounding: CPT20-12 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 09:10 Site: Raymond Road, Verona, WI

### Sounding: CPT20-13 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 09:52 Site: Raymond Road, Verona, WI

### Sounding: CPT20-14 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 10:02 Site: Raymond Road, Verona, WI

### Sounding: CPT20-14B Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 10:20 Site: Raymond Road, Verona, WI

### Sounding: CPT20-14C Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 10:50 Site: Raymond Road, Verona, WI

### Sounding: CPT20-15 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 11:00 Site: Raymond Road, Verona, WI

### Sounding: CPT20-15B Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 11:24 Site: Raymond Road, Verona, WI Sounding: CPT20-16 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 12:03 Site: Raymond Road, Verona, WI

### Sounding: CPT20-17 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 12:35 Site: Raymond Road, Verona, WI

### Sounding: CPT20-18 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 12:47 Site: Raymond Road, Verona, WI Sounding: CPT20-19 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 13:27 Site: Raymond Road, Verona, WI Sounding: CPT20-20 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 14:19 Site: Raymond Road, Verona, WI

### Sounding: CPT20-21 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 14:42 Site: Raymond Road, Verona, WI

### Sounding: CPT20-22 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 17:03 Site: Raymond Road, Verona, WI

### Sounding: CPT20-23 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 17:26 Site: Raymond Road, Verona, WI

### Sounding: CPT20-24 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 17:59 Site: Raymond Road, Verona, WI Sounding: CPT20-25 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 18:38 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-26 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 19:34 Site: Raymond Road, Verona, WI

### Sounding: CPT20-27 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 08:58 Site: Raymond Road, Verona, WI

### Sounding: CPT20-28 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 08:25 Site: Raymond Road, Verona, WI

### Sounding: CPT20-29 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-18 20:01 Site: Raymond Road, Verona, WI

### Sounding: CPT20-30 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 09:30 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-31 Cone: 640:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 10:45 Site: Raymond Road, Verona, WI

### Sounding: CPT20-32 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 11:22 Site: Raymond Road, Verona, WI

### Sounding: CPT20-33 Cone: 568:T1500F15U500


Job No: 20-61-20766 Date: 2020-04-19 11:53 Site: Raymond Road, Verona, WI

### Sounding: CPT20-34 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 12:22 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-35 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 12:52 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-36 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 13:28 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-37 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 14:51 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-38 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 15:23 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-39 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 16:01 Site: Raymond Road, Verona, WI

### Sounding: CPT20-40 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 16:32 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-41 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 17:40 Site: Raymond Road, Verona, WI Sounding: CPT20-42 Cone: 568:T1500F15U500



Standard SBT Chart (UBC 1986)

Job No: 20-61-20766 Date: 2020-04-19 18:09 Site: Raymond Road, Verona, WI

### Sounding: CPT20-43 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 18:25 Site: Raymond Road, Verona, WI

### Sounding: CPT20-43B Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 18:49 Site: Raymond Road, Verona, WI

### Sounding: CPT20-44 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-19 19:30 Site: Raymond Road, Verona, WI Sounding: CPT20-45 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 08:45 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-46 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 09:30 Site: Raymond Road, Verona, WI

### Sounding: CPT20-47 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 10:16 Site: Raymond Road, Verona, WI Sounding: CPT20-48 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 10:48 Site: Raymond Road, Verona, WI Sounding: CPT20-49 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 11:21 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-50 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 12:50 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-51 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 13:11 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-51B Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 13:21 Site: Raymond Road, Verona, WI

### Sounding: CPT20-51C Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 13:56 Site: Raymond Road, Verona, WI

### Sounding: CPT20-52 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 14:41 Site: Raymond Road, Verona, WI

### Sounding: SCPT20-53 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 15:33 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-54 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 15:57 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-55 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 16:20 Site: Raymond Road, Verona, WI

### Sounding: CPT20-56 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 16:59 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-57 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 17:27 Site: Raymond Road, Verona, WI

#### Sounding: SCPT20-57B Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 18:23 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-58 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 18:34 Site: Raymond Road, Verona, WI

#### Sounding: SCPT20-59 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 18:59 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-60 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 19:16 Site: Raymond Road, Verona, WI

#### Sounding: CPT20-61 Cone: 568:T1500F15U500



Job No: 20-61-20766 Date: 2020-04-20 19:35 Site: Raymond Road, Verona, WI Sounding: CPT20-62 Cone: 568:T1500F15U500



Pore Pressure Dissipation Summary and Pore Pressure Dissipation Plots





Job No: Client: Project: Start Date: End Date: 20-61-20766 Barr Engineering Raymond Road, Verona, WI 17-Apr-2020 20-Apr-2020

CPTu PORE PRESSURE DISSIPATION SUMMARY						
Sounding ID	File Name	Cone Area (cm <sup>2</sup> )	Duration (s)	Test Depth (ft)	Estimated Equilibrium Pore Pressure U <sub>eq</sub> (ft)	Calculated Phreatic Surface (ft)
CPT20-22	20-61-20766_CP22	15	75	34.45	10.0	24.5
CPT20-62	20-61-20766_CP62	15	70	31.00	10.1	20.9
Total Duration	2 dissipations		2.4 min			



Job No: 20-61-20766 Date: 04/18/2020 14:42 Site: Raymond Road, Verona, WI Sounding: CPT20-22 Cone: 640:T1500F15U500 Area=15 cm<sup>2</sup>




Job No: 20-61-20766 Date: 04/20/2020 19:35 Site: Raymond Road, Verona, WI Sounding: CPT20-62 Cone: 568:T1500F15U500 Area=15 cm<sup>2</sup>



Attachment B Soil Borings



# **Standard Guide to Geotechnical Logs**

	TI	ne Unified Soil	Classification System (AS	TM D-2487)			
			OUP SYMBOLS AND	SOIL CL	ASSIFICATION		
GROUP NAMES USING LABO				GROUP SYMBOL	GROUP NAME <sup>B</sup>		
ave		CLEAN GRAVELS	$C_{_{U}} \ge 4$ and $1 \le C_{_{C}} \le 3^{E}$	GW	Well-graded gravel <sup>F</sup>		
Coarse-grained soils Nore than 50% retained on the No. 200 sieve	GRAVELS More than 50% of coarse	Less than 5% fines <sup>c</sup>	$C_{u}$ > 4 and/or 1 > $C_{c}$ > 3 <sup>E</sup>	GP	Poorly graded gravel <sup>F</sup>		
	fraction is retained on No. 4 sieve	GRAVELS WITH FINES	Fines classify as ML or MH	GM	Silty gravel FGH		
		More than 12% fines <sup>c</sup>	Fines classify as CL or CH	GC	Clayey gravel FGH		
		CLEAN	$C_{_{U}} \ge 6 \text{ and } 1 \le C_{_{C}} \le 3^{E}$		Well-graded sand '		
	SANDS 50% or more of	SANDS More than 12%	$C_{u}$ < 6 and/or 1 > $C_{c}$ > 3 <sup>E</sup>	SP	Poorly graded sand <sup>/</sup>		
	coarse fraction passes No. 4 sieve	SANDS WITH FINES	Fines classify as ML or MH	SM	Silty sand GHI		
Ĕ		More than 12% fines <sup>D</sup>	Fines classify as CL or CH	SC	Clayey sand <sup>GHI</sup>		
θ	LOW	Inorganic	PI > 7 and plots on or above the "A" line <sup>J</sup>	CL	Lean clay KLM		
) siev	PLASTICITY SILTS AND		PI < 4 or plots below "A" line J	ML	Silt KLM		
Fine-grained soils or more passes the No. 200 sieve	CLAYS Liquid Limit less than 50	Organic	Liquid Limit (oven dried) < 0.75 Liquid Limit (not dried)	OL	Organic clay <sup>KLMN</sup> Organic silt <sup>KLMO</sup>		
e-gi e pas	HIGH	Inorganic	PI plots on or above the "A" lines	СН	Fat clay KLM		
r mor	PLASTICITY SILTS AND	Inorganic	PI plots below the "A" line	МН	Elastic silt KLM		
50% or	CLAYS Liquid Limit 50 or more	CLAYS .iquid Limit 50 or Organic Liquid Limit (oven dried) < 0. Liquid Limit (not dried)		ОН	Organic clay <sup>KLMP</sup> Organic silt <sup>KLMQ</sup>		
	Highly organic	soils	Primarily organic matter, dark in color, and organic odor	PT	Peat		

A. Based on material passing the 3-inch (75-mm) sieve

- B. If field sample contained cobbles and/or boulders, add "with cobbles and/or boulders" to group name.
- C. Gravels with 5 to 12% fines by weight require dual symbols.

GW-GM: well-graded gravel with silt GW-GC: well-graded gravel with clay GP-GM: poorly graded gravel with silt GP-GC: poorly graded gravel with clay

Sands with 5 to 12% fines by weight require dual symbols. SW-SM: well-graded sand with silt

SW-SM: well-graded sand with slit SW-SC: well-graded sand with clay SP-SM: poorly graded sand with slit SP-SC: poorly graded sand with clay

$$C_{c} = \frac{D_{30}^{2}}{D_{60}^{*} D_{10}}$$
  $C_{u} = \frac{D_{60}}{D_{10}}$ 

If soil contains ≥ 15% sand by weight, add "with sand" to group name. If soil contains < 15% sand by weight, add "trace sand" to group name.

If fines classify as CL-ML, use dual symbol GC-GM, SC-SM.

If fines are organic, add "with organic fines" to group name.

If soil contains ≥ 15% gravel by weight, add "with gravel" to group name. If soil contains < 15% gravel by weight, add "trace gravel" to group name.

If the Liquid Limit and Plasticity Index plot in hatched area on plasticity chart, soil is a CL-ML, silty clay.

K. If soil contains 15 to 29% plus No. 200 by weight, add "with sand" or "with gravel," whichever is predominant. If soil contains < 15% plus No. 200 by weight, add "trace sand" or "trace gravel," whichever is predominant.

. If soil contains ≥ 30% plus No. 200 by weight, predominantly sand, add "sandy" to group name.

M. If soil contains ≥ 30% plus No. 200 by weight, predominantly gravel, add "gravelly" to group name.

PI ≥ 4 and plots on or above "A" line

- PI < 4 or plots below "A" line
- P. PI plots on or above "A" line
- Q. PI plots below "A" line

Representative Particle Sizes									
Boulders	Larger than 12"								
Cobbles	3" to 12"								
Coarse gravel	3/4" to 3"								
Fine gravel	No. 4 (4.75 mm) sieve to 3/4"								
Coarse sand	No. 10 (2 mm) to No. 4 (4.75 mm) sieve								
Medium sand	No. 40 (0.425 mm) to No. 10 (2 mm) sieve								
Fine sand	No. 200 (0.075mm) to No. 40 (0.425 mm) sieve								
Silt	0.002 mm to No. 200 (0.075mm) sieve								
Clay	Finer than 0.002 mm								



Sampling Symbols									
SS	Split spoon (2" OD, unless otherwise noted)								
3T	3" Shelby tube/thin wall								
5T	5" Shelby tube/thin wall								
MCS	Modified California sampler								
PS	Piston sampler								
PT	Pitcher sampler								
тс	Texas cone penetrometer								
WS	Wash sample								
HA	Hand auger sampler								
BS	Miscellaneous bag/bottle/bucket bulk sample								
NR	No recovery								
RS	Rotosonic								
DP	Direct push (Geoprobe)								
RC	Rock core								
VC	Vibro core								
СТ	Cuttings								

	Laboratory Tests
DD	Dry density
WD	Wet density
MC	Natural moisture content
LL	Liquid Limit
PL	Plastic Limit
PI	Plascticity Index
P200	Percent passing No. 200 sieve
OC	Organic content
S	Degree of saturation
SG	Specific gravity
φ	Angle of internal friction
Q <sub>u</sub>	Unconfined compressive strength
С	Shear strength

	Drilling Symbols
HSA	Hollow-stem auger
MRO	Mud rotary
WRO	Water rotary
ARO	Air rotary
BRO	Biodegradable mud rotary
DBC	Diamond bit coring
TTC	Triple tube coring with diamond bit
DPT	Direct push technology (Geoprobe)
DHH	Downhole hammer
RSC	Rotosonic coring
TP	Test pit

	Miscellaneous Codes
$Q_p$	Undrained shear strength from pocket penetrometer
TV	Undrained shear strength from torvane shear test
FV	Field vane shear test
REM	Remolded field vane shear test
NPT	Water pressure (packer) test
WH	Penetration test <sup>3</sup> produced 1-foot penetration under weight of rods and hammer alone. No driving required.
WR	Penetration test <sup>3</sup> produced 1-foot penetration under weight of rods alone. No driving required.
RQD	Rock quality designation

Exploration Methods
Hollow-stem auger borings performed in accordance with ASTM Test Method D-6151
Flight auger borings performed in accordance with ASTM Test Method D-1452
Test pits typically performed with a backhoe
Rock coring performed in accordance with ASTM Test Method D-2113
RQD performed in accordance with ASTM Test Method D-6032
Penetration test <sup>3</sup> performed in accordance with ASTM Test Method D-1586
Thin wall tube sampling performed in accordance with ASTM Test Method D-1587
Soil classification performed in accordance with ASTM Test Method D-2487

Water Level Measurement Symbols									
ED	End of drilling								
WS	While sampling								
WD	While drilling								
BCR	Before casing removal								
ACR	After casing removal								
AI	Immediately after installation								
WCI	Wet cave-in								
DCI	Dry cave-in								

Relative Density of Cohesionless Soils <sup>1</sup>										
N-value <sup>2</sup> Qualitative Density Description Estimated D										
< 4 BPF	Very loose	D <sub>R</sub> < 15%								
4 to 10 BPF	Loose	D <sub>R</sub> up to 35%								
10 to 30 BPF	Medium dense	D <sub>R</sub> up to 65%								
30 to 50 BPF	Dense	D <sub>R</sub> up to 85%								
> 50 BPF	Very dense	D <sub>R</sub> > 85%								

Consistency of Fine-Grained Soils <sup>1</sup>										
N-value <sup>2</sup>	Estimated Q <sub>u</sub>									
< 2 BPF	Very soft	Q <sub>u</sub> < 0.25 tsf								
2 and 4 BPF	Soft	Q <sub>u</sub> up to 0.50 tsf								
4 to 8 BPF	Medium stiff	Q <sub>u</sub> up to 1.0 tsf								
8 to 15 BPF	Stiff	Q <sub>u</sub> up to 2.0 tsf								
15 to 30 BPF	Very stiff	Q <sub>u</sub> up to 4.0 tsf								
> 30 BPF	Hard	Q <sub>u</sub> > 4.0 tsf								

<sup>1</sup>*Reference: Terzaghi, Peck, and Mesri,* Soil Mechanics in Engineering Practice, *third edition,* 1996.

 $^2Based$  on  $N_{_{60}}$  values corresponding to 60% hammer efficiency.

<sup>3</sup>The penetration resistance, or N-value, is the total number of blows required to drive two successive 6" rams with a 2" split-spoon sampler. The sampler is driven with a 140-pound hammer falling 30", unless otherwise noted. The N-value is reported in blows per foot (BPF).



BARR TEMPL													
ORT BARR						L	OG	OF	BO	RIN	G	1A	
Project: Job No. Locatio Coordir Datum:	: .: n: nates:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,854.0 ft E 783,655.3 ft NAVD 88	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	1004 HSA Split 9.0 f	Ba					Sh	leet	<u>1 of</u>	<u>1</u>
WEARR.COMPROJECTS/WPLS49 WI1349131019 VERONA WI SINKHOLE EVALUATION/WORKFILES/FIELD DATA/BORING AND TEST TRENCH LOGS/SOIL BORINGS/FISHER BREN BORING LOGS.GFU BARR.LIBRARY.GLB BOREHOLE LOG REPORT ULI COT OF 000 1 0 000 1 0 0 0 0 0 0 0 0 0 0 0 0	Depth, feet	MATERIAL DESCRIPTION	Completion Depth.	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	STANE TEST 10 REC%[ 20 SHE	DATA 20	N in b 30 $0\% \blacklozenge$ 60	10ws/ft 40 80	t ©
	0	Surface Elev.: 1004.6 ft Crushed asphalt/ Topsoil mix. (004.1 ft POORLY GRADED SAND (SP): very fine to fine grained; tan; mo medium dense; slight silt; 5% fines. 999.6 ft	/ 0.5f		X	1	100	13 9	0 1 9 9		2,5		E
	5	SILTY SAND (SM): tan to brown; moist; very loose; 20% fines. 998.1 ft POORLY GRADED SAND (SP): very fine to fine grained; tan; mo dense; 95% sand. 995.6 ft Bottom of Boring at 9.0 feet	5.0f iist; medium 6.5f 9.0f		X	3	100 100	4	4	2			
4/BORING AND TEST TI													
ORKFILES/FIELD DATA													
KHOLE EVALUATION/W													
31019 VERONA WI SIN													
2TS\MPLS\49 WI\13\4915													
Date Bo Date Bo Logged Drilling Drill Ric	oring C I By: Contra	Completed: 6/29/19 CSM	Remarks: over sewe Weather:	r pipe.						feet.	Boring	g locat	ted

UKI BAKA						LC	G	OF	BOF	RING	G 1	в
BA	RR	,								She	et 1	of 1
	: n: nates:	Verona, Wisconsin N 463,850.8 ft E 783,651.5 ft	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	1004 HSA Split 29.0	Bai							
AK Y										DATAN		ws/ft ©
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION		Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	10 REC% 20 SHE/	20 RQD 40 AR STF	60	40 80 H, tsf
D N N		Surface Elev.: 1004.9 ft		1.17.					0	2	5	5
L RC	-	TOPSOIL: Crushed asphalt/topsoil. ~1003.9 ft			X	1	78	14		4		
	-	POORLY GRADED SAND (SP): very fine to fine grained; tan; mois medium dense; 10% fines.	st; very loose to		X	2	67	10				
1000- 	5 —	Silty sand lense observed at 5 feet (20% fines).			X	3	83	6	6 ©			
	-				X	4	78	7	70			
995 	10				X	5	78	3	30			
	-	989.9 ft			X	6	61	2	2			
	15	WELL GRADED SAND (SW): fine to coarse grained; very loose; 5	% gravel. 15.0f	t	X	7	33	2	2 ©			
	· _	Rod drop 12-inches by weight of Auto-Hammer; moist to wet at 17 $\ensuremath{\neg}986.5$ ft	.5 feet. /18.4f			8	33	3	30			
985-	20-	Lens of silty sand (5 inches) at 18 feet WELL GRADED SAND WITH GRAVEL (SW): medium brown; sat to very dense; lense of course sand and gravel at 26 feet.	/		X	9	33	28	+		28 ©	
	-				X	10	56	21		21 101		
980	25				X	11	50	44				44
	-	975.9 ft Bottom of Boring at 29.0 feet	29.0f	F	X	12	50	59				
			20.01					·				
Date Bo Date Bo Logged Drilling Drill Rig	oring C By: Contra	ompleted: 6/29/19 CSM Time of Drilling 17.5	Remarks: Weather:	sunny a	appro	oxima	tely 9	0 F				

TEMPL											
ORT BARF					L	C	OF	BOR	ING	i 2/	A
Project: Job No. Locatio Coordir B Datum:	: .: n: nates:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin IN A403,866.01 E 783,565.6 ft     Surface Elevators: IN AAD 88     1004.8 ft Drilling Method: Surface Elevators: IN AAD 88     ISA Surface Elevators: IN AAD 8	et 1	of 1							
WBARR.COMPROJECTS/WP1349131019 VERONA WI SINKHOLE EVALUATION/WORKFILES/FIELD DATABORING AND TEST TRENCH LOGS/SOIL BORINGS/FISHER BREN BORING LOGS.GFJ BARR.LBRARY.GLB BOREHOLE LOG REPORT BARR TEMPLA         ULUCITION       ULUCITION         ULUCITION       ULUCITION	Depth, feet		Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	TEST DA	ATA N i 20 RQD % 40	n blow 30 60	/s/ft © 40 80
- BREN BORIN 	BARE     Surface Elevation:     1004.8 ft       ub No.     49191019.00/100/300     Surface Elevation:     1004.8 ft       Disting Method:     Sylf Barrel     Completion Digit:     9.0 ft       warm     NAVD 88     MATERIAL DESCRIPTION     But and the second se	22 ©									
RENCH LOGS/SOIL BOR		Fisher Bren 49191019.00/100/300 Verona, Wisconsin nete::::::::::::::::::::::::::::::::::									
A/BORING AND TEST T			Elevation:       1004.8 ft         Lethod:       HSA Method:       Split Barrelion         on Depth:       9.0 ft       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split Barrelion       Image: Split Barrelion       Image: Split Barrelion         Image: Split								
ORKFILES/FIELD DAT/									Image: Sheet 1         Image		
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1019 VERONA WI SIN											
TS/MPLS/49 WI/13/4913											
Date Bo Date Bo Logged Drilling Drill Ric	oring C I By: Contra	Completed: 6/28/19 CSM Time of Drilling Dry							rushec	l asph	halt

						Sheet 1 I.8 ft Barrel ft v or of a standard PENETRA TEST DATA N in blow 10 20 30 REC% RQD % RQD % 20 40 60					
ORT BARR							L	C	OF	BORING 2	B
GREP	<b>BA</b>	RR	,							Sheet 1	of 1
JOHENOL BOREHOL Co	B       Surface Elev::       1004.8 ft         TOPSOIL/CRUSHED ASPHALT MIX.       TOPSOIL/CRUSHED ASPHALT MIX.         POORLY GRADED SAND WITH SILT (SP-SM): very fine:         brown; moist; loose to medium dense; 90% sand, 10% fine         1000       5         At 5.5 feet 95% sand and 5% fines, fine to medium grain, s         995       10         10       -         995       -         10       -         985       -         985       -         985       -         985       -         985       -         985       -         985       -         986       -         987       -         985       -         985       -         985       -         985       -         986       -         987       -         988       -         980       -         981       -         983       -         984       -         985       -         980       -         981       -         981	49191019.00/100/300 Verona, Wisconsin N 463,856.8 ft E 783,568.7 ft	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	HSA Split	Ba						
BRARY			Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463.856.8 ft E 783.568.7 ft NAVD 88       Surface Elevation: 1004.8 ft Dilling Method: Split Barrel         MATERIAL DESCRIPTION       9 gl	TEST DATA N in blo							
DATAIBORING AND TEST TRENCH LOGS/SOIL BORINGS/FISHER BREN BORING LOGS.GPJ BARRLIBRARY	Project: Fisher Bren ob No: 49191019.00/100/300 coation: Verona, Wisconsin Coordinates: N 463,856.8 ft E 783,568.7 ft NAVD 88 MATERIAL DESCRIPTION Surface Elev: 1004.8 ft TOPSOIL/CRUSHED ASPHALT MIX. 1000 5 - At 5.5 feet 95% sand and 5% fines, fine to medium grain, s 995 10 996 10 - 45.5 feet 95% sand and 5% fines, fine to medium grain, s 997 10 998 3 ft WELL GRADED SAND WITH GRAVEL (SW): fine to coarse provin; moist; loose to medium brown; saturated; loose; 65% s 988 20 983 3 ft WELL GRADED SAND WITH GRAVEL (SW): fine to coarse 983 16 980 25 981 3 ft WELL GRADED SAND (SM): medium brown; saturated; loose; 65% s 983 17 WELL GRADED SAND (SW): fine to coarse grained; mediu very loose; 95% sand, 5% fines. 978.8 ft POORLY GRADED GRAVEL WITH SAND (GP): medium t medium dense; angular to subrounded. 975.8 ft Bottom of Boring at 29.0 feet Water Levels (ft Bottom of Boring at 29.0 feet At Time of Drilling Contractor: SEE	MATERIAL DESCRIPTION		Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	REC% RQD % ◆ 20 40 60	80 TH, tsf	
DRING		-0-		00/300 Isin E 783,568.7 ft     Surface Elevation: Dilling Method: E 783,568.7 ft     1004.8 ft HSA Samping Method: Split Barrel Completion Deph: 29.0 ft       MATERIAL DESCRIPTION     Image: Completion Deph: Bigging bigging bigg	5						
ER BREN BO	-	-	Sheet 1 of 1         Fisher Bren 4919101500/100/300 Verona, Wiscontain NAS3 656.8 ft E 783,568.7 ft       Burlane Elexition: Dilling Method: HSA Sampling Method: Split Barrel Completion Depth:       1004.8 ft 29.0 ft         MATERIAL DESCRIPTION         Status and the status a								
NGS/FISHE	- - -000	-		Sheet 1 of 1         r Bren (019.00/100/300 (a) Wisconsin (a) Wisconsin (b) 56       Surface Elevation: 1004.8 ft Drilling Method: HSA Sampling Method: Split Barrel Completion Depty: 29.0 ft         MATERIAL DESCRIPTION         MATERIAL DESCRIPTION         Surface Elevation: 1004.8 ft UCRUSHED ASPHALT MIX: ICRUSHED ASPHALT							
VSOIL BORI	-	-	At 5.5 feet 95% sand and 5% fines, fine to medium grain, slight co								
	opject:       Fisher Bren 49191019.00/100/300 vection:       Verona, Wisconsin N 463,856.8 ft E 783,568.7 ft NAVD 88         mathematical statum:       N 403,856.8 ft E 783,568.7 ft NAVD 88         mathematical statum:       NAVD 88         mathematical statum:       NAVD 88         mathematical statum:       Surface Elew:: 1004.8 ft         TOPSOIL/CRUSHED ASPHALT MIX.			X	4	100	11				
D TEST TRE		10	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,856.8 ft E 783,568.7 ft NAVD 88         MATERIAL DESCRIPTION         Surface Elev::       1004.8 ft         TOPSOIL/CRUSHED ASPHALT MIX.         1003.8 ft         POORLY GRADED SAND WITH SILT (SP-SM): very fine to fine brown; moist; loose to medium dense; 90% sand, 10% fines.         At 5.5 feet 95% sand and 5% fines, fine to medium grain, slight of well GRADED SAND WITH GRAVEL (SW): fine to coarse grain brown; loose; subrounded; 25% gravel, 75% sand, very moist to signification brown; loose; subrounded; 25% gravel, 75% sand, very moist to signification brown; loose; 95% sand, 5% fines.         986.3 ft       WELL GRADED SAND WITH GRAVEL (SW): fine to coarse grain brown; loose; subrounded; 25% gravel, 75% sand, very moist to signification brown; loose; 95% sand, 5% fines.         981.3 ft       SILTY SAND (SM): medium brown; saturated; loose; 65% sand, - 983.3 ft         981.3 ft       WELL GRADED SAND (SW): fine to coarse grained; medium brown very loose; 95% sand, 5% fines.         978.8 ft       POORLY GRADED GRAVEL WITH SAND (GP): medium brown medium dense; angular to subrounded.         975.8 ft       Bottom of Boring at 29.0 feet         Bottom of Boring at 29.0 feet       Ya Time of Drilling 12			X	5	100	13		
ORING ANI		-		Du/100/300 Sconsin 8 ft E 783,568.7 ft       Surface Elevation: Drilling Method: Samping Method: Split Barrel Completion Depth: 29.0 ft       HSA Samping Method: Split Barrel Completion Depth: 29.0 ft         MATERIAL DESCRIPTION       Bot og b g g g g g g g g g g g g g g g g g g	100	10					
	-90 - -	15			7	100	5	5' ©			
RKFILES/FII	1		Fisher Bren 49191019.00/100/300 Verona, Wisconsin N463,856.8 ft E 783,568.7 ft NAVD 88       Surface El Completion         0       Surface Elev:: 1004.8 ft TOPSOL/CRUSHED ASPHALT MIX.       Completion         0       TOPSOL/CRUSHED ASPHALT MIX.       1003.8 ft POORLY GRADED SAND WITH SILT (SP-SM): very fine to fine grained; me brown; moist; loose to medium dense; 90% sand, 10% fines.       10         5       At 5.5 feet 95% sand and 5% fines, fine to medium grain, slight coarse sand.       10         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         10       -       -         986.3 ft       -       - </td <td></td> <td>5ft •••••</td> <td></td> <td>8</td> <td>50</td> <td>8</td> <td></td> <td></td>		5ft •••••		8	50	8		
	- 285	20-	983.3 ft	Sheet 1 of 1 D/100/300 Consistin If: E 783.568.7 ft MATERIAL DESCRIPTION MATERIAL D							
	-	-	WELL GRADED SAND (SW): fine to coarse grained; medium brow	wn; saturated; 23.	5ft	$\mathbb{Y}$	10	50	4	4	
	-080 - -	25	978.8 ft POORLY GRADED GRAVEL WITH SAND (GP): medium brown; ;	saturated; 26.			11	50	22	Sheet 1         STANDARD PENETR         TEST DATA N in blow         10       20       30         RQD % ●       30         SHEAR STRENGTH       25         14       17         15       17         17       9         10       13         11       13         10       9         5       8         7       9         4       22         8       9         7       9         4       22	
9 VERON	Surface Elev:         1004.8 ft         20           TOPSOLIC/CIUSHED ASPHALT MIX.         TOPSOLIC/CIUSHED ASPHALT MIX.         1.0ft         1         83         14         14         14           POORLY GRADED SAND WITH SLT (SP-SM): very fine to fine grained; medium         1.0ft         1.0ft         1         83         14         14         14           POORLY GRADED SAND WITH SLT (SP-SM): very fine to fine grained; medium         1.0ft         1         1         13         100         15           10         5         At 5.5 feet 35% sand and 5% fines, fine to medium grain, slight coarse sand.         1         4         100         11         13         10         15         10         13         10         15         10         13         10         15         10         13         10         15         10         13         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10 <t< td=""><td></td></t<>										
3/491310			Bottom of Boring at 29.0 feet	29.	σπ						
CTSMPLS49 WN1349131019 VERONA WI SINKHOLE EVALUATION/WORKFILES/FIELD											
щ Ор Da		0       TOPSOIL/CRUSHED ASPHALT MIX. <ul> <li>1003.8 ft</li> <li>POORLY GRADED SAND WITH SILT (SP-SM): very fine to ft brown; moist; loose to medium dense; 90% sand, 10% fines.</li> <li>5</li> <li>At 5.5 feet 95% sand and 5% fines, fine to medium grain, sligt</li> <li>10</li> <li>10</li> <li>11</li> <li>10</li> <li>986.3 ft</li> <li>WELL GRADED SAND WITH GRAVEL (SW): fine to coarse to brown; loose; subrounded; 25% gravel, 75% sand, very moist lobrown; loose; subrounded; 25% gravel, 75% sand, very moist lobrown; loose; subrounded; 25% gravel, 75% sand, very moist lobrown; loose; 95% sand, 5% fines.</li> <li>981.3 ft</li> <li>WELL GRADED SAND (SW): fine to coarse grained; medium very loose; 95% sand, 5% fines.</li> <li>978.8 ft</li> <li>POORLY GRADED GRAVEL WITH SAND (GP): medium brom medium dense; angular to subrounded.</li> <li>975.8 ft</li> <li>Boring Completed:</li> <li>6/28/19</li> <li>Water Levels (ft)</li> <li>Y At Time of Drilling of Completed:</li> <li>96/28/19</li> <li>Y At Time of Drilling</li> </ul>			s:						
Da Lo Dr	ite Bo gged illing	oring C By: Contra	Sheet 1 c         Fisher Bren (931910102000) WA 633 868 R E 783.568.7 ft NAVD 88       Surface Elevation: Dulling Method: Surface Elevation: NAVD 88       1004.8 ft HSA Sanging Method: Surface Elevation: NAVD 88       TORSOLOGY Surface Elevation: NAVD 88         MATERIAL DESCRIPTION       Image: Surface Elevation: Surface Elevation: NAVD 88       Image: Surface Elevation: Surface Elevation: Surface Elevation: Surface Elevation: Surface Elevation: NAVD 88       Image: Surface Elevation: Surface Elev								
BA		,. ,		Weather	r: sunny a	appr	oxim	ately	90 F		

BA	RR	  ,				LO	g of	BORING 3A
Project: Job No. Locatior Coordin	: n: lates:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,540.1 ft E 782,965.8 ft NAVD 88	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	1003 HSA Split 10.0	Bar			Sheet 1 of
Elevation, feet	troject: Fisher Bren ob No. 49191019.00/100/300 ocation: Verona, Wisconsin NAVD 88		Graphic Log	Samples	Sample No.	% recovery SPT, N value or ROD %	STANDARD PENETRATION TEST DATA N in blows/ft @ 10 20 30 40 REC% RQD % ◆ 20 40 60 80 SHEAR STRENGTH, tsf	
Project: Fisher Bren Job No.: 49191019.00/100/300 Location: Verona, Wisconsin Coordinates: N 463,540.1 ft E 782,965.8 ft Datum: NAVD 88 MATERIAL DESCRIPTIO Surface Elev.: 1003.7 ft Surface Elev.: 1003.7 ft Surface Elev.: 1003.7 ft O Surface Elev.: 1003.7 ft Difficult drilling from approximately 1.5 feet to 3 feet due to 1 Decreasing gravel from 7.5 to 9 feet. Increasing gravel from 9 to 10 feet. 0995 Decreasing gravel from 9 to 10 feet. 000 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			X		1 23 2 17			
Job No.: 49191019.00/100/300 Location: Verona, Wisconsin Coordinates: N 463,540.1 ft E 782,965.8 ft Datum: NAVD 88	Increasing gravel from 9 to 10 feet.			X	4 4	60 19 4 16 60 14	19 16 9 14 9	
Project: Fisher Bren Job No: 49191019.00/100/300 Location: Verona, Wisconsin Coordinates: N 463,540.1 ft E 782,965.8 ft Datum: NAVD 88	bolloni or bolling at 10.0 reet							
		49191019.00/100/300 Verona, Wisconsin NAVD 88       Drilling Method:       I Sampling Method:       Sampling Method: <td< td=""><td></td><td></td><td></td><td></td><td></td></td<>						
	Boring Started: Boring Started: Boring Completed: Boring Completed:							
Date Bo Logged Drilling	No::     49191019.00/100/300 Verona, Wisconsin N 463,540.1 ft E 782,965.8 ft     Drilling Meth Sampling Met Completion E       Image: N 463,540.1 ft E 782,965.8 ft     Completion E       Image: N 463,540.1 ft E 782,965.8 ft     Completion E       Image: N 463,540.1 ft E 782,965.8 ft     Completion E       Image: N 463,540.1 ft E 782,965.8 ft     Completion E       Image: N 463,540.1 ft E 782,965.8 ft     Completion E       Image: N 463,540.1 ft E 782,965.8 ft     Completion E       Image: N 463,540.1 ft E 782,965.8 ft     Completion E       Image: N 463,540.1 ft     Surface Elev:: 1003.7 ft       Image: N 463,540.1 ft     Surface Elev:: 100.1 ft       Image: N 463,7 ft     Bottom of Boring at 10.0 feet       Image: N 463,7 ft     Bottom of Boring at 10.0 feet       Image: N 463,7 ft     Sufficient C 4630.1 ft       Image: N 463,7 ft     Sufficient C 5630.1 ft       Image: N 47,7 ft	Remarks: Weather:						

DAI									Sheet 1
		Fisher Bren 49191019.00/100/300	Surface Elevation:	1003 HSA		ft			
		Verona, Wisconsin	Drilling Method: Sampling Method:	Split		rrel			
		N 463,545.8 ft E 782,965.2 ft NAVD 88	Completion Depth:	29.0					
									STANDARD PENETRA TEST DATA N in blows
eet	et			bo	<i>"</i>	<u>o</u>	y.	lue %	10 20 30 REC%
tion, f	th, fe	MATERIAL DESCRIPTION		Graphic Log	Samples	Sample No.	% Recovery	N va	RQD % ◆ 20 40 60
Eleva	Dep			Grap	Sal	Sam	% R	SPT, N value or RQD %	SHEAR STRENGTH,
		Surface Elev.: 1003.9 ft							
	-0-	SILTY SAND WITH GRAVEL (SM): very fine to coarse grained;	tan; moist;	0 KC	Μ	1	67	25	0 2,5 25 (P)
-	_	medium dense; some crushed concrete from 2.5 to 4 feet causir 15% gravel, 70% sand, 15% fines.	ng difficult drilling;	Pap	μ	1	01	20	
-	-			000		2	50	23	23
1000-	_								
_	5					3	44	10	10
-	_			000					
-	_			Pape	X	4	67	18	) '18 ()
995-	ob No.:         ocation:         coordinates:         latum:         <								
Project: Job No.: Location: Coordinates: Datum:		Less gravel observed from 10 to 12 feet.			М	5	44	18	
Project: Job No.: Location: Coordinates: Datum:	990.9 ft		Pap					14	
990-	_	POORLY GRADED SAND WITH GRAVEL (SP): fine to coarse medium dense to dense; 15% gravel, 80% sand, 5% fines.	grained; brown; 13.0ft		М	6	33	14	
-	15-			Poor	$\mathbf{H}$				16
-	_			000	Ш	7	0	16	
V	-					•			22
985-	-	Trace silt; moist to wet at 18 feet.		00	Щ	8	39	22	
-	20-			000	M	9	44	35	35 ©
_	-			000	М	5			
-	-	Siltier with depth at 22.5 to 24.5 feet.		000		10	33	38	
980-	_	979.4 ft SILTY SAND WITH GRAVEL (SM): fine grained; brown; wet; de	noo: 10% group! 04 54	60 C					
-	25-	SILTY SAND WITH GRAVEL (SM): fine grained; brown; wet; de 60% sand, 30% fines.	nse; 10% gravel, 24.5ft			11	33	34	34
-	-	976.4 ft							
075	_	SILTY SAND (SM): fine grained; brown; wet; very dense; 5% gra $\rm _{15\%}$ fines.	avel, 80% sand, 27.5ft		X	12	67	70	
9/5-	_	₩75.4 ft POORLY GRADED GRAVEL WITH SAND (GP): brown; wet; ve	20.05		[]				
		gravel, 15% sand, 5% fines. 974.9 ft	, ,						
		Bottom of Boring at 29.0 feet							

BAF										Sh	eet	1 0
Project: ob No.:	_	40101010 00/100/200	Surface Elevation:	1003		ft						
op No.: .ocation		Verona, Wisconsin	Drilling Method:	HSA								
Coordina	ates:	N 400,07 0.0 IT E 700,00 1.0 IT	Sampling Method:	Split 30.0		rrei						
Datum:			Completion Depth:	30.0								
Ŧ								m	10	20	N in b 30	lows
Elevation, feet	feet			Graphic Log	les	e No.	overy	valu D %	REC%	RQI		
evatio	Depth, feet	MATERIAL DESCRIPTION		aphi	Samp	ample	Rec	T, N r RQ	20 SHF			E TH
Ĕ				Ū		Š	%	Ч S O				,
		Surface Elev.: 1003.9 ft							0		2,5	
-		POORLY GRADED GRAVEL WITH SILT AND SAND (GP-GM): bro gravel, 15% sand, 10% fines.	own; fill; 75%	000								
-	-	Difficult drilling from 1 to 3 feet due to gravel. 1000.9 ft		000	]							
1000	_	SAND WITH SILT TO SILTY SAND (SP-SM): fine to medium graine	ed; brown. 3.0ft									
1000- -	- 5											
-	-											
-	_											
- 995-	-											
-	- 10-											
-												
_	-											
- 990-	_											
-	- 15											
-	_											
_	-											
985	_											
-	20-				Barrel ft see of the standard penet test data N in blo penet REC% RQD % ◆	_						
_	_											
_	_	981.4 ft LEAN CLAY TO FAT CLAY TRACE SAND, TRACE GRAVEL (CL-0	CH): dark grav: 22 5ft									
980-	_	moist; medium to high plasticity; 5% gravel, 5% sand, 90% fines.	July, Jain gray, 22.01									
-	25—								-   -	++	$\left  \right $	_
-	-											
	_											
975-	-											
-	30-	973.9 ft Bottom of Boring at 30.0 feet	30.0ft							+	+	_
									-   -			_

BA	RR	,							BORI		
Project: Job No. Locatio Coordir Datum:	: n: nates:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,446.7 ft E 782,864.9 ft NAVD 88	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	1005 HSA Split 10.0	Ba				S	Sheet	1 (
Elevation, feet	Oepth, feet	MATERIAL DESCRIPTION Surface Elev.: 1005.6 ft	00°/	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	STANDARI TEST DAT 10 2 REC% R 20 4 SHEAR \$	A N in b <u>ρ 3ρ</u> QD % ● ρ 6ρ	GTH,
1005— - - -		SILTY SAND WITH GRAVEL (SM): tan; moist; loose to dense; 5 <sup>rd</sup> sand, 15% fines. Difficult drilling from approximately 1.5 to 3 feet due to large grave	-		X	1	61 78	40			4
	5 — - - - 10—	Increased silt and decreased gravel from 7.5 feet to 10 feet. 995.6 ft				3 4 5	56 56 50	12 10 8	12 10 () () () () () () () () () () () () ()		

				1000		£1				She	et 1
Project: Job No.		49191019.00/100/300	Surface Elevation: Drilling Method:	1006 HSA		π					
_ocation		Verona, Wisconsin N 463,452.2 ft E 782,863.0 ft	Sampling Method:	Split		rrel					
Datum:		NAVD 88	Completion Depth:	29.0							
									STANE TEST	DARD PE DATA N	NETRA
eet	et			bo	<i>"</i>	<u>o</u>	ery	lue %	10 REC%		30
Elevation, feet	th, fe	MATERIAL DESCRIPTION		Graphic Log	Samples	Sample No.	% Recovery	N Va	20	RQD %	60 60
Eleva	Dep			Grap	Sal	Sam	% Re	SPT, N value or RQD %	SHE	AR STRE	ENGTH
		Surface Elev.: 1006.0 ft									
	-0-	SILTY SAND WITH GRAVEL (SM): fine to medium grained; tar	n; moist; very loose		Μ	1	89	32	0	2,5	32 ©
1005-	_	to very dense; dense to very dense from 0-4 feet; 5% gravel, 80			μ	I	09	52			
_	_	Difficult drilling from approximately 2 to 3 feet due to large grave	<b>.</b>			2	78	68			
-	-				μ	-					
- 1000-	5					3	61	18		18	+
-	_									$I \mid I$	
-	_				M	4	61	11	11 (P)	1	
_	-										
995-	10-				M	5	22	9	9		
_	_										
_	-				Х	6	33	11	11   19		
_	- 15										
990-	-				X	7	39	8	8		
-	-										
Ţ		Loose/very loose at 18 feet.			М	8	28	2	ő		
_	20-	POORLY GRADED GRAVEL WITH SILT AND SAND (GP-GM	): fine to medium	ft						+ $+$ $+$ $+$	
985-	lo.: 49191019.00/1 ion: Verona, Wisco M 463,452.2 ft NAVD 88 Surface Elev.: 100 	grained; medium brown; saturated; very loose to medium dense sand, 5% fines.	; 70% gravel, 25%	0.0. 0.0.0	Ю	9	28	4	4		
_	_			0.						14	
_	-			0.0.0.	М	10	22	14			
-	25-	WELL GRADED SAND (SW): fine to medium grained; medium	brown; saturated; 25.0	<u>بر بر المعام (</u> ft (مراجع)				_	7/	+++	++
980-	_	loose to medium dense; 95% sand, 5% fines.			М	11	50	7		$\downarrow    $	
	_					12	22	25			
-	_	977.0 ft Bottom of Boring at 29.0 feet	29.0		М	12	22	20		$\ $	
			_0.0							+++	++
		Started: 6/30/19 Water Levels (ft)	Remarks								

TEMPL													
EPORT BARR	BÅ	BB					LC	DG	OF	во	RIN	G 5	A
GLB BOREHOLE LOG R	Project: Job No. Location Coordin	: n: nates:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,706.5 ft E 783,241.3 ft	Drilling Method: Sampling Method:	HSA Split	Spo					Sh	eet 1	of 1
United Construction:     49191019.00/100/300     Drilling Method:       Coordinates:     N 463,706.5 ft E 763,241.3 ft     Completion Depth:       Tatum:     NAVD 88     Completion Depth:	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	TEST 10 REC% 20	DATA 20 RQI 40	N in blo 30 D % ♠ 60	ws/ft ©  				
WPLS49 W/1349131019 VERONA WI SINKHOLE EVALUATIONWORKFILES/FIELD DATA/BORING AND TEST TRENCH LOGS/SOIL BORINGS/FISHER BREN BORING	1000		POORLY GRADED SAND WITH GRAVEL AND SILT (SP-SM): fir moist; medium dense; 15% gravel, 75% sand, 10% fines. (003.6 ft POORLY GRADED GRAVEL WITH SAND (GP): gray; moist; very gravel, 25% sand, 5% fines, contains crushed rock and concrete; d (002.1 ft POORLY GRADED SAND WITH GRAVEL AND SILT (SP-SM): fir brown; moist; medium dense to very dense; laminated; 15% gravel, 15% fines. End of laminations at 4 feet. 995.6 ft	1.5ft dense; 70% lifficulty drilling. 3.0ft ne grained; , 70% sand,			1 2 3 4	92 100 25	17 57 19			27	
ARR.COMPROJECTS	Date Bo Logged	Boiling Started:     Bisher Brein 49191019.001/00/300 dinates     Surface Elevation: Verona, Wisconsin admites     1005.1 ft HSA Split Spoon       0     MAVD 86     F 23.241.3 ft MATERIAL DESCRIPTION     Split Spoon       0     POCH VGRADE SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     100       0     POCH VGRADED SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     100       0     POCH VGRADED SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     100       0     POCH VGRADED SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     100       0     POCH VGRADED SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     100       0     POCH VGRADED SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     100       0     POCH VGRADED SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     300       0     POCH VGRADED SAMD WITH GRAVEL AND BILT (SP-SM): fine grained: fair med. 26% and 0% free.     300       1003.0 ft     100     27       1003.0 ft     100     27       1003.0 ft     100     27       1003.0 ft     100     27       100     100     100       100     100     100       100     100     100       100     100     1											
2				Sheet 1 of Surface Elevation: 1005.1 ft Drilling Method: Split Spoon Completion Depth: 9.5 ft									

TEMPI													
DRT BARR							LC	)G	OF	во	RIN	G t	5B
3 REPC	BA	RR	,								Sh	eet <sup>-</sup>	l of 1
.GLB BOREHOLE LOO	Project: Job No. Locatior Coordin Datum:	: n:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,713.0 ft E 783,240.6 ft NAVD 88	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	HSA Split	Sp							
BRARY.			Sheet         Sheet           Figher Bren 49191019.00/100/300 Verona, Wisconsin NAVD 88         Surface Elevation: 1005.2 ft NAVD 88         1005.2 ft Drilling Method: Smpling Method: 29.5 ft         Surface Elevation: 1005.2 ft           MATERIAL DESCRIPTION         9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9		ows/ft ⊚								
DATAIBORING AND TEST TRENCH LOGS/SOIL BORINGS/FISHER BREN BORING LOGS.GPJ BARRLIBRARY.GLB BOREHOLE LOG REPORT BARR TEMPLA	Elevation, feet	Depth, feet		Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	REC%	RQE 40	60	80	
	_	-0-		gravel 70%						0			5
BREN B	-	APRE         ext:       Fisher Bren 49131013100/100/300 incirc Verona, Wisconsin Canades:       Surface Elevator::       1005.2 ft HSA Sampling Method:       HSA Sampling Method::       HSA Sampling Method::       Split Spoon         immediate       NAVD 88       MATERIAL DESCRIPTION       Image: Split Spoon       Split Spoon         immediate       NAVD 88       MATERIAL DESCRIPTION       Image: Split Spoon       Image: Split Spoon         immediate       Surface Elev::       1005.2 ft       Image: Split Spoon       Image: Split Spoon         immediate       NAVD 88       MATERIAL DESCRIPTION       Image: Split Sp	() ()	$\left \right $									
<b>NGS/FISHER</b>	-	-	Fisher Bren Vertex, Wisconsin NAVD 88       Surface Elevation: Duling Method: Surface Elevation: NAVD 88       1005.2 ft HSA Surface Elevation: Diling Method: Surface Elevation: December 1005.2 ft       1005.2 ft HSA Surface Elevation: Diling Method: Surface Elevation: December 1005.2 ft         MATERIAL DESCRIPTION       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft         Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft         Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft         Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft         Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft       Image and the formed Surface Elevation: December 1005.2 ft         Surface Elevation: December 1005.2 ft       Image and the formed Elevation Imag	5									
DIL BORIN	1000-	5											
CH LOGS/SC	-	-		Surface Elevation:       1005.2 ft         Drilling Method:       Split Spoon         Completion Depth:       29.5 ft         IDESCRIPTION       Image: split sp									
TEST TREN	995— _	10	Cobbly at 10 to 13 feet.										
SORING AND	-	-				X	6	25	6	Sheet 1			
	990-	15 - -				X	Sheet 1 or         on         on       on         on       on         on       on         on       on         on       on         on       on         on       on         on       on         on       on       on         on       on       on         on       on       on         on       on       on       on         on       on       on       on       on         on       on       on       on       on       on         on       on       on       on       on       on       on         on       on       on       on       on       on       on       on       on       on       on       on       on       on       on       on       on <tho>       on       <tho> <tho< td=""><td></td></tho<></tho></tho>						
ORKFILES/F	<u> </u>		Clayey sand lens 17.5 to 18 feet. loose to medium dense. POORLY GRADED GRAVEL WITH SAND (GP): brown; wet; very	Matrix       Enduring Method:       EDS 2, ft         Drilling Method:       Split Spoon.         Completion Depth:       29.5 ft         DESCRIPTION       Image: split s									
ALUATION/M	985- - -	-			<pre>Sheet 1 of i 1005.2 ft HSA Dot 20.5 ft  To 1 06 22 Dot 00 00 00 00 00 00 00 00 00 00 00 00 00</pre>								
NKHOLE EV/	- - 980-	- - 25-				X	10	50	32			Sheet 1         ARD PENETRADATA N in blow         20       30         RQD % ◆         40       60         AR STRENGTH         25         22         19         19         19         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         32         33         34         5         6	
CONA WI SIN	-	-				X	11	50	18				
19 VER	-	_	15% gravel, 70% sand, 15% fines.       3       83       19         Cobbly at 10 to 13 feet.       5       25       6         6       25       6       6         7       25       4       6         987.2 ft       6       25       6         Clayey sand lens 17.5 to 18 feet, loose to medium dense.       7       25       4         POORLY GRADED GRAVEL WITH SAND (GP): brown; wet; very loose to dense;       8       13       3         9       71       11       10       50       32         975.7 ft       9       11       50       18       11       50         975.7 ft       Bottom of Boring at 29.5 feet       29.5ft       15       15       15										
NBARR.COMPROJECTSMPLS49 WI1349131019 VERONA WI SINKHOLE EVALUATIONWORKFILESFIELD			10       Cobbly at 10 to 13 feet.         10       Cobbly at 10 to 13 feet.         15       987.2 ft         15       POORLY GRADED GRAVEL WITH SAND (GP): brown; wet; very loose to dense;         20       80% gravel, 15% sand, 5% fines.         20       987.2 ft         987.2 ft       10.0 ft         987.2 ft       987.2 ft         987.2 ft       10.0 ft         987.2 ft       987.2 ft         987.2 ft       10.0 ft         987.2 ft       987.2 ft         987.2 ft       10.0 ft         987.7 ft       987.7 ft         975.7 ft       Bottom of Boring at 29.5 feet         975.7 ft       99.5 feet         975.7 ft       91.0 ft         97.1 ft       91.0 ft         97.1 ft       91.0 ft         97.1 ft       91.0 ft         97.1 ft       91.0 ft										
BARR.COMPROJE	Date Bo Logged	oring C By: Contra	ompleted: 6/28/19 4:40 pm AMS3 ictor: SES	<sup>0</sup> at 13 feet.	Non-su	bsic	lence	area	boring		al wire	6 inch	es long

TEMPL														
ORT BARR								LC	G	OF	BOF	RING	G 5	SC
G REP	BA	RR	,									She	et 1	of 2
<b>1.GLB BOREHOLE LO</b>	Project: Job No. Location Coordir Datum:	.: n: nates:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,712.0 ft E 783,237.3 ft NAVD 88	Surface Ele Drilling Me Sampling N Completior	thod: ⁄lethod:	1005 HSA Split 41.5	Spo							
BRAR												DATA N		RATION ws/ft © 40
LOGS.GPJ BARRLI	Elevation, feet	Depth, feet	MATERIAL DESCRIPTION			Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	REC%	20 RQD 40 AR STR	% ♠ <sub>60</sub>	80
ORING	1005	-0	Surface Elev.: 1005.4 ft	and lithology							0	2	5	5
NBARR COMPROJECTSWPLS49 W11309131019 VERONA WI SINKHOLE EVALUATION/WORKFILES/FIELD DATA/BORING AND TEST TRENCH LOGS/SOIL BORINGS/FISHER BREN BORING LOGS/GPJ BARR IBRARY GLB BOREHOLE LOG REPORT BARR TEMPLA	1005— - - - 1000—	5 -	5C starts at 30 feet. Refer to 5B for adjacent boring blow counts description for 0 to 29.5 feet.	and lithology										
T TRENCH LOGS/SOIL	- - - 995-													
A\BORING AND TES	- - - 990-	  15-												
SVFIELD DAT	990 - -	-												
ORKFILE	_	20-												
HOLE EVALUATION/M	985— - - -													
<b>D19 VERONA WI SINKH</b>	980— - - -	25	975.4 ft											
NI\13\49131(	975— _	30-	POORLY GRADED GRAVEL (GP): fine to coarse grained; brow dense; few sand, trace silt; 80% gravel, 15% sand, 5% fines. 973.4 ft				X	1	33	21		21 ©		
ECTS\MPLS\49 \	-		SILTY SAND (SM): fine to coarse grained; brown; wet; medium 5% gravel, 80% sand, 15% fines. Sandy silt and lens from 33.5 to 34 feet, 0% gravel, 20% sand a <i>Continued Next Page</i>		;;e; 32.0ft		X	2	67	21		21 (9)		
ARR.COM/PROJE	Date Bo Date Bo Logged Drilling Drill Rig	oring Si oring C By: Contra	tarted: 7/1/19 11:25 am Water Levels (ft) pompleted: 7/1/19 1:00 pm AMS3 At Time of Drilling 1 poserved in adjacent bo	18.0 bring 5B	Remarks: I for bedrock	. Blind	drille	ed to			a boring	drilled	to inv	estigate
₿					Weather: 8	NINOS	ay Ə	unny						

RADD					L	OG	OF	BC	RIN	IG :	5C
B       B         970       -35         SILTY SAND (SM): fine to coarse grained; brown; wet; me         5% gravel, 80% sand, 15% fines. (Continued)         965         40         964.4 ft         POORLY GRADED GRAVEL WITH SAND, TRACE SILT         wet; dense; 80% gravel, 15% sand, 5% fines.         963.9 ft         Bottom of Boring at 41.5 feet         71/19 11:25 am         Water Levels (         20ate Boring Started:         71/19 11:25 am         Water Levels (         71/19 11:25 am         Water Levels (         71/19 11:20 pm         Y At Time of Dmill	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	1005 HSA Split 41.5	Sp						heet		
Elevation, feet Depth, feet	MATERIAL DESCRIPTION		Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	TES	T DATA 20 6 RC 40	QD % 🔶	lows/f
970	SILTY SAND (SM): fine to coarse grained; brown; wet; medium 5% gravel, 80% sand, 15% fines. <i>(Continued)</i>	dense to dense;		X	3	67 0	32 35	0			35 ©
	POORLY GRADED GRAVEL WITH SAND, TRACE SILT (GP): wet; dense; 80% gravel, 15% sand, 5% fines. \$63.9 ft	greyish brown; <u>41.0fi</u> 41.5fi			5	56	35				335 (@)
Date Boring C Logged By: Drilling Contra	0       35       SILTY SAND (SM): fine to coarse grained; brown; wet; medi 5% gravel, 80% sand, 15% fines. (Continued)         1       964.4 ft         9OORLY GRADED GRAVEL WITH SAND, TRACE SILT (C wet; dense; 80% gravel, 15% sand, 5% fines. 963.9 ft         Bottom of Boring at 41.5 feet         1         964.4 ft         POORLY GRADED GRAVEL WITH SAND, TRACE SILT (C wet; dense; 80% gravel, 15% sand, 5% fines. 963.9 ft         Bottom of Boring at 41.5 feet         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9         9 <td>-</td> <td></td> <td>drill</td> <td></td> <td>o 30 fe</td> <td></td> <td>ea borir</td> <td>ng drille</td> <td>ed to in</td> <td>vestig</td>	-		drill		o 30 fe		ea borir	ng drille	ed to in	vestig

BAI	RR	,					99	UL	BO			
Project: lob No. .ocatior Coordin Datum:	: 1:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,827.3 ft E 783,459.8 ft NAVD 88	0	1006 HSA Split 9.0 f	Sp						eet 1	
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION		Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	TEST 10 REC% 20	DARD F DATA 20 RQE 40 GAR STI	N in blo <u>30</u> 0 % ♠ 60	ws/fl
- 1005 - -	0  	Surface Elev.: 1006.2 ft POORLY GRADED SAND WITH GRAVEL (SP): fine to medium g moist; dense to very dense; 15% gravel, 80% sand, 5% fines. Difficult drilling from 1.5 to 3 feet due to large gravel. 1001.7 ft	rained; brown;		X	1 2	83 83	31 66	0		31 (©	
- 1000- - -	5 - -	SILTY SAND (SM): fine grained; brown; moist; loose to medium de 80% sand, 15% fines.	ense; 5% gravel, 4.5ft		X	3	94 83	16 10	10 @	16 9		4
Date Bo	oring S	tarted: 7/1/19 3:05 pm ompleted: 7/1/19 3:30 pm AMS3	Remarks:	Non-sı	ubsid	dence	e area	boring	<u>].</u>			

TEMP												
PORT BARR							LO	g of	во	RIN	G 6	В
OG RE	BAI	RR	,							Sh	eet 1	of 1
GLB BOREHOLE LO	Project: Job No. Location Coordin Datum:	: n:	Fisher Bren 49191019.00/100/300 Verona, Wisconsin N 463,820.9 ft E 783,463.2 ft NAVD 88	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	1005 HSA Split 29.0	Spo						
RARY									TEST	DARD F DATA		RATION ws/ft ⊚
B LOGS.GPJ BARRLIE	Elevation, feet	Depth, feet	MATERIAL DESCRIPTION		Graphic Log	Samples	Sample No.	SPT, N value or RQD %	10 REC% 20 SHE	20 RQD 40 EAR STI	30 0 % ♠ 60 RENGT	40 80 H, tsf
		_0_	Surface Elev.: 1005.8 ft		××××			_	0	21	5	5
FISHER BREN BO	1005— - -	-	-CRUSHED ASPHALT. (005.3 ft POORLY GRADED GRAVEL WITH SAND (GP): moist; medium of gravel, 15% sand, 5% fines, difficult drilling due to large gravel. (002.8 ft CLUE (CANE) (CM) fine mained has monitored interactions of the second secon	3 Oft		X	1 6 2 8					45
BORINGS	- 1000-	5 -	SILTY SAND (SM): fine grained; brown; moist; medium dense to 0 75% sand, 20% fines.	dense; 5% gravel,		X	3 7	8 12	1:	2		
HLOGS/SOIL	-	_					4 7	8 11				
ST TRENCH	- 995-	- 10- -				X	5 8	3 7	7			
RING AND TE	-	_				X	6 8	9 7	7 (9)			
D DATA/BOI	990-	15 -				X	7 8	3 7				
RKFILES/FIEI	-	-	988.3 ft SANDY SILT (ML): fine grained; grey brown to orange brown; moi gravel, 30% sand, 70% fines.	ist; stiff; 0% 17.5ft		X	8 1	00 10	10 ©			
UATION/WO	- 985 <u>¥</u>	20-	984.8 ft POORLY GRADED SAND WITH GRAVEL (SP): fine to coarse gr wet; medium dense; 20% gravel, 75% sand, 5% fines.	rained; brown; 21.0ft		X	9 5	6 27			27	
KHOLE EVAL	-		981.8 ft POORLY GRADED GRAVEL (GP): brown; wet; very dense; 95% 0% fines.	gravel, 5% sand, 24.0ft		X	10 5	6 23		23		
NA WI SINF	980- -	25	0% mes.			X	11	) 22		22 ම		+
1019 VERO	_	-	976.8 ft Bottom of Boring at 29.0 feet	29.0ft	000		12	100				
WBAR. COMPROJECTSIMPLS49 WI1349131019 VERONA WI SINKHOLE EVALUATIONWORKFILESIFIELD DATAIBORING AND TEST TRENCH LOGSISOIL BORINGSIFISHER BREN BORING LOGS. GPJ BARRIBRARY. GLB BOREHOLE LOG REPORT BARR TEMPLA												
BARR.COM/PROJE	Date Bo Date Bo Logged Drilling Drill Rig	oring C By: Contra	ompleted: 7/2/19 9:30 am AMS3	0 Remarks:				rea borin	g.		<u>ı I İ</u>	

#### NOTES

- 1. The boundary lines between different soil strata, as shown on the Soil Boring Records, are approximate and may be gradual.
- 2. The drillers' field log contains a description of the soil conditions between samples based on the equipment performance and the soil cuttings. The Soil Boring Records contain the description of the soil conditions as interpreted by a geotechnical engineer and/or a geologist after review of the drillers' field logs and soil samples and/or laboratory test results.
- 3. We define "Caved Level" as the depth below the existing ground surface at a boring location where the soils have collapsed into the borehole following removal of the drilling tools.
- 4. We define "Water Level" as the depth below the existing ground surface at a boring location to the level of water in the open borehole at the time indicated unless otherwise defined on the Soil Boring Records.
- 5. We define "at completion" for a boring as being the time when our drilling crew has completed the removal of all drilling tools from the borehole.
- 6. The Notes and Legend Record and the Soil Boring Records should not be separated.

<b>RELATIVE PE</b>	RCENTAGE TERMS	 RELAT
no	0%	Frozen or
trace	<5%	Dry = Dus Moist or M
few	5 to <10%	Moist or M
little	10 to <30%	Wet or W
some	30 to < 50%	

#### **TEST RESULTS LEGEND**

 $q_p$  = Penetrometer reading,  $t_{ft^2}$ MC = Moisture Content, % moisture by weight  $P_{200}$  = % Passing the No. 200-mesh Sieve

## RELATIVE MOISTURE TERMS AT TIME OF SAMPLING

Frozen or F = Frozen material Dry = Dusty, dry to touch, absence of moisture Moist or M = Damp to touch, no visible water Wet or W = Visible free water

#### DRILLING METHODS LEGEND

HSA = Continuous flight hollow-stem augers

### **N-VALUE LEGEND**

DS = Drove Stone WH = Weight of hammer and sampling rods.

#### REMARKS LEGEND

NOTES AND LEGEND RECORD

Storm Sewer Exploration

Raymond Road

CTH PD to Ice Age Scenic Trail

City of Madison, Dane County, Wisconsin

WisDOT State ID 5992-09-83

13282

NR = No Recovery OO = Organic Odor

# SAMPLER TYPE LEGEND



2-inch-outside-diameter, split-barrel sampler



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CONSULTING CIVIL ENGINEERS SINCE 1966	WisDOT State ID 5992-09-83	













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	1102 STEWART STREET • MADISON, WISCONSIN 537 Phone: 608-274-7600 • 888-866-SOIL (7645) Fax: 608-274-7511 • Email: soils@soils.ws	13 Storm Sewer Exploration Raymond Road CTH PD to Ice Age Scenic Trail City of Madison, Dane County, Wisconsin

General ocation:												Bori	ng 5	
ATITUDE:	_	LONGITUDE:	_	COUNTY:	Dane	SECTION:		3	CREW CHIEF:	SWK	ORILL RIG:	CME 85	PAGE:	1 of 2
IORTHING:	_	EASTING:	_	TOWNSHIP: (Verona)	6 N	1/4:		SE	LOG REVIEW:	CMR		tomatic	TOTAL DEPTH	21'-0"
TATION:	526+00	OFFSET:	4.0'	RANGE:	8 E	1/4 1/4:		NE	LOG QC:	CMB	DATE STARTEL	01/2019	DATE COMPLI	)/01/2019
1004.0		<i>l</i> lateria		crintia	n	I		N-Val	lue Test Re			- Sample ┌─ Recov		I
	SILTY	GRAVEL	WITH	SAND	'11				1651116	suns	V		110115	0
- - 1002 -	to low moist, <b>FILL</b> -[;	<b>M)</b> — fine plasticity f medium d 36" thick]	fines, vo lense re	ery pale k elative de	prown,		-[3  	0		MC=	=5.3]-/	-  		3
- - 999 —	plastic	<b>CLAY (Cl</b> ity, dark g onsistency	rayish-i		oist,		-[1	8	q <sub>p</sub> =6.	0+; MC=´	19.0]-/	[м		
-	plastic very st	CLAY (CI ity, brown tiff consiste	mottled ency	d, moist, s						q_=2.4,	1.8	———— —[м		6 6
996 —	plastic consis	-	, moist,	stiff to ve	ery stiff	·/· ·/·			q <sub>p</sub> :	=1.5, 3.5, MC=	4.0] <sup>//</sup>	- <sup>∎</sup> \[M		
996	AND C coarse plastic yellow	LY-GRAD GRAVEL ( grained, f ity fines, p ish-brown, /, OUTWA	<b>SP-SM</b> non-pla ale bro , moist,	) — fine t astic to lo wn to dense re	o w elative			3		MC=	=4.2]-/	—[м		
- - - - 990 - -	gravel, POOR SILTY SILT (I GRAV (GP/G mediu	CCASION CLY-GRAD SAND (SI ML) and P EL WITH M) layers; m grained mated pipe c	al cobb DED SA M), and OORL SILT A with fir layers	oles, with ND (SP), I occasion Y-GRADE ND SANE ND SANE ne and fin	some few nal ED D D e to				MC=	MC= P <sub>200</sub> =3	/	[м		- <b>12</b> - - -
- - - 987		timated pipe		at 1 elevation 98	1'-3 <sup>5</sup> / <sub>8</sub> "		[3:	9		MC= P <sub>200</sub> =1		[м		<b>15</b>   
					Boring	5 Con	tinued	On P	age 2/2		V	I		└─18
WATE	R LEVEL	LEGEND		THER LEV	EL LEGEN	ND	] [	DRIL METH		CASING	DRILL FLUID	DE FROM	PTH	HOLE
<b>2</b> ∐ 13'-6" Dr	y at comp	letion	<b>4</b> 13'	-6" Caved a	at completion	on		HSA SAMPL SURFA	LING METH	— IOD(S): AS <del>I</del> : —	None TM D1586	0'-0" 6	 21'-0"	<u>DIA</u> 6.3"
				-	The Note	hae and				Cuttings, B				n Record
	1102 STE Ph F	WART STRE one: 608-27- ax: 608-274-	EET ● M 4-7600 ● 7511 ●	ng Ser ADISON, V 888-866-S Email: soils	VISCONSI OIL (7645 @soils.ws	<b>Inc.</b> N 5371: )			St St CTH I	DIL BORI OIL BORI Orm Sewe Raymo PD to Ice dison, Da DOT State	NG REC er Explo ond Roa Age Sc ne Cou	CORD pration d enic Trai nty, Wisc	I	



Soils & Engineering Services, Inc. 1102 STEWART STREET • MADISON, WISCONSIN 53713 Phone: 608-274-7600 • 888-866-SOIL (7645) Fax: 608-274-7511 • Email: soils@soils.ws CONSULTING CIVIL ENGINEERS SINCE 1966	SOIL BORING RECORD Storm Sewer Exploration Raymond Road CTH PD to Ice Age Scenic Trail City of Madison, Dane County, Wisconsin WisDOT State ID 5992-09-83	13282
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eneral ocation:												Borir	ng 6A	
ATITUDE:	_	LONGITUDE:		COUNTY:	Dane	SECTION:		3	CREW CHIEF:	SWK	DRILL RIG:	<b>CME 85</b>	PAGE:	<b>1</b> of 2
ORTHING:	_	EASTING:	_	TOWNSHIP: (Verona)	6 N	1/4:		SE	LOG REVIEW:	CMB			TOTAL DEPTH	23'-0
TATION:	523+96	OFFSET:	3.5'	RANGE:	8 E	1/4 1/4:		NE	LOG QC:	CMB	DATE START	0/02/2019	DATE COMPLE	/02/201
1004.4		Materia GRAVEL	I Des	criptio	n		╷╷┥╹	I-Va	<sup>ue</sup> Test Re	esults		Sample Recov		<b>—</b> 0
- - - 1002 —	(GM/S to low brown	<b>SM)</b> — fine plasticity , moist, m y, <b>FILL</b> -[3	graine fines, lig edium (	d, non-pla ght yellow dense rela	vish-		-[27			МС	≈=6.4]-	/[м		- - - - - -
- - - 999 —	fine to low pl brown <b>FILL</b> ,	<b>SAND W</b> medium g asticity fin , moist, de mixed with OIL-[36" t	grained es, darl ense rei h LEAN	, non-plas k gray witi lative den	stic to h sity,		-[33			МС	≔9.4]–	м		
- - - - 996	plastic moist <b>TOPS</b>	<b>CLAY (C</b> sity, very a to wet, stii <b>OIL</b> -[12" t <b>CLAY (C</b> sity, brown	lárk bro ff consis hick]m	wn to blac stency, edium	I					a =1 8	<u>=1.8</u> , 2.8 23.9	<u></u> / <u></u>  		6 - - - - - - - - - 9 -
996-	moist consis	to wet, ve stency, oc h-brown	ry stiff t	o stiff			-[5		q <sub>p</sub> =^	q <sub>p</sub> =1.7 1.8; MC=	′, 2.1]– 26.3]∕	/[м `-{м 		9 - - -
993 — - - -		imated pipe	·	at 1	1'-6 <sup>5</sup> / <sub>8</sub> "		-[6		MC=27	q <sub>p</sub> =1.7 q <sub>p</sub> =1.8 7.5; P <sub>200</sub> =	Г	/[м [м 		- 12 -
990 —	-	wet fine to n	SAND (	SP) lense at	t 13'-6"		-[4				5, 2.2]-			-
- - - 987 — -	— fine plastic reddis to mee	EY SAND to mediu ty fines, o h-brown, dium dens stimated pip	m grain dark bro moist to e relati	ed, media own to dat owet, very ve density elevation 98	um rk y loose y 7.2') at		-[4		MC=23	– <del>q<sub>p</sub>=2.6</del> 3.4; P <sub>200</sub> = MC= P <sub>200</sub> =	;95.7] ;95.7] ;21.7] ;36.8]			15 - - - - - - - 18
\\/\TE	ER LEVEL				borning c				_	CARINI				
	t completi							DRIL <u>IETH(</u> HSA	DD SIZE		G DRIL FLUI None	D FROM	PTH <u>TO</u> 23'-0"	HOLE DIA 6.3"
							S	AMPI	ING METH	HOD(S): A	STM D15	86		
											0			
				-	The Nete				ILL: Bento				nil Parine	
								и ке				t of this So	NI ROLINÓ	j Kecol
	1102 STE Pi F	S & Eng WART STR hone: 608-27 Fax: 608-274	EET ● M 74-7600 ● -7511 ●	IADISON, W 888-866-S Email: soils	VISCONSI SOIL (7645 @soils.ws	N 5371 )	3	С	St CTH ity of Ma	PD to Ice dison, Da	ver Exp ond Ro e Age S ane Co	loration		





General location:													Bori	ng 7	
ATITUDE:	_	LONGITUDE:	_	COUNTY:	Dane	SECTION:		3	CREW CH		SWK	RILL RIG:	CME 85	PAGE:	<b>1</b> of
IORTHING:		EASTING: OFFSET:	_	TOWNSHIP: (Verona)	6 N	1/4:		SE	LOG REVI	EW:			omatic	TOTAL DEPTH:	23'-0
TATION:	523+00	OFFSET:	20.0'	RANGE:	8 E	1/4 1/4:		NE	LOG QC:			ATE STARTED: 10/0	02/2019	DATE COMPLE	/02/201
1005.5		Material	Des	criptio	n			N-Va	lue Test	Res	ults				<b>-</b> 0
1005 -	plastic stiff co	city, brown, onsistency,	moist, <b>FILL</b> -[	<i>medium</i> 24" thick]				/H/12 / <del>6</del> "— -	2"		q <sub>p</sub> =0.9,	1.4]- 7.8]- /	—[M ₽-[M ₽\∈ <b>№</b> — -		-
1002	AND mediu plastic moist,	RLY-GRAD GRAVEL (S Im grained, city fines, da loose to de [3'-6" thick]	S <b>P/SM</b> non-p ark yel	) — fine to lastic to lo lowish-br	o ow own,		-[3:					4.0]-	[м		- 3 - - -
999 -	fine gi	<b>SAND WI</b> rained, non- city fines, br to loose re thick]	-plasti rown, i	c to low noist, me	dium	+	+    -[1:	 5			MC=	6.6]-7	  -[M		6   
996-	-						-[6				MC=	6.2]-7	-[M		- 9 - -
993 -	<b>AND</b> coarse plastic	RLY-GRADI GRAVEL (S e grained, r city fines, br	S <b>P-SM</b> non-pla rown te	) — fine to astic to log o yellowis	0 W sh-		8						[NR     [M		- - - <b> 12</b> - - -
990 -	OUTV occas POOF SILTY SILT GRAV (GP/G	n, moist, loo VASH, trace ional cobble RLY-GRADI SAND (SN (ML) and PC (EL WITH S GM) layers;	e to so es, wit ED SA 1), and OORL SILT A with fir	me grave h some ND (SP), l occasior Y-GRADE ND SANE	el, few nal ED D		8 [8				MC=	5.5]	[M		- - - - - - - -
		ım grained l	ayers		Boring	; ∣ 7 Coni	<b>⊥</b> ¶ tinued	On F	Page 2	2/2		1 1			<b>1</b> 8
WATI	ER LEVEL	LEGEND		THER LEV	EL LEGEN	ND	Ι.				CASING			PTH	HOLE
<u>)</u> 17'-2" [	Dry at com	pletion	<b>L</b> 17'	-2" Caved a	at completion	on		METH HSA	A 2	<u>SIZE</u> 2 <sup>1</sup> / <sub>4</sub> "	<u>SIZE</u> — D(S): AST	FLUID None	FROM 0'-0"	TO 23'-0"	<u>DIA</u> 6.3"
								SURFA	ACE PA	ATCH:	_				
					The Note	a and					e Chips, C sidered			il Porine	Dooo
	1102 STE Pl	<b>5 &amp; Engi</b> EWART STRE hone: 608-274-7 Fax: 608-274-7	ET • M -7600 •	ng Ser ADISON, W 888-866-S	VISCONSI SOIL (7645	<b>Inc.</b> N 53713			C	<b>SOI</b> Stor	L BORIN m Sewe Raymon D to Ice	NG REC r Explor nd Road Age Sce	<b>ORD</b> ration		j Reco





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BAR	Κ	,							Sh	eet 1	of
Project: Job No. Location Coordin Datum:	: n: iates:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02180703° Long: -89.52696058° NAVD 88	Drill San	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:			l: 5	1004.4 ft HSA Split Spoon 34.0 ft			
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION Surface Elev.: 1004.4 ft	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	□ Qp/2	0 80 WATE PL		1 <u>20</u> ENT
-	-0	POORLY GRADED SAND WITH GRAVEL (SP): fine	****	M	1	07	24	0 2,5 5 31 0	20	40	60
-	-	to medium grained; grayish tan; moist; dense to medium dense; angular. Crushed concrete and cobbles caused difficult drilling from 1.0 to 3.5 feet. 1000.9 ft		$\lambda$	2	67 79	31 28	28			
1000-	- 5 — -	POORLY GRADED SAND WITH SILT (SP-SM): fine grained; tan; moist; medium dense; trace gravel. 997.9 ft			3	75	10				
-	_	CLAYEY SILT (ML): gray; moist; stiff; orange mottling. 6.5ft 996.4 ft LEAN CLAY (CL): gray; moist; soft to stiff; orange 8.0ft		Å	4	67	5	0 0 6 25			
995-	- 10-	mottling; trace sand.		X	5	100	4				
-	-			X	6	100	4	4 0.75 5			
- 990-	-			Å	7	100	5	0.25			
-	15— -			Å	8	100	4	0.2 <b>6</b>			
-	-			Ň	9 10	100 100	6 6	⊕] 0.75 II ⊕] 0.75 Q.75			
985- 	20-	983.4 ft LEAN CLAY WITH SAND (CL): gray; moist to wet; 21.0ft		$\langle \rangle$	11	100	6	0.75 			-
_	_	LEAN CLAY WITH SAND (CL): gray; moist to wet; 21.0ft very soft to medium stiff; black and orange mottling.		$\langle \rangle$	12	100	5	0.5 5 (0) 0/15			
980¥	- 25-			$\left( \right)$	13	92	1 0				_
-	-				14	83			15 27 15 27 27	,	
- 975-	-	974.4 ft		$\left  \right\rangle$	15	92	4				
-	30-	CLAYEY SAND WITH GRAVEL (SC): fine grained; 30.0ft brown; wet; medium dense.		$\left  \right\rangle$	16	92	18				T
-	-	970.4 ft Bottom of Boring at 34.0 feet 34.0ft		X	17	75	10	10 @			
Date Bo Date Bo Logged	oring (	Started: 5/11/20 10:00 am Water Levels (ft)	l.5					Ground surface elevation base Sample 16 was too disturbe			disc

BAI	κR								Sh	eet 1 of
Project: Job No. Locatio Coordin Datum:	:: n: iates:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02094313° Long: -89.52809309° NAVD 88	Drilli Sam	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:				1008.4 ft HSA Split Spoon 50.0 ft		
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	STANDARD PENETRATION TEST DATA N in blows/ft ⊚ 10 20 30 40 REC% RQD % ◆ 20 40 60 80 SHEAR STRENGTH, tsf	D ( 80 WATE	JRAL DRY ENSITY pcf) * 100 120 R CONTEN (%) X LL
-	-0	Surface Elev.: 1008.4 ft POORLY GRADED SAND WITH GRAVEL (SP): fine	~~~					0 2,5	5 20	40 60
_	-	to medium grained; tan; moist; dense.		X	1	92	44	44	62	
1005-	_	POORLY GRADED SAND WITH SILT AND GRAVEL 2.5ft (SP-SM): fine to medium grained; brown; moist; very dense to loose.		X	2	100	62	25	©	
-	5 —			Å	3	100	25			
- 1000-	_			Å	4	100	14			
-	- 10-			X	5	100	15	15 ©		
-	_			X	6	100	8			
995— _	_			X	7	75	5			
-	15— -			Ň	8	92	10			
- 990	_	989.4 ft		Å	9	100	7			
_	- 20-	POORLY GRADED SAND (SP): fine grained; tan; 19.0ft	<u> </u>		10	79	17			
Ţ	_	986.4 ft SILTY SAND (SM): fine grained; brown; moist to wet; 22.0ft		$\left( \right)$		100	21			
985 <sup></sup>	-	medium dense; interbedded SP-SM lenses; few gravel.		Ň		100	19	20	13.9	
-	25			$\overline{\langle}$	13	83	20	20		
- 980-	-			$\overline{\langle}$	14 15	100 100	20 16			
-	30-			$\overline{\langle}$	15	100	24			
-	_			$\langle \rangle$	17	100	24 26			
975— -	- 35–			$\bigwedge$	.,	100	20	18		
Date Bo Date Bo	oring §	Continued Next Page         Started:       5/12/20 10:55 am         Completed:       5/12/20 3:20 pm         AMS3       After Drilling						Ground surface elevation ba		

Project Job No Locatio Coordir Datum:	: .: n: nates:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin	Surface Elevation Drilling Method: Sampling Method Completion Dep				l : :t	1008.4 ft HSA Split Spoon 50.0 ft	Sheet	2 of
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	10	NATURAL DENSI (pcf) 80 100 WATER CO (%) > PL	TY * 120 NTENT < LL
970	-35- - - 40- - - - - - - 50-	972.4 ft POORLY GRADED SAND WITH GRAVEL (SP): fine 36.0ft to medium grained; brown; wet; very dense to medium dense. 958.4 ft Bottom of Boring at 50.0 feet 50.0ft			18 19 20 21 22 23 24 25	100 63 75 63 58 50 50		0 18 2,5 5 45 0 100/ >>@ 100/ >>@ 100/ >>@ 100/ >>@ 100/ >>@ 100/ >>@		
Date Bo Date Bo Logged Drilling Drill Rig	oring ( By:	Started: 5/12/20 10:55 am Completed: 5/12/20 3:20 pm AMS3 actor: SES ✓ After Drilling Not measured (water add 2 A Time of Drilling 23	ed) 5.0		2	2016	Lidaf	Ground surface elevation base R. Boring flushed with water sta filled with bentonite chips to st	arting at 36' b	Madiso gs.

BAI	R								Sh	eet 1 o				
Project: Job No. Location Coordir Datum:	.: n: nates:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02198134° Long: -89.52678931° NAVD 88	Drilli Sam	Surface Elevation:1001.2 ftDrilling Method:HSASampling Method:Split SpoonCompletion Depth:30.0 ft										
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION Surface Elev.: 1001.2 ft	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	□ Qp/2	80 WATE	URAL DRY DENSITY (pcf) * 100 120 R CONTEN (%) × LL				
_	-0			M	1	0	6		5 <u>2</u> 0	40 60				
1000	-	LEAN CLAY (CL): dark brown; moist; very stiff to stiff; trace roots; trace gravel; trace sand.		$\mathbb{X}$	2	63	10							
-	5 —			X	3	75	7							
995— _ _	-	994.2 ft LEAN CLAY (CL): grayish tan; moist; very soft to stiff; 7.0ft orange mottling.		Å	4	88	4	4 9 0625						
_	- 10-			X	5	100	5	5 [∰ 0.3 <b>/</b> 5	18 31.2	35 ¥				
- 990 _	-			X	6	100	3 <sub>0.</sub>	3 (9) 125						
-	-			X	7	100	6	6) 0.625	10	38				
- 985-	15			Å	8	58	3	0. <b>2</b> 5	29.5	(ii)				
-	-	983.2 ft		X	9	100	5	5 [@) 0.375						
-	- 20-	LEAN CLAY (CL): dark gray to black; moist; very soft 18.0ft to medium stiff.		X	10	100	5							
980- -	-			Å	11	100	1 0.	1' 97 1025	3	47 3				
-	_			Å	12	100	4 0.							
- 975-	25			Å	13	100								
_	_	972.2 ft		$\bigwedge$	14	100	4							
_	- 30-	SILT (ML): gray; moist; medium stiff; trace sand @ 29.0ft 29'. 971.2 ft Bottom of Boring at 30.0 feet		Δ	15	100	6	0.375						

BAR		Fisher Bren Privileged and Confidential	S	faar		vatior		1003.5	ft			Sh	eet 1	of
Project: Job No.		49131019.20/004/300			Meth			ISA	п					
Locatioı Coordin		Verona, Wisconsin Lat: 43.02189895° Long: -89.52665127°		-		ethoo		Split Sp	boon					
Datum:	ales.	NAVD 88		•	-	Dept		30.0 ft						
										PENETR/ N in blow				
et			ŋ		Ċ	~	e	10 REC%	2,0	30	40		URAL DI	
on, fe	, feet		ic Lo	ples	le N	ovel	D %			• % ♦	<b>_</b>		(pcf) ★ 100	
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	20 SHE	40 AR ST		80 I. tsf		R CONT	
Ē			G		S	%	<u>г</u> 0			Qp/2	, 	PL	(%)×	LL
		Surface Elev.: 1003.5 ft						0		Qp/2 2,5	5	20	4 <u>0</u>	60
_		LEAN CLAY (CL): dark brown; moist; trace roots; trace sand.		M	1	46	6	6 0- 0.875						
-	-	1003.0 ft LEAN CLAY (CL): brown; moist; stiff; trace roots;						0.875						
- 1000-	-	trace sand. 2.5ft			2	50	5	5 @] 0. <b>6</b> 25						
-		1001.5 ft POORLY GRADED SAND (SP): brown; moist; few			0	07	_							
-	5 -	silt; trace gravel.		$\mathbb{N}$	3	67	7	7 0 75						
-	_	LEAN CLAY (CL): dark brown; moist; stiff to medium stiff; light brown mottling.		$\mathbb{N}$	4	75	6	6 00 00						
- 995-	-	997.5 ft												
-	-	LEAN CLAY (CL): grayish tan; moist; medium stiff; trace sand; orange and black mottling.			5	88	5	) 05						
_	10-			$\mathbb{N}$	6	100	5	5 [(@)				19	<i>,</i> 40	
_		991.5 ft		$\mathbb{N}$	0			0.3 <b>7</b> 5				28.7		
- 990-	-	LEAN CLAY (CL): grayish tan; moist; soft; orange 12.0ft mottling.			7	100	4	4 [@] 0.26						
390-	-			$\left\{ \right\}$				_5						
_	15-			$\mathbb{N}$	8	75	5	0.2 <b>5</b>						
-				$\mathbb{N}$	9	100	5	5 29 25						
- 985—	-							0.25						
905-	-				10	100	5	0.25				19 28.9		
_	20-			$\mathbf{N}$		7-	-							+
.7		981.0 ft		$\mathbb{A}$	11	75	7	0.25 <b>\</b>						
$\underline{\nabla}$	-	CLAYEY SAND WITH GRAVEL (SC): brown; wet; 22.5ft			12	58	12	12						
980-	-	medium dense.						`	18					
_	25-			$\mathbb{N}$	13	25	18		$ \vec{r} $					
-		Rock fragments observed @ 26-28'.		$\mathbb{N}$	14	4	16		16 P					
075	-	Brown poorly graded sand with silt @ 28-29'.							1					
975- -	-	973.5 ft			15	75	14		4					
	30-	Bottom of Boring at 30.0 feet 30.0ft	1.1.1.											
	rine	Startad: E/40/00.0.50 am Matan Laure (#)			- 1	20	rles of				tion b			
Date Bo	oring C		2.2		2	2016	Lidar	. Boring	j back			ed on Cit onite chip		
Logged Drilling	ру:	actor: SES AMS3	3.0		- I N	vo dri	ilina fl	uids use	d.					

BA	RR	,							Shee	et 1 o	of
Project lob No Locatio Coordir	.: n:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02263444° Long: -89.52516762°	Surface Elevation: Drilling Method: Sampling Method:	1006. HSA Split \$	Spoo	on					
Datum:		NAVD 88	Completion Depth:	50.01	ť			STAND	ARD PE	NETRA	TIO
							0	TEST D			
Elevation, feet	, feet			Graphic Log	Samples	% Recovery	SPT, N value or RQD %	REC%	RQD %		
evatio	Depth, feet	MATERIAL DESCRIPTION		raphi	Samples	Rec	or RQ	20 SHEA	40 R STRE	60 8 ENGTH,	8 <sub>0</sub> tsf
Ξ				U			SF SF			)p/2	
	-0	49131019.20/004/300       Drilling Met         Verona, Wisconsin       Sampling M         Lat: 43.02263444° Long: -89.52516762°       Completion         MAVD 88         MAVD 88         MATERIAL DESCRIPTION         Surface Elev.: 1006.9 ft         POORLY GRADED SAND WITH GRAVEL TO GRAVELLY SAND (SP): fir         poor model of the	ND (SP): fine		/			0	2,5		Τ
- 1005-	_	⊣to medium grained; tan; moist.	0.51		( 1	71	9	9 ©□ \.25			
- 1005	-	LEAN CLAY (CL): brown; moist; very stiff; trace gravel.	/	Ī		75	14	14			
-	_	POORLY GRADED SAND (SP): fine to medium grained; tan; r	moist; loose to								
-	5 —				( 3	67	9	- Ö			+
1000-	_				4	83	10	10 (4)			
-	_						10	13 Ø			
-	- 10-					92	13				
_	_				ίε	100	13	13 ©\			
995-	-			ĥ	/ ,	92	17		17 9		
_	_			ł	) '				/		
-	15-	990.9 ft			( ε	100	14				┢
990-	-	SILTY CLAY WITH SAND (CL-ML): brown; moist; stiff.	16.01	t		100	13	  13  □  ©   0 75			
Ţ		987 9 ft						0.75			
-	- 20-	POORLY GRADED SAND (SP): fine to medium grained; tan; v		/	( 1	0   100	13	0.75			
_	- 20	986.9 ft	/20.0f	t	1	1 100	8	0.375 80			
985-	_	gravel.							\		
	-	Interbedded with 6" layers of sand with gravel (SP).			1:	2 67	21	0.375	Ň		
_	25-				1	3 63	24	0.375	24		+
- 980-	_				1	4 58		_ 11			
-	_							D.375			
-	-	977.4 ft GRAVELLY SAND (SP): fine to medium grained: brown: wet: o	dense to 29.5f		1	5 50	10	□ <sup>10</sup> 0.25	$\downarrow$		
-	30	medium dense.		• O	1	3 42	40				40 Φ
975—	_			0				13	$\square$	$\square$	
-	-				1	7   13	13	Ö	+	$\downarrow$	
	35-	Continued Next Page			$\langle$					$\pm$	$\uparrow$
		Started:         5/14/20 10:30 am         Water Levels (ft)           Completed:         5/14/20 2:30 pm	Remarks 2016 LiD	Ground	l surf	ace ele	evation	based or	n City o	of Madi	ISO

BAI	RR		-1						Sheet 2	of
Project: Job No. Locatio Coordir Datum:	.: n: nates:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02263444° Long: -89.52516762° NAVD 88	Surface Elevation: Drilling Method: Sampling Method: Completion Depth:	1006 HSA Split 50.0	Spo	on				
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION		Graphic Log	Samples	% Recovery	SPT, N value or RQD %	TEST D 10 REC% 20	RD PENETR/ ATA N in blow 20 30 RQD % ♠ 40 60 R STRENGTH □ Qp/2	vs/ft @ 40 ] 80
- 970 -	-35 - - -	GRAVELLY SAND (SP): fine to medium grained; brown; we medium dense. <i>(Continued)</i> 968.9 ft SILTY SAND (SM): fine grained; brown; wet; dense to mediu		o o O ft	1		40		2,5	40/
- 965- -	40	Interbedded with sand and gravel layers.			2	1 83	45		29	4
- - 960-	- 45- - -	interbedded with sand and graven ayers.			2				29 () 31 ()	
-	- - 50-	956.9 ft Bottom of Boring at 50.0 feet	50.01	ft	2	5 63	23		23 ©	
Date Bo Date Bo Logged	oring C	tarted: 5/14/20 10:30 am Completed: 5/14/20 2:30 pm AMS3 Not measured (water ac		AR. Wat	er ad	ded du	iring dr	illing. Inst	n City of Mac	

BAF	κR								Sheet 1 of
Project: Job No. Locatior Coordin Datum:	: 1:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02192090° Long: -89.52670129° NAVD 88	Dril Sar	ling npli	Meth ng M	vatior nod: ethoc Deptl	H 1: 5	1004.3 ft HSA Split Spoon 50.0 ft	
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION Surface Elev.: 1004.3 ft	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	□ Qp/2	NATURAL DRY DENSITY (pcf) ★ 80 100 120 WATER CONTENT (%) × PL LL
-	-0	POORLY GRADED SAND WITH GRAVEL (SP): fine		\$\/					5 20 40 60
-	-	to medium grained; grayish tan; moist; dense; crushed asphalt @ 2.3-2.5'. _1001.8 ft SILTY SAND (SM): fine to medium grained; tan;	t		1	88 92	34 37	37	
1000-	_	moist; dense to loose; few gravel.						15	
-	5			$\mathbb{N}$	3	83 83	15 5		
- - 995-	-				5	75	5		
_	10-				6	92	7		
- - 990-	-	991.3 ft LEAN CLAY (CL): gray; moist; stiff to medium stiff; 13.0f black and orange mottling.	t		7	100	5	0.625	
-	15			Ň	8 9	58 100	5 5	(0) 0) 0) 0) 0) 0) 0) 0) 0) 0)	20_35
-	_	985.3 ft							27.3
985— -	20-	SILT (ML): dark gray; moist to wet; medium stiff to 19.0f soft; light gray laminations.	t		10 11	100 100	5		
	-			$\left  \right\rangle$	12	100	4	0.25 4 0.375	
980	- 25-			$\bigvee$	13	100	5	5 - Ro	
-		977.3 ft		$\left(\right)$				0.375	
-	-	LEAN CLAY (CL): gray; moist; very soft to soft; black 27.0f and orange mottling; trace sand.			14 15	100 100	4 0. 4	1126 4 	18 34 37.7
975	30-							0.25	37.7
-	-	972.3 ft POORLY GRADED GRAVEL WITH SILT AND SAND 32.0f (GR_GM); fine to medium grained; brown; wot:	t o da		16 17	17 92	WH[ 24		
- 970-	35-	(GP-GM): fine to medium grained; brown; wet; medium dense to very dense. Continued Next Page			17	92	24	12	
Date Bo Date Bo Logged	oring C		23.5	1	2	2016 I	Lidar	Ground surface elevation ba R. No drilling fluids used. Bor hips to surface.	

Project: Job No. Location Coordin Datum:	: 1:	Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02192090° Long: -89.52670129°			Meth ng M	vatior nod: ethoo Dept	l :: t	1004.3 ft HSA Split Spoon 50.0 ft	Sh	eet 2	of
Elevation, feet	Depth, feet	MATERIAL DESCRIPTION	Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	□ Qp/2	0 80 WATE PL	ER CONT (%) ×	120 EN <sup>-</sup>
965- - 960- - 955-	-35- 	954.3 ft 954.3 ft 954.3 ft 50.0ft			<ol> <li>18</li> <li>19</li> <li>20</li> <li>21</li> <li>22</li> <li>23</li> <li>24</li> <li>25</li> </ol>	67 38 58 50 25 42 55 42 42	12 22 25 65 17 24 42				
Date Bo Date Bo Logged Drilling	oring ( By:	Completed: 5/11/20 4:20 pm AMS3	3.5 4.0		2	2016	Lidar	Ground surface elevation base R. No drilling fluids used. Borir hips to surface.	ed on Cit Ig backfi	y of Mao lled with	disc

		LOG OF BORING SPT-20-07							,				
	RR	,								S	heet	1 c	of 2
Job N Job N Locati Coord Datum	o.: on: inates:	49131019.20/004/300 Verona, Wisconsin Lat: 43.02182868° Long: -89.52692496° San	face Elevation: ing Method: npling Method: npletion Depth:	1004 HSA Split 36.0	Sp		1						
ARKLIBRARY.JUB BUREHULE LI Elevation, feet	Depth, feet	MATERIAL DESCRIPTION Surface Elev.: 1004.3 ft		Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	TES 1 REC%	6 7 40 HEAR S	A N in   <u>30</u> 2D % €	GTH, t	ft © 
		BLIND DRILLED TO 28'.											
	20-												
	25-												
	30-	976.3 ft SANDY SILTY CLAY (CL-ML): brown; wet; soft; few gravel. 972.3 ft POORLY GRADED SAND WITH GRAVEL (SP): fine to medium gr brown; wet; loose to medium dense; trace clay.	28.0ft ained; 32.0ft			15 16 17	75 25 63	6 ( 3 ( 5					
48131018 VE	35-	Continued Next Page			A					14 9			
Date E Date E Logge Drilling Drill R	d By: g Contra	ompleted: 5/13/20 9:20 am AMS3 At Time of Drilling Not measured (blind drilled)	Remarks: 2016 LiDA 2012932). Weather:	R. No Boring	drill J ba	ing fl ckfill	uids	used.	Install	ed VW	/P @	Madis 36' (V	son's /WP

VTE.GDT					L	OG	0	FE	30	RIN	G	SP	<b>T-2</b>	0-0	)7
EMPLA	BARR					Sheet 2 of 2									
<b>DG REPORT BARR T</b>	Project: Job No.: Location: Coordinates: Datum:		Fisher Bren Privileged and Confidential 49131019.20/004/300 Verona, Wisconsin Lat: 43.02182868° Long: -89.52692496° NAVD 88	Surface Elevatio Drilling Method: Sampling Metho Completion Dep	d:	1004 HSA Split 36.0	ι Sp		ו						
RRLIBRARY.GLB BOREHOLE LC	Elevation, feet	Depth, feet	MATERIAL DESCRIPTION			Graphic Log	Samples	Sample No.	% Recovery	SPT, N value or RQD %	TE REC	EST DA 10 2% F	TA N ii 20 RQD % 40	n blov <u>30</u> 60 NGTI	40 3 80
J BAF	_	-35-	968.3 ft				X	18	63	14			2,5		
P.IMPLS49 WI1349131019 VERONA WI SINKHOLE EVALUATION/WORKFILES/FIELD DATA/BORINGS TRENCHES AND X SECTIONS/SOLI BORINGS/FISHER BREN BORING LOGS/GPJ BARRUBRARY/GLB BOREHOLE LOG REPORT BARR TEMPLATE.GDT			Bottom of Boring at 36.0 feet		36.0ft										
3\4915	Date B	oring S	tarted: 5/13/20 8:30 am Water Levels (ft)	Rema	arks:	Grour	 nd s	urfac	 ce ele	vation	base	ed on	Lity o	f Ma	dison's
1PLS\49 WI\1	Date B Logged Drilling Drill Rig	oring C I By: Contra	ompleted: 5/13/20 9:20 am AMS3 A t Time of Drilling Not measured (blind dr	illed) 2016 21.7 2012	LiDA 932).	R. No Boring	drill g ba	ling f Ickfill	luids	used.	Insta	lled V	WP @		(VWP
P:\M				Weat	ther:	50 F S	Sunr	ny							

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Attachment C Test Pit Logs

Project Name: Verona WI Sinkhole Evaluation	Log of Test Pit No.	Map View										
Client:												
Number:	Elevation:	Total Depth:	Refer to figures 2-5									
Location:	Date Started:		7									
Contractor:	Logged By:											
Depth Control of the	Description											
0-1.5' N/A N/A N/A N/A Compacted, light brown sand. Fill. 80/15/5 (s/f/	ŋ) SM.											
			<u>Remarks:</u>									
1.5-         2.5'         2.5-         12'	avel material with brick pieces, fill.											
			Figure									
	TH (feet)		Barr Engineering Co. 4700 W 77th St. Suite 200 Edina, MN 55435 BARR Telephone: 952-832-2600 Fax: 952-862-2601									
Project Nar	roject Name: Verona WI Sinkhole Evaluation		Log of Test Pit No.	Map View								
-----------------------------------------	--------------------------------------------	---------------	---------------------	-------------------------------------------------------------------------------------------------------------------------------------------------	-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------	--------------	-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------	--	--	--	--	--
Client:												
Number:					Elevation:	Total Depth:						
Location:	ocation:				Date Started:		Refer to figures 2-5					
Contractor:	ontractor:				Logged By:							
Depth (feet) PID Reading (ppm)	Odor	Discoloration	Sheen	DI	DESCRIPTION							
	N/A	N/A	N/A	Compacted, light brown sand. Fill. 70/15/15 (s/f/g	ı) SM.							
1.0- 2.5'				Gravel material with brick pieces, fill.			<u>Remarks:</u>					
2.5-				Brown sand with cobble. 60/10/30 SP-SM.			- The pipe was approximately 12.8' below ground surface (bgs).					
10' 10' 10- 19'				- at 8' excavator found a tar pail lid - at 9.5-10' uncovered a root Fine sand with cobble, 60/10/30 SP-SM. -Water present at 18' bgs.	Image: Sector of the sector		- Tests for density and moisture, and samples for grain size analysis were collected at 5', 12' and 17' bgs. - nuclear density gauge - sand cone - sample for grain size					
							Figure					
				LENGT	H (feet)		Barr Engineering Co. 4700 W 77th St. Suite 200 Edina, MN 55435 Telephone: 952-832-2600 Fax: 952-862-2601					

Project Name: Verona WI Sinkhole Evaluation	Log of Test Pit No.	Map View
Client:		
Number:	Elevation: Total Depth:	
Location:	Date Started:	Refer to figures 2-5
Contractor:	Logged By:	
trace sand and trace gravel 10/85/5, CL	the test trench, lean-fat clay present. Brown clay with trace gray modeling, -CH. Medium-high plasticity, qu = soft-medium (0.5-0.75).	Remarks: - The pipe was approximately 12.6' below ground surface (bgs).
7.0- Lean-fat brown clay with trace sand and	st trench, clay and silt are present. I trace gravel. 15/80/5 CL-CH. Medium-high plasticity. ench, tan silty sand (SM) with a few silt layers 65/30/5. Appears to be undisturbed	<ul> <li>Tests for density and moisture, and samples for grain size analysis were collected at 5', 7.5' and 12' bgs.         <ul> <li>nuclear density gauge</li> <li>sand cone</li> <li>sample for grain size</li> <li>Note: 7.5' bgs sample was based on clay presence.</li> </ul> </li> </ul>
		Figure
	LENGTH (feet)	Barr Engineering Co. 4700 W 77th St. Suite 200 Edina, MN 55435 <b>BARR</b> Telephone: 952-832-2600 Fax: 952-862-2601

Proje	roject Name: Raymond Road - Storm Sewer Soils			nond	Road - Storm Sewer Soils	Log of Test Trench No. TT-	Log of Test Trench No. TT-04						
Client	t: Int	tegrit	ty Gra	ding a	nd Ex	cavating	Project Station: 526+50						
Numb	ber: 4	49/1:	3-101	9.20			Elevation: 1004.7 ft MSL	Total Depth: 4 feet					
Locat	ocation: Raymond Road, Verona, WI				, Verc	na, WI	Date Started: May 20, 2020						
Contra	acto	or: IO	θE				Logged By: KAE						
_	<	1		1	1								
Depth (ft)	Depth (ff) Moisture Color Color												
0-1						FILL - light brown poorly graded silty sand an	d gravel			Remarks:			
1.3	3-2					FILL - gray coarse gravel to cobble			Density te	sting at 2' and 4'			
									6' wide bu	ucket; 10' long			
2-4						FILL - light brown poorly graded sand with gra	avel						
									eet MS				
									ELEVATION (feet MSL)				
									LEVAT				
									Ξ				
									Figure				
									Ba 47	rr Engineering Co. 00 W 77th St. Suite 200 ina, MN 55435			
		1	,	I	1	LENG	GTH (feet)		BARR Te	lephone: 952-832-2600 x: 952-862-2601			

Project Name: Raymond Road -	- Storm Sewer Soils	Log of Test Trench No. TT-(	Map View	
Client: Integrity Grading and Excavating		Project Station: 518+46		
Number: 49/13-1019.20		Elevation: 1007.8 ft MSL	Total Depth: 4 feet	
Location: Raymond Road, Verona, WI		Date Started: May 20, 2020		
Contractor: IGE		Logged By: KAE		
Depth (ft) Moisture g/s/f Grain Size Color	DE	ESCRIPTION		
	ght brown poorly graded silty sand and g	ravel		Remarks:
1.8-2.5	ray coarse gravel to cobble			Density testing at 4'
2.5-4 FILL - re	eddish-brown silty sand			6' wide bucket; 7' long
				WSL)
				ELEVATION (feet MSL)
				EVATIC
				Figure
				Barr Engineering Co. 4700 W 77th St. Suite 200 Edina, MN 55435 Telephone: 952-832-2600 Fax: 952-862-2601

Attachment D Laboratory Test Data



			G	rain Si	ze D	istribu	ition ASTM D	422-16	Job No. : 12551
	Project: Raym	iond Roa	ıd						Test Date: 5/25/20
Beno	rted To: Barr I	Indinor	ing Company						Report Date: 5/28/20
перо	neu io. pari	Ingineer	ing company		Sample				Tiepon Dale. 5/26/20
	Location / Bor	ing No.	Collection Date	Depth (ft)		T		Soil Classification	_
Spec 1	SPT-20-(	)1	5/11/2020	Lean Clay (CL)					
Spec 2	SPT-20-(	)1	5/11/2020	14-16	Bag				
Spec 3	SPT-20-(								
						Sieve	Data		
	Spe	cimen	1			Specir	men 2	5	Specimen 3
	Sieve		% Passing		Sieve		% Passing	Sieve	% Passing
	2"				2"			2"	
	1.5"				1.5"			1.5"	
	1" 3/4"				1" 3/4"			1" 3/4"	
	3/4 3/8"			-	3/4			3/4	
	#4			-	#4			#4	
	#10		100.0		#10		100.0	#10	100.0
	#20		99.9		#20		100.0	#20	99.9
	#40		99.8		#40		99.9	#40	99.3
	#100		99.4		#100		99.6	#100	86.2
	#200		99.2		#200		99.3	#200	76.9
	Sno	cimen	1	-	<u> </u>	ydromet Specir			Specimen 3
Diar	neter (mm)	CIIIIEII	% Passing		Diamet		% Passing	Diameter	% Passing
Dia	0.026		88.8	· ·	0.025		84.8	0.029	63.7
	0.018		74.5	-	0.017		70.1	0.019	51.3
	0.011		53.6		0.011		50.3	0.012	38.1
	0.008		42.1		0.008		40.3	0.009	32.5
	0.006		33.6		0.006		33.0	0.006	26.5
	0.003		26.7 21.6		0.003		24.9 21.0	0.003	<u>21.1</u> 16.0
	0.001		21.0		0.001	Rema		0.001	10.0
	Spe	cimen	1			Specir		5	Specimen 3
	9530	James A	Ave South		Ę.		EERING IG, INC.	Blooming	gton, MN 55431



			Gi	rain Si	ize D	istribu	ution ASTM D	422-16	Job No. : 12551
	Project: Raym	iond Roa	،d						Test Date: 5/25/20
Repo	rted To: Barr I	Ingineeri	ing Company						Report Date: 5/28/20
Перс		Ilgineen	ing Company		Sample				nepuli Dale. Jizuzu
	Location / Bor	ing No.	Collection Date	Depth (ft)				Soil Classification	
Spec 1	SPT-20-0	13	5/13/2020	8-10	Bag	<u> </u>			
Spec 2	SPT-20-0	)3	5/13/2020	14-16	Bag	<b> </b>			
Spec 3	SPT-20-0								
						Sieve	Data		
	Spe	cimen	1	Т		Specir	men 2	S	Specimen 3
	Sieve		% Passing		Sieve		% Passing	Sieve	% Passing
	2"	<u> </u>			2"			2"	
	<u>1.5"</u> 1"				1.5" 1"	ł		1.5" 1"	
	<u>1"</u> 3/4"				<u>1"</u> 3/4"	<u> </u>		1" 3/4"	<u> </u>
	3/8"				3/4			3/8"	<u> </u>
	#4	-			#4			#4	100.0
	#10		100.0		#10		100.0	#10	100.0
	#20		99.6		#20		100.0	#20	100.0
	#40		99.4		#40		99.9	#40	100.0
	#100	_	99.1		#100		99.8	#100	99.9
	#200		98.9		#200 ⊔		99.6	#200	99.9
	Sne	cimen	4			lydromet Specir		<b>I</b> q	Specimen 3
Diar	neter (mm)	Cimen	% Passing	- <b> </b> ,	Diamet		% Passing	Diameter	% Passing
Dia	0.027		82.9	- <del> </del>	0.027		85.8	0.025	92.0
	0.018	-	66.3		0.018		70.1	0.017	78.3
	0.011	1	45.5		0.011	1	49.4	0.011	58.9
	0.008		37.1		0.008		38.0	0.008	48.8
	0.006	_	30.5		0.006		29.8	0.006	38.3
<u> </u>	0.003		24.9		0.003		21.7	0.003	27.5
	0.001		20.4		0.001	Rema	18.4	0.001	21.2
	Spe	cimen	1			Specir			Specimen 3
	9530	James A	Ave South		- <b>Ļ</b>		EERING IG, INC.	Bloomin	gton, MN 55431



				Gı	ain Si	ze D	istribu	ution ASTM D	422-16	Job No. : 12551			
	Project:	Raymor	nd Road	t						Test Date: 5/25/20			
Beno	rtad Tai	Barr En	rinoorii	ng Company						Report Date: 5/28/20			
перо	ieu ro.	Dall Eng	gineern	ng company		Sample				11epon Date: 3/20/20			
	Location	n / Boring	g No.	Collection Date	Depth (ft)	Туре			Soil Classification				
Spec 1	SF	PT-20-04		5/12/2020	10-12	Bag			Lean Clay (CL)				
Spec 2	SF	PT-20-04		5/12/2020	18-20	Bag							
Spec 3	SF	C)											
	Sieve Data												
		Speci	mon 1		I		Specir	mon 9		Specimen 3			
	Sieve	% Passing											
	2"			% Passing		Sieve 2"	,	% Passing	Sieve 2"	70 T 400 mg			
-	1.5"					1.5"			1.5"				
	1"					1"			1"	100.0			
	3/4"					3/4"			3/4"	93.4			
	3/8"					3/8"			3/8"	86.1			
	#4					#4			#4	81.4			
	#10			100.0	_	#10		100.0	#10	75.7			
	#20			100.0		#20		99.9	#20	69.4			
	#40 #100			99.9 99.8		#40 #100		99.9 99.6	#40 #100	<u>60.9</u> 52.2			
	#100			99.6		#100		99.4	#100	44.9			
	11200			00.0				ter Data	11200				
		Speci	men 1				Specir			Specimen 3			
Diar	neter (m			% Passing	]	Diamet		% Passing	Diameter	% Passing			
	0.027	/		80.8		0.026		86.9	0.031	26.5			
	0.018			61.9		0.017		74.1	0.020	20.6			
	0.012			44.0	0.011			56.5	15.3				
	0.008			36.7		0.008		47.3	13.1				
	0.006			30.6	_	0.006		37.7	0.006	10.9			
	0.003			25.2		0.003		29.2	0.003	8.4			
	0.001			21.8		0.001	Rema	23.4	0.001	6.8			
-		Speci	mon 1				Specir			Specimen 3			
		9530 Ja	ames A	ve South				EERING IG, INC.	Bloomin	gton, MN 55431			



				Gr	ain Si	ze D	istribu	ution ASTM D	422-16	Job No. : 12551		
	Project:	Raymor	nd Road	d						Test Date: 5/25/20		
Repo	rtad Tai	Down En	-in cori:	ng Company						Report Date: 5/28/20		
перы	leu io.	Barr En	gineern	ng Company		Sample				Report Date: 5/28/20		
	Locatio	n / Boring	g No.	Collection Date	Depth (ft)	Туре			Soil Classification			
<b>a</b> 1		Silty Clay w/sand (CL-MI										
Spec 1	S	L)										
Spec 2	ec 2 SPT-20-05 5/14/2020 20-22 Bag Silty Clay w/sand (CL-M									L)		
Spec 3	S											
							Sieve	Data				
		Speci	men 1		<b>—</b>		Specir	men 2		Specimen 3		
	Sieve			% Passing	1	Sieve		% Passing	Sieve	% Passing		
	2"	_				2"			2"			
	1.5"				<u> </u>	1.5"			1.5"			
<b> </b>	1"					1"			1"			
	3/4"				-↓	3/4"			3/4"			
	<u>3/8"</u> #4					3/8" #4			3/8" #4			
	#4 #10			100.0		#4	<u> </u>	100.0	#4	100.0		
	#10			99.9		#20		100.0	#10	100.0		
	#40			99.5	1	#40		99.5	#40	99.9		
	#100			93.5	1	#100		93.6	#100	99.6		
	#200			78.9		#200		84.1	#200	99.5		
						H	lydromet					
		Speci					Specir			Specimen 3		
Diar	neter (n	nm)		% Passing		Diamet		% Passing	Diameter	% Passing		
	0.028			59.0	<u> </u>	0.028		61.3	0.026	89.8		
	0.019			46.0 33.3		0.019		48.9 34.0	0.018 0.011	76.7 56.6		
	0.012			26.1		0.012		27.3 0.008		46.1		
	0.005			20.2	+	0.006		21.1	0.006	36.0		
	0.003			13.9	1	0.003	3	13.9	0.003	25.0		
	0.001			10.0	<u>1_</u>	0.001		10.8	0.001			
							Rema					
		Speci	men 1		<u> </u>		Specir	men 2		Specimen 3		
		9530 Ja	ames A	ve South		Ē	OIL NGINI ESTIN	EERING NG, INC.	Bloomin	gton, MN 55431		



			G	rain Si	ze Di	istribu	ition ASTM D	422-16	Job No. : 12551
	Project:	Raymond Roa	d						Test Date: 5/25/20
Reno	rted To:	Barr Engineer	ing Company						Report Date: 5/28/20
пере		Dari Engineer	ing company		Sample				Tiepon Date: 3/20/20
	Locatio	on / Boring No.	Collection Date	Depth (ft)		-		Soil Classification	
Spec 1	S	BPT-20-06	5/11/2020	28-30	Bag			Lean Clay (CL)	
Spec 2	S	6PT-20-07	5/13/2020	28-30	Bag		Sandy Si	lty Clay w/a little gravel	(CL-ML)
Spec 3									
						Sieve I	Data		
		Specimen	1			Specir	men 2		Specimen 3
	Sieve		% Passing		Sieve		% Passing	Sieve	% Passing
	2"				2"			2"	
	1.5"				1.5"			1.5"	
	1"			_	1"		(00.0	1"	
	3/4"			_	3/4"		100.0	3/4"	
	3/8" #4				3/8" #4		93.7 90.6	3/8" #4	
	#4		100.0		#4		88.4	#4	
	#20		99.1		#20		85.8	#10	
	#40		98.6		#40		83.2	#40	
	#100		97.2	#100			75.6	#100	
	#200		95.2		#200 69.2 #20				
					Hy	ydromet			
		Specimen				Specir			Specimen 3
Diar	neter (n	nm)	% Passing		Diamete		% Passing	Diameter	% Passing
	0.027		83.4 70.1	_	0.029		50.5 37.8		
	0.018		55.4		0.020		26.8		
	0.008		46.5		0.009		20.3		
	0.006		37.0		0.006		16.0		
	0.003		25.1		0.003		10.7		
	0.001		17.7		0.001		6.5		
		<u> </u>				Rema			
		Specimen	1			Specir	nen 2	5	Specimen 3
		9530 James /	Ave South		- <b>F</b> -	OIL NGINI ESTIN	EERING IG, INC.	Bloomin	gton, MN 55431





COPER	Mini			ex Unit Weig	ht		
TESTING LABORATORY		ASTM D4	254 & AST	M D4253			
CTL Job No.: 553-018			Boring	St. 518+46		Date:	6/1/2020
Client: Barr Enginee	rina	-	Sample:			Tested :	PJ
Project Name: Raymond Rd	-	-	Depth (ft.):			Checked:	-
Project No: 49/13-1019.2		-	• • •			· · ·	
		_					
Visual Description: Brown SAND	w/Silt & Gra	avel					
INC	DEX UN	T WEIG	HT TEST	RESULT	S		
					•	]	
	Minimu	m Index Ur	nit Wt., pcf	88.9			
				<u> </u>			
	Maximu	m Index Ur	nit Wt., pcf	122.2			
			- 1				
	GRAD	ΔΤΙΟΝ Τ	EST RE			ן	
	UNAD			JULIU			
		Gradation /	As Received				
	Sieve #	Wt. Retained	% Retained	% Finer			
	3"	-	-	-			
	1 1/2"	-	-	-			
	3/4"	-	-	-			
	1/2"	-	-	-			
	3/8"	-	_	-			
	#4	_	_	_			
	π <b>-</b> +	1 -	-	-			
		Tosting	Remarks				
Minimu	m Index Unit	-					
Test Method used:		A	ן				
Size of mold used (ft <sup>3</sup> ):		).5					
	Method A - F	Funnel/ Scoop	).				
	m Index Unit		1				
Test Method used:		IB					
Size of mold used (ft <sup>3</sup> ): Remarks:	Method 1B -	).5 Wet Soil.				]	
Neillaiks.							

COPER	Mini			ex Unit Weig	ht		
TESTING LABORATORY		ASTM D4	254 & AST	M D4253			
CTL Job No.: 553-018			Boring	St 526-50		Date:	6/1/2020
Client: Barr Enginee	rina	-	Sample:			Tested :	0/1/2020 PJ
Project Name: Raymond Rd	-	-	Depth (ft.):			Checked:	-
Project No: 49/13-1019.2		-				· -	
Visual Description: Brown SAND	w/Silt & Gra	avel					
IND	<b>DEX UN</b>	T WEIG	HT TEST	RESULT	S		
						1	
	Minimu	m Indox I Ir	nit Wt., pcf	107.1			
	wiiiiiiiu		ni wi., por	107.1			
			I				
	Maximu	m Index Ur	nit Wt., pcf	128.9			
	GRAD	ATION T	EST RE	SULTS			
						-	
		Gradation	As Received				
	Sieve #		% Retained	% Finer			
	3"		/o rectained	/01 11101			
		-	-	-			
	1 1/2"	-	-	-			
	3/4"	-	-	-			
	1/2"	-	-	-			
	3/8"	-	-	-			
	#4	-	-	-			
		Testing	Remarks				
	m Index Unit	Weight:	1				
Test Method used:		A	ł				
Size of mold used (ft <sup>3</sup> ):		).5 Funnel/ Scoop					
Remarks:			•				
Maximu	m Index Unit	Weight:					
Test Method used:		IB					
Size of mold used (ft <sup>3</sup> ):		).5					
Remarks:	Method 1B -	wet Soil.					



June 11, 2020

Project 13282

Mr. Kevin Eisen Barr Engineering Company 4300 Market Pointe Drive Minneapolis, Minnesota 55435

Subject: Test Pits and Field Density Testing Raymond Road Storm Sewer Madison, Wisconsin Field Report 3

Dear Mr. Eisen:

On May 20, 2020, we met with you at the subject site to perform field density tests on and to collect samples of previously-placed storm sewer trench backfill materials.

You directed us to a test pit site excavation located on the existing Raymond Road storm sewer line at Station 526+50. At that location, a backhoe equipped with a cleaning bucket was used to excavate a roughly 10-foot-long test pit alongside the storm sewer trench line to a depth of approximately 2 feet below the existing road subgrade surface. At your request, we performed a series of three nuclear density tests on the exposed trench backfill material at the 2-foot depth along the length of the test pit. We collected a composite sample of the backfill material from each of our density test locations. The depth of the test pit was then extended to approximately 4 feet below the road subgrade surface. As instructed, we perform another 3 field density tests on the backfill material at this elevation and collected another composite sample of the material from our field density test locations.

You requested that a second test pit be excavated on the storm sewer trench at Station 518+46. The test pit at that location was excavated directly to a depth of approximately 4 feet below the road subgrade surface. At your direction, we performed one density test on the existing trench backfill material at the 4 foot depth. Upon completion of our testing, you instructed the backhoe operator to extend the test pit depth by approximately 1 foot in order to retrieve a larger sample of the trench backfill material immediate below the surface of our test site. This was accomplished and we collected a 200(+/-)-pound representative sample of material retrieved from the bottom foot of that test pit. You took possession of approximately 150 pounds of the composite sample from the Station 518+46 test pit. We retained the remainder of that sample and delivered it, along with the two composite samples we collected from the test pit at Station 526+50, to our laboratory for testing. We labeled each composite sample according to the test pit location and depth from which

Phone: 608-274-7600 888-866-7645

00 Fax: 608-274-7511

511 www.soils.ws soils@soils.ws Barr Engineering Company Raymond Road Storm Sewer June 11, 2020 Project 13282 Field Report 3 Page 2

they were collected. The sample number, material description, test pit station, and depth for each of the three samples are summarized in Table 1.

Sample No.	Test Pit Station	Depth <sup>*</sup> (in feet)	Material Description
1	526+50	2	Poorly-graded Silty Sand and Gravel (SP-SM), light brown
2	526+50	4	Poorly-graded Sand with Gravel (SP), light brown
3	518+46	4	Silty Sand (SM), reddish-brown

\*Depth is the distance below the existing road subgrade surface at the named test pit location.

We performed the particle size analysis test (ASTM Designations C117 and C136) and the optimum moisture and maximum density determination test (ASTM D698) on each of Samples 1, 2 and 3. Based on the particle size analyses test results, we classified the samples in accordance with the Unified Soil Classification System (USCS) as described in ASTM Designation D 2487. The Particle Size Analysis Test Reports for Samples 1, 2, and 3 are included with this report as Figures 1a, 2a, and 3a, respectively. The Optimum Moisture/Maximum Density Test Reports for Samples 1, 2, and 3 are included with this report as Figures 1b, 2b, and 3b, respectively.

We performed the field density tests in accordance with nuclear gauge test procedure ASTM Designation D 6938. The percent compaction is based on the maximum dry density determined for the material in accordance with ASTM Designation D 698, standard Proctor method. The test results are summarized as follows:

Test	Location of Test	Depth*	Percent <u>Moisture</u>	Field Dry Density, pcf	Maximum Density, pcf	Percent Compaction
	Raymond Road Storm Sewer Trench B	ackfill				
1	Station 526+50, West End of Test Pit	2	4.5	121.9	127.0	96
2	Station 526+50, Center of Test Pit	2	6.3	121.3	127.0	96
3	Station 526+50, East End of Test Pit	2	9.0	131.3	127.0	103
4	Station 526+50, West End of Test Pit	4	5.2	129.1	123.6	104
5	Station 526+50, Center of Test Pit	4	7.8	127.8	123.6	103
6	Station 526+50, East End of Test Pit	4	7.0	128.4	123.6	104
7	Station 518+46, Center of Test Pit	4	7.7	121.4	120.5	101

\*\* Depth is the distance in feet from the existing top of roadway subgrade elevation to the surface of the test site.

We have prepared this report for the subject project according to materials testing engineering practices generally accepted at this time. No other warranty, either expressed or implied, is made.

If you have any questions concerning this work or if we can be of any further assistance to you, please contact us at (608) 274-7600.



Barr Engineering Company Raymond Road Storm Sewer June 11, 2020

Project 13282 Field Report 3 Page 3

Duane E. Reichel, P.E.

DER:MKM:mkm









Computed Total Sample Maximum Dry Density\*\* = **127.0** pcf

\*Laboratory Maximum Dry Density determined for material passing the <sup>3</sup>/₄-inch sieve using a mechanical rammer with a pie-shaped face.
\*\*Computed Total Sample Maximum Dry Density determined using an estimated coarse aggregate specific gravity = 2.6 and an oversize (retained <sup>3</sup>/₄" sieve) material content of 13%.

						Percent Material		Remarks				
Moisture	Liquid	Plastic	Plasticity		Retained	Retained						
Content	Limit	Limit	Index	3/4"	3/8"	#4	#200					
_			-	13	20	24	8.2					
		ample Nan	ne					Sample Classification				
Sample 1 SES Sam		397 {obtai	ned 5/20/20	020}	fines, fine	r-GRADE e to mediu	ADED SAND WITH SILT AND GRAVEL (SP-SM) — 24% gravel, 68% sand, 8.2% nedium grained, non-plastic to low plasticity fines, light brown					
	1102 ST F	EWART S Phone: 608 Fax: 608-	STREET • 3-274-7600 274-7511 •	MADISO • 888-8 • Email: s	Service N, WISCO 66-SOIL (7 soils@soils RS SINCE	NSIN 537 645) s.ws	1	LABORATORY TEST RESULT RECORD Raymond Road Dane County, Wisconsin				





# Computed Total Sample Maximum Dry Density\*\* = 123.6 pcf

\*Laboratory Maximum Dry Density determined for material passing the <sup>3</sup>/<sub>4</sub>-inch sieve using a mechanical rammer with a pie-shaped face. \*\*Computed Total Sample Maximum Dry Density determined using an estimated coarse aggregate specific gravity = 2.6 and an oversize (retained <sup>3</sup>/<sub>4</sub>" sieve) material content of 7%.

				Percent Material			Remarks					
Moisture	Liquid	Plastic	Plasticity		Retained P		Passing	TOHIG RS				
Content	Limit	Limit	Index	3/4"	3/8"	#4	#200					
	-			7	14	19	3.3					
Sample 2		ample Nan	ne					Sample Classification				
Sample 2 SES Sample No. 1898 {obtained 5/20/2020} POORLY-GRADED SAND WITH GRAVEL (SP) — 18% gravel, 79% sand, 3.3% fines, fine to medium grained, light brown, trace fines												
	1102 ST F	EWART S Phone: 608 Fax: 608-	nginee STREET ● S-274-7600 274-7511 ○ NG CIVIL E	MADISO • 888-8 • Email: s	N, WISCO 66-SOIL (7 soils@soils	NSIN 537 645) .ws		LABORATORY TEST RESULT RECORD       N         Raymond Road       N         Dane County, Wisconsin       N	FIGURE 2b			



Printed on 5/22/2020



\*Laboratory Maximum Dry Density determined for material passing the 3/4-inch sieve using a mechanical rammer with a pie-shaped face.

					Percen	t Material		Remarks			
Moisture	Liquid	Plastic	Plasticity		Retained	1	Passing				
Content	Limit	Limit	Index	3/4"	3/8"	#4	#200				
	_		-	5	7	8	13.6				
		ample Nan	ne					Sample Classification			
Sample 3 SES Sam		399 {obtai	ned 5/20/2	020}	SILTY S plasticity	AND (SM) fines, red	<b>1)</b> — 8% gravel, 78% sand, 13.6% fines, fine grained, non-plastic to low ddish-brown, few gravel				
Printed on 5/2	1102 ST F	EWART S Phone: 608 Fax: 608-	<b>nginee</b> STREET • 3-274-7600 274-7511 • NG CIVIL E	MADISO • 888-8 • Email: s	N, WISCC 66-SOIL (7 soils@soils	NSIN 537 7645) s.ws		LABORATORY TEST RESULT RECORD Raymond Road Dane County, Wisconsin			



May 5, 2020

Project 13282

Mr. Thor Wick Integrity Grading & Excavating, Inc. 605 Grossman Drive Shofield, Wisconsin 54467

Subject:

Test Pits and Field Density Testing Raymond Road Storm Sewer Madison, Wisconsin Report 2

Dear Mr. Wick:

On April 20, 2020, we were at the subject site, at your request, to make field observations and to perform field density tests for use in correlating information collected by a truck-mounted cone penetrometer testing rig.

We met at the site with Mr. Brian Moynihan of Barr Engineering Company. A backhoe operated by personnel from your firm had excavated a test pit into the previously-placed storm sewer trench backfill material at approximately Station 526+50 on Raymond Road. The material excavated from the test pit had been set aside for use as backfill to restore the test pit area to the existing top of street subgrade elevation. The test pit was approximately 12 feet long and 6 feet wide and extended to a depth of 12 to 14 feet below the existing road subgrade surface. We observed that a portion of the side of one section of the reinforced concrete storm sewer pipe was exposed near the bottom of the test pit.

Based on our telephone conversation with you on the morning of April 20, 2020, we understood that the test pit was to be backfilled in a manner consistent with the current City of Madison requirements for placing and compacting utility trench backfill materials in roadway areas. We further understood that we were to perform field density tests on the backfill material at several different elevations as the backfill was being placed and compacted.



1102 Stewart Street Madison, WI 53713-4648 Phone: 608-274-7600 888-866-7645

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Integrity Grading & Excavating Raymond Road Storm Sewer May 5, 2020 Project 13282 Report 2 Page 2

Due to safety concerns related to the depth and side wall stability of the test pit, we did not enter the excavation until the backfilling had been completed to within 4 feet of the adjacent roadway subgrade surface.

We noted that the backfill materials tested consisted of a well-graded, fine- to medium-grained sand with silt, gravel, and some broken concrete "breaker run" pieces. During our site visit, we observed that the backfill material was placed in approximately 18-inch-thick lifts and that each lift was compacted using a backhoe-mounted vibratory plate compactor.

We performed the density tests in accordance with ASTM Designation D 6938, nuclear test procedure. Percent compaction was not determined. Our test results are summarized as follow:

Test	Location of Test	Depth*	Percent Moisture	Field Wet Density, pcf	Field Dry Density, pcf
	Test Pit Backfill at Raymond	Road Station 52	6+50		
А	Center of Test Pit	4.0	7.1	140.3	130.9
В	Center of Test Pit	2.5	6.3	133.2	125.2
С	Center of Test Pit	1.0	6.4	137.9	129.6

\* Depth is the distance in feet from the test surface to the existing street subgrade elevation.

At your direction, we provided Mr. Moynihan with our field density test results prior to leaving the site.

We have prepared this report for the subject project according to materials testing engineering practices generally accepted at this time. No other warranty, either expressed or implied, is made.

If you have any questions concerning this work or if we can be of any further assistance to you, please contact us at (608) 274-7600.

Respectfully submitted,





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Attachment E Collier Geophysics MASW May 2020 Report



7711 W. 6<sup>th</sup>, Ste G | Lakewood, CO 80214 O: (720) 487-9200

A Service-Disabled Veteran-Owned Small Business (SDVOSB)

June 8, 2020

Barr Engineering Company 4300 Market Pointe Drive Suite 200 Minneapolis, MN 55435

Attn: Kevin Eisen, P.E. Office: (952) 832-2937 Email: kkeisen@barr.com

RE: MASW Geophysical Investigation Stormwater Pipeline Alignment, Verona, WI Collier Geophysics Project 20-066

Collier Geophysics, LLC (Collier) conducted a geophysical investigation on behalf of Barr Engineering Company (Barr) along the storm water pipe beneath Raymond Road in Verona, WI (Figure 1). The objective of the investigation was to determine the presence of soft soil conditions, laterally and vertically, near the existing and recently constructed stormwater pipeline. To meet this objective Collier utilized the Multi-channel Analysis of Surface Waves (MASW) method to measure the shear-wave velocity (Vs) along the pipeline alignment. The MASW method allowed for the determination of the two-dimensional distribution of shear-wave velocity (Vs) in the subsurface to depths of between 50 to 70 feet below ground surface along the pipe alignment.

The geophysical investigation was conducted between April 28<sup>th</sup> and May 4<sup>th</sup>, 2020. Three survey lines (Lines 1, 2 and 3) were conducted along the pipe alignment: Line 2 directly over the pipe and Lines 1 and 3 located parallel to Line 2 and 15 feet off center. The MASW seismic survey was led by a Collier senior geophysicist Ted Powell. The following report presents results from the geophysical investigation and summarizes the site conditions, field methods, data acquisition, and processing procedures. For further information regarding the details of the MASW technique, Collier will provide a MASW "method addendum" upon request.

# Site Conditions

Seismic Lines 1 and 2 were located primarily on Raymond Road, which was constructed of both asphalt and gravel. Line 3 was collected in the right-of-way along the southeast side of Raymond Road. On each end of the pipeline the pipe alignment deviated from beneath Raymond Road in fairly sharp bends referred to as "doglegs", with the survey lines tracking the alignment across grassy areas, gravel and minor asphalt pathways. The weather ranged from intermittent rain at the beginning of the surveys to generally warm and sunny, sometimes windy,

spring days with cool mornings. Ground conditions ranged from partially wet and somewhat muddy on the doglegs to dry pavement and gravel on the road and grass and weedy areas on the right of way. See Figure 3 for representative site photos.



Figure 1. Site location shown by red line (Google Earth Imagery).



Figure 2. Seismic coverage shown in red (Google Earth Imagery).

Seismic Investigation Project #20-066 June 8, 2020



Figure 3. Photos showing typical site conditions at the time of MASW data collection.

### Data Acquisition

Data were acquired using a Geometrics Geode, 24channel seismograph (*inset photo at right*) with 24 4.5 Hertz vertical component geophones mounted on a Geostuff land streamer. A land streamer is an array of geophones fastened to a tow strap designed to be towed along the ground. The land streamer used for this project had a geophone spacing of 5 feet.

Due to site conditions at the time of data collection, the southern end of Line 3 was collected using the same shot and geophone spacing as the rest of the data collected, but with geophones mounted on



spikes that were planted into the ground. Only the first 115 feet of Line 3 were collected using the spiked geophones before switching back to the land streamer for the remaining portion of the line.

This system utilizes a 24-bit seismograph connected to a field laptop via Ethernet cable. Analog data from the 24 geophones are collected in the Geode seismograph where the data are digitized, transmitted to the laptop computer, and then recorded on the hard drive. The land

streamer and geophones were placed on the low side of the Geode, with geophone 1 farthest away and geophone 24 nearest to the Geode (in the direction of travel of the system).

MASW acquisition parameters consisted of a 2 second record length and a sample interval of 0.5 milliseconds. The seismic source for the surveys was provided by manually striking a plastic plate with a 16-pound sledge hammer. Three hits were collected at each shot point. The shot point spacing was 30 feet. The first shot location was collected 30 feet off the high end of the streamer. After acquiring three shots at the first shot point, the streamer was pulled (via pickup truck or by hand) 30 feet ahead, and data was acquired at another shot point, as prior.

Approximately 9,000 feet of seismic data were collected along the pipeline alignment between the three lines. Line placement was based on previously surveyed station markers, plan maps of the alignment location, and visible cues from surface features such as manholes and end of pipe alignment. Shot locations and select geophone positions were recorded using a Trimble Geo7x handheld GPS unit capable of sub-foot horizontal precisions.

# **Data Processing**

MASW analysis consists of generating a frequency-velocity transform from surface waves, picking the transformed data to derive a dispersion curve, and then inverting this dispersion curve to a layered Vs model. Figure 4 illustrates this dispersion curve picking approach used for MASW soundings, with an example from this investigation. These steps are repeated for each shot location down-line using 6 geophones at a time. The resulting layered models are combined to generate a two-dimensional (2D) S-wave velocity (Vs) model. The program SurfSeis, version 5.3, by the Kansas Geological Survey was used to accomplish these steps.

The Vs models output from SurfSeis were processed, filtered, and gridded using Geosoft Oasis montaj (Geosoft), version 9.3, a processing and data visualization software suite used for analysis of geophysical data sets. Final figures were generated using Surfer (Golden Software), version 16.6.



Figure 4. Example dispersion curve from this project.

#### **Results and Discussion**

The results from the MASW survey can be found in Figures 5-8, attached to the end of this report. The results are displayed as color cross sections with warm colors (red, orange and yellow) representing lower velocity values and green representing higher velocity values. The horizontal axis is in distance down the line and the vertical axis is in elevation, both in feet. The project stationing is displayed along the top of the color cross sections.

Figure 5 shows the results from all three lines at a vertical exaggeration of 4:1. Figures 6-8 show each line at a 5:1 vertical exaggeration along with a larger scale overall.

### Closure

The MASW data were collected and processed using industry standard methods. The data was of sufficient quality to make measurements of the subsurface seismic shear wave velocities of the material beneath the lines which can be related to geotechnical properties of the material.

The geophysical methods and field procedures defined in this report were applicable to the project objectives and have been successfully applied by Collier geophysicists to investigations of similar size and nature. However, sometimes field or subsurface conditions are different from those anticipated and the resultant data may not achieve the investigation objectives. Collier warrants that our services were performed within the limits prescribed for this project, with the usual thoroughness and competence of the geophysical profession. Collier conducted this project using the current standards of the geophysical industry and utilized in house quality control standards to produce a precise geophysical survey.

If you have any questions regarding the field procedures, data analyses, or results presented herein, please do not hesitate to contact us. We appreciate working with you and look forward to providing Barr Engineering with geophysical services in the future.

Respectfully Submitted, Collier Geophysics, LLC

Ted Powel Senior Geophysicist

(1 copy e-mailed PDF format)

acob Sheekan

Jacob Sheehan Senior Geophysicist







Centerline Results							
Raymond Rd. MASW							
Verona, WI							
Barr Eng							
Project #: 20-066	COLLIER GEOPHYSICS						
Drafted by: J. Sheehan	Checked by: N. Pendrigh	Figure 7					

